

DEPARTMENT OF TRANSPORTATION
BUREAU OF HIGHWAYSJEFFERSON COUNTY
LOUISVILLE-LEXINGTON ROAD
164 E. B. SAFETY WIDENING

ESTIMATED QUANTITIES

SHEET DESIGNATION	SHEET NUMBER	CONCRETE CLASS "A" (CU. YDS.)	CONCRETE CLASS "AA" (CU. YDS.)	SHEAR ② CONNECTORS (LUMP SUM)	STEEL REINFORCEMENT (LBS.)	STRUCTURAL STEEL ① (LUMP SUM)	STRUCTURE EXCAVATION COMMON (CU. YDS.)	END BENT BACKFILL ⑤ (LUMP SUM)	4" CONCRETE SLOPE PROTECTION (SQ. YDS.)	REMOVAL OF EXISTING ③ REINF. CONC. (LUMP SUM)	DECK DRAINAGE (LUMP SUM)	OVERHEAD SIGN SUPPORT (LUMP SUM)	BRIDGE RAILING-NEW (LIN. FT.)	BRIDGE RAILING-RELOCATED (LIN. FT.)	PILES-14" CAST IN PLACE FURNISHED (LIN. FT.)	PILES-14" CAST IN PLACE DRIVEN (LIN. FT.)	LIGHTING (LUMP SUM)	MAINTAIN TRAFFIC ④ (LUMP SUM)	EXPANSION HOOK BOLT ⑥ (EACH)
TITLE SHEET	1																		
GENERAL NOTES	2-2A																		
LAYOUT	3-4																		
TYPICAL SECTIONS	5		22.6		5029														
END BENT IOIE	6-7	46.6	22.4		5,015			1	285						606	606			
PIERS	8	240.0	797.0		21,222	280									1,571	1,571			
STRUCTURAL STEEL	9-22		1127.4	1		1													
EXPANSION DAMS	23-24		632.0		167,747	186,699													
SUPERSTRUCTURE	25-30		576.0		163,120														
CONSTRUCTION ELEVATIONS	31-34																		
PILE RECORD	35-36																		
SIGN SUPPORT	37-40																		
LOG OF SOUNDINGS	41																		
BRIDGE LIGHTING	42-43																		
Δ Δ	52-54	56Δ	22.6	797.0	26251	186,699													
			22.4	1127.4	26,251	186,699													
SUBTOTAL - SUBSTRUCTURE		286.6	22.4	632.0	26,251	186,699	280	1	285						2,177	2,177			
SUBTOTAL - SUPERSTRUCTURE			576.0	1	163,120	1							25	890					2640
TOTAL		286.6	598.4	1	189,357	1	280	1	285	1	1	1	25	890	2,177	2,177	1	1	2640
DECK OVERLAY (Sheets 44 thru 50) For Quantities See Sheet 44.																			

SPECIAL PROVISIONS

- ① Structural Steel weight is approximately 573,400 lbs. (Includes 22,800 lbs. for Expansion dams, shoes, & armored edges) included in lump sum bid for Structural Steel. (Includes 100,880 lbs. for raising Exp. Dams etc. as shown on Sheet 44.) 139,307
- ② Approximately 1858 lbs. included in Lump Sum Bid for Shear Connectors.
- ③ Approximately 890 L.F. of deck slab, plinth, curb, & one wingwall to be removed.
- ④ Plus 1200 L.F. of Guardrail, Curb and Slab.
- ⑤ Includes both traffic on surface streets and on I64.
- ⑥ Approximately 155 C.Y. End Bent Backfill and 250 C.Y. End Bent Structure Excavation Common. (see sheet 7)

- NO. 12 JOINT SEALING COMPOUND
- NO. 30-B MEMBRANE CURING OF CONCRETE STRUCTURES
- NO. 35-B CLASS "AA" CONCRETE.
- NO. 36-A SET-RETARDING ADMIXTURES FOR CONCRETE.
- NO. 37-D PREFORMED COMPRESSION JOINT SEAL.
- NO. 46-D WATER POLLUTION CONTROL.
- NO. 48-A BRIDGE DECK REPAIRS.
- NO. 73-A EPOXY GROUT AND BOND COAT.
- NO. 75-A OPERATION OF CONSTRUCTION VEHICLES OVER PAVEMENT AND STRUCTURES.
- NO. 80-B BLAST CLEANING AND PAINTING STRUCTURAL STEEL.
- NO. 82-A ROADWAY LIGHTING SYSTEMS.
- NO. 89-B REFLEX-REFLECTIVE MATERIAL.
- NO. 90-A CONTRACT TIME.
- NO. 99-A CONCRETE SURFACE FINISH.
- NO. 101 WELDED STEEL BRIDGES.
- NO. 102 COARSE AGGREGATE
- NO. 108-B LATEX CONCRETE OVERLAY
- NO. 111 TERMINATION OF CONTRACT IN PUBLIC INTEREST
- NO. 112-A PORTLAND CEMENT CONCRETE OVERLAY

SPECIAL NOTES

- FOR MAINTENANCE OF TRAFFIC ON SURFACE STREETS
- FOR DECK OVERLAY
- FOR RUMBLE STRIPS
- FOR RAISED PAVEMENT MARKERS

REFERENCES

STANDARD DRAWINGS LISTED BELOW ARE CURRENT EDITION AND ARE TO BE USED WITH THESE PLANS

BGX-004 BGB-001-01 TSC-240
BGX-005 BGB-002-01 TSC-241
BHA-003 BGB-003-01 TSC-245
BHA-004 BJE-001-01 TSC-246
BPC-005-01 BJE-002-01 TSC-210
BJJ-003-01 TSC-200-01
TSC-260-02

Publications

Kentucky Bureau of Highways - Standard Specifications, current edition.

Kentucky Bureau of Highways - Uniform Traffic Control Devices, current edition.

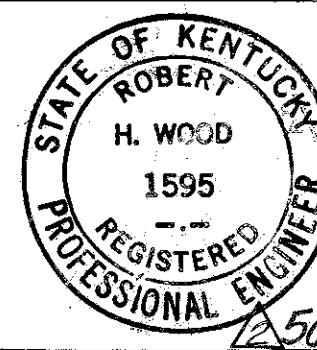
Standard Highway Signs - Federal Highway Administration, current edition.

AWS D20-69 "Specifications for Welded Highway and Railway Bridges.

TITLE SHEET

PREPARED & SUBMITTED BY:
HAZELET & ERDAL
CONSULTING ENGINEERS
FILE NO. 955-00

By Robert H. Wood
Date May 30, 1975



164 E.B. OVER PRESTON STREET

SHEET 1 OF 5

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF

JEFFERSON
LOUISVILLE-LEXINGTON
ROAD

STATION 267+00

CONSTRUCTION PROJECT NO.
I 64-2(100) 5

P.E. PROJECT NO. 164-2(92)

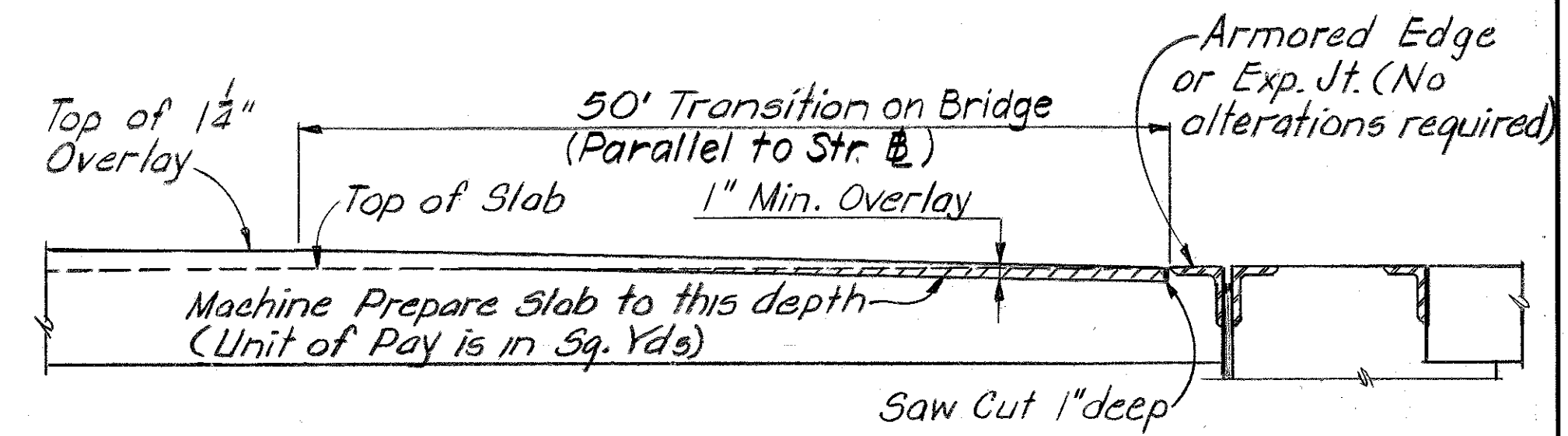
MAINTENANCE PROJECT NO.

DRAWING NO.

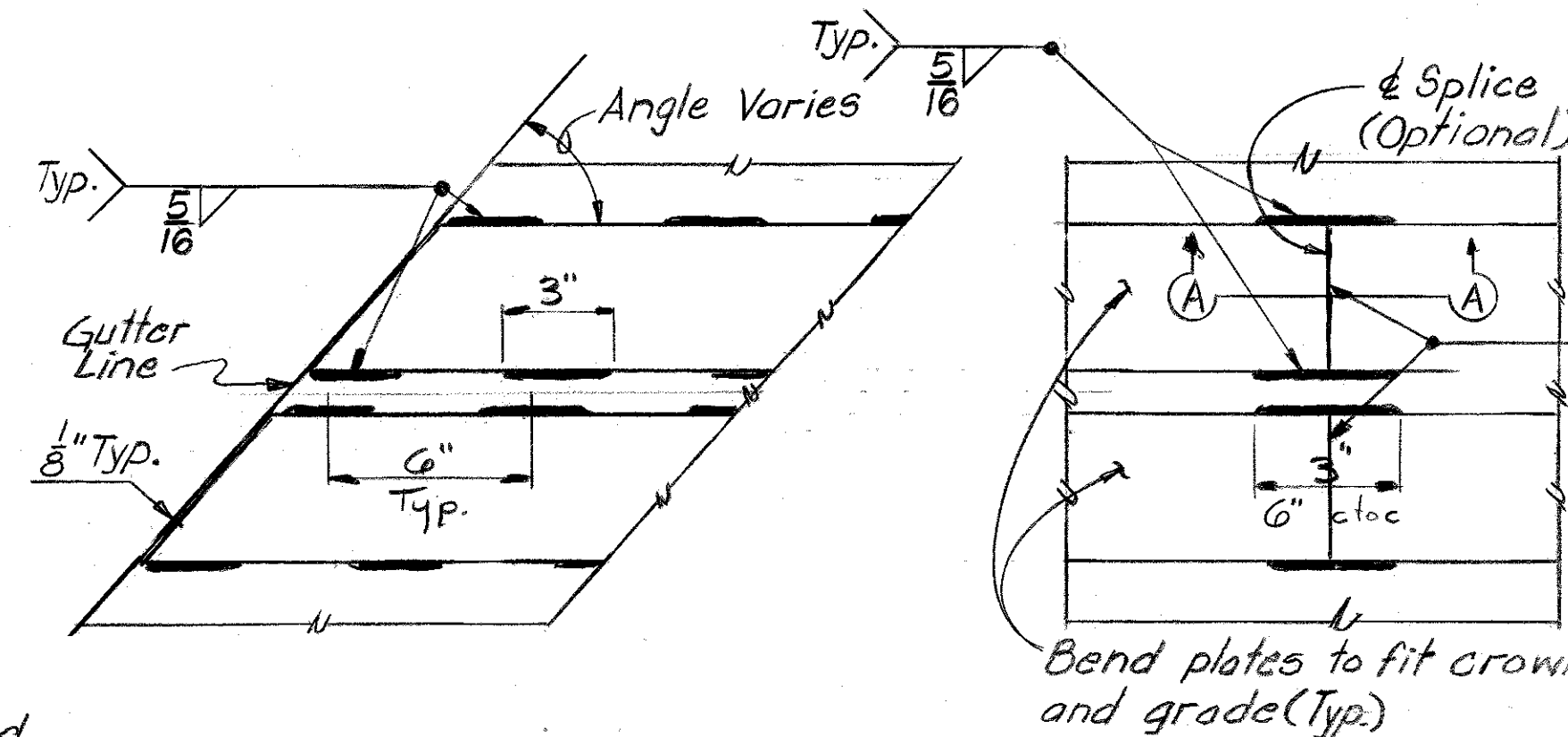
19238

For Bill of Incidental Materials See Sheets 7,8,22,&24.

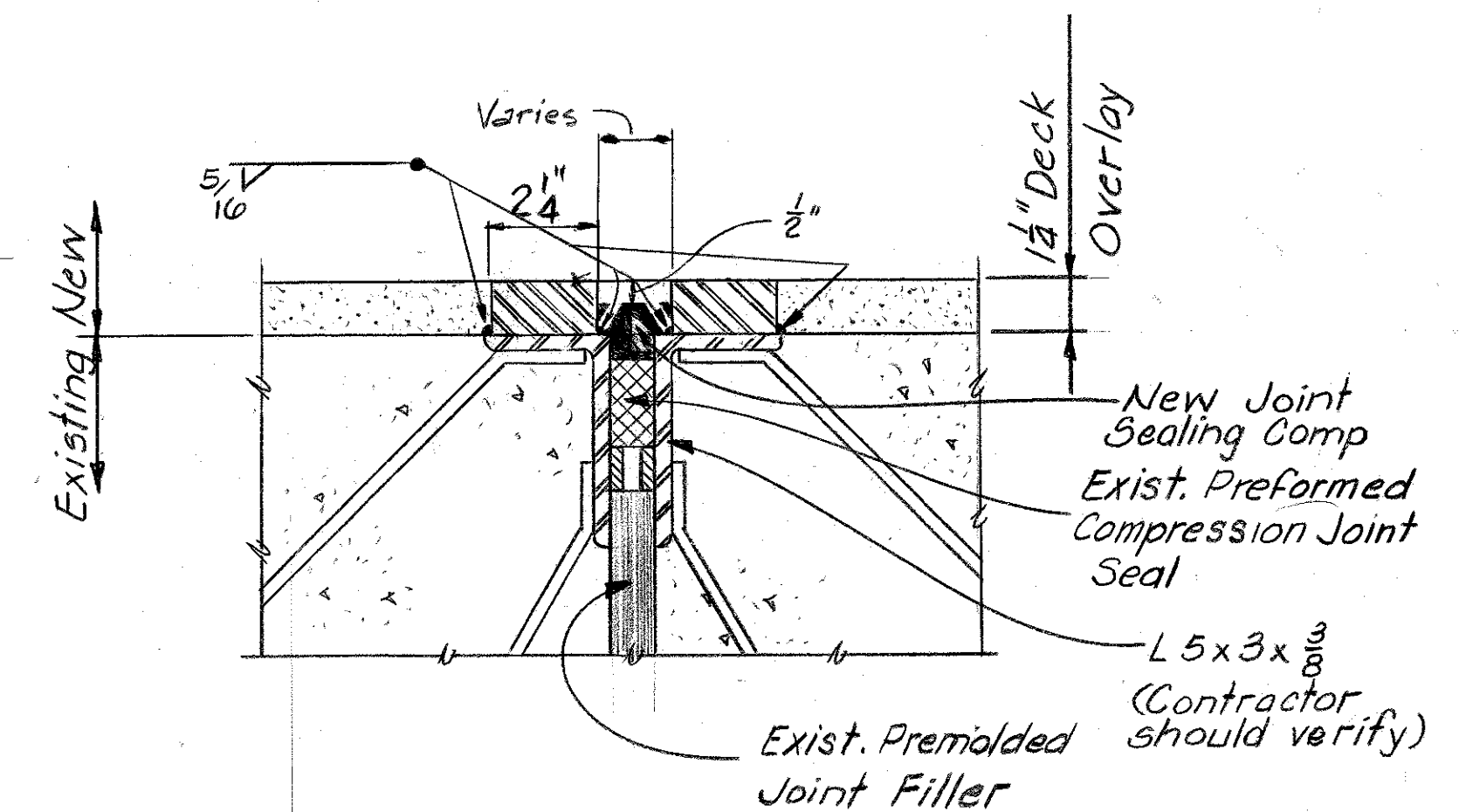
DESIGNED BY	CHECKED BY	DATE	REVISED STEEL QUANTITIES	DATE	JBW	DATE	9-5-75
REVIEWED BY	CHECKED BY	DATE	REVISED CONCRETE QUANTITIES	DATE	✓	DATE	5-12-76
REVIEWED BY	CHECKED BY	DATE	REVISED	DATE		DATE	



PLAN OF DECK OVERLAY @ 22ND ST.



PLAN OF ARMORED EDGE



SECTION THRU ARMORED EDGE

SECTION THRU CURB & GUTTER

Epoxy Sand Slurry is to be applied on both sides of bridge for the entire length of the bridge as specified above after all outlined work has been completed. Care shall be taken to mask all joints. Epoxy Sand Slurry shall be done in accordance with Special Provisions No 48, current edition. All Surfaces to be in contact with the epoxy shall be sandblasted.

Note A: The surface is to be sandblasted until existing concrete is absolutely clean and is to have a grout bond coat scrubbed into the surface as specified in Special Provisions 108 or 112, current edition. (Typical for all curbs with or without Machine Preparation). Entire Bridge Deck to receive 1 1/2" Deck Overlay, unless noted.

- ① Type I Deck Drain (Typ @ Median Curb).
- ② Type II Deck Drain (Typ @ Fascia Curb).

See Sheet 49 for Deck Drain modification.

Place 1 1/2" minimum Deck Overlay plus additional required to fill machine prepared slab or patches. See Special Notes for "Resurfacing Bridges".

No longitudinal Construction Joint will be permitted in new overlay except at Lane lines. Unless permitted by the Engineer the transverse construction joint shall be limited to those at the expansion dams or armored edges.

*No alterations required

For Details of Expansion Dam Type A, see Sheet 48

The expansion dam and armored edge dimensions are taken from the bridge design drawings. The Contractor shall obtain the plate lengths and verify the plate widths by field measurements.

All Expansion Dam and Armored Edge Steel shall be A242 unless noted.

ESTIMATE OF QUANTITIES							887.4
ITEM	UNIT	AMOUNT				TOTAL	
		22 ND St.	17 TH 13 TH St.	7 TH 2 ND St.	2 ND -Preston		
DECK OVERLAY	Cu. Yd.	184.3	719.7	956.2	816.7	2676.9	
BLAST CLEANING	Sq. Yd.	5,307	20,728	27,538	23,523	77,096	
MACHINE PREPARE SLAB	Sq. Yd.	1,161	918	590	9,611	12,280	
EPOXY SAND SLURRY	Batch	28	116	153	102	399	
STEEL	Lbs.	25,050	15,360	37,480	22,796	100,686	
RUMBLE STRIPS	Lin. Ft.	-	-	-	1,750	1,750	
RAISED PVMT. MARKERS	Each	-	-	-	146	146	
			24,590	51,570	38,097	139,307	

BILL OF INCIDENTAL MATERIALS*						
ITEM	UNIT	AMOUNT				TOTAL
		22nd St.	17th-13th	7th-2nd	2nd-Preston	
JOINT SEALING COMPOUND	Lin Ft.	878	89	355	119	1,441

* Items incidental to "Class 'AA' Concrete"

DECK OVERLAY - 22ND ST.

-2747.6 19238

SHEET NO 44

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

FRANKFORT
COUNTY OF

JEFFERSON

LOUISVILLE-LEXINGTON

STATION 267 + 00
CONSTRUCTION PROJECT

P. E. PROJECT NO. I64-2(92)

DRAWING NO.
7238

SPECIFICATIONS: The Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, current edition with revisions, shall apply to this project.

DESIGN LOAD: These bridge alterations are designed for HS 20 loading as specified in 1973 AASHD Specifications or alternate loading of two 24-kip axles spaced four feet apart, whichever produces the greater stress. This bridge is designed for a wind load based on a wind velocity of 84 m.p.h.

DESIGN STRESSES: For reinforced concrete use the following:

Class A	Class AA
$f_s = 20,000$ psi $f_c = 1,200$ psi $f'_c = 3,000$ psi u (for embed.) = 200 psi u (for $\Sigma\phi$) = 300 psi $n = 10$	$f_s = 20,000$ psi $f_c = 1,600$ psi for other than slabs $f_c = 1,200$ psi for deck slabs $f'_c = 4,000$ psi u (for embed.) = 200 psi u (for $\Sigma\phi$) = 300 psi $n = 8$
Class D Concrete $f'_c = 3,000$ psi	

For Structural Steel use the following:

A36	$f = 20,000$ psi	
A242	Over 1-1/2" to 4" Incl.	$f_s = 23,000$
A242	Over 3/4" to 1-1/2" Incl.	$f_s = 25,000$
A242	3/4" and under	$f_s = 27,000$

MATERIALS: ASTM Specifications, current edition, as designated below, shall govern materials furnished.

A615-74a	Deformed Billet-Steel Bars for Concrete Reinforcement - Grade 60
A616-72	Rail-Steel Deformed Bars for Concrete Reinforcement - Grade 50 or 60
A617-74	Axle-Steel Deformed Bars for Concrete Reinforcement - Grade 60
A36-74	Structural Steel, 36,000 psi Minimum Yield
A242-74	High Strength Low-Alloy Structural Steel
A325-74	High Strength Bolts, Nuts and Washers
A307-74	Carbon Steel Machine Bolts, Nuts and Tap Bolts
B29-55(1971)	Sheet Lead and Pig Lead
A108-73	Stud Shear Connectors, Grade 1015
A108-73	Pintles, Grade 1016 to 1030

Welding Electrodes shall conform to the appropriate AWS Specifications.

CONCRETE: Class "AA" concrete in accordance with the requirements of Special Provision No. 35, current edition, is to be used throughout the superstructure including parapet, and is to be used in the portions of the substructure above the bearing seats. Class "A" concrete is to be used in the substructure below the bearing seats, except in the concrete piles. Class "D" concrete is to be used in the concrete piles.

SET RETARDING ADMIXTURE FOR CONCRETE: An approved admixture shall be added to the concrete for the bridge floor slabs to delay the initial set of the concrete so as to permit the placement and finishing of concrete in all spans in a single continuous operation for each unit. The admixture and its use shall conform to the requirements of Special Provision No. 36, current edition. The amount of delay will be dependent on the quantity of admixture and the quantity shall be carefully determined on the basis of temperature, relative humidity, wind conditions and required placing time. The retarding action shall delay the initial set in each span until after the next adjacent span in the bridge has been placed. The Contractor shall secure the Engineer's approval of the quantities of admixture to be used for each placement.

SLAB FORMS: Stay-In-Place forms will not be permitted for concrete bridge floors.

CONCRETE SURFACE FINISH: All concrete surfaces shall be finished according to the requirements of Special Provision No. 99, current edition.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are clear distances unless otherwise shown. Spacing of bars is from center to center of bars.

BEVELED EDGES: All exposed edges shall be beveled 7/8" unless otherwise shown.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal measurements. Dimensions and elevations shown are theoretical, based on original construction plans and may have to be adjusted in the field to match the existing structure. The Contractor shall verify dimensions with field measurements prior to ordering materials or fabricating structural steel.

ANCHOR BOLT HOLES: Holes of depth and dimensions shown shall be drilled for anchor bolts, after base plates are properly set, by the superstructure contractor who shall be responsible for keeping holes dry in freezing weather. After base plates are properly set, anchor bolt holes drilled, and anchor bolts are placed in drilled holes, molten lead shall be poured in holes and packed until holes are completely filled flush to top of base plates. At the time of setting, anchor bolts are to be heated to a blue heat to assure free flow of lead to the bottom of the hole. The cost of drilling anchor bolt holes, furnishing lead, and filling holes with molten lead shall be incidental to and included in the lump sum bid for structural steel.

PAYMENT FOR STRUCTURAL STEEL: The Lump Sum Bid for structural steel shall be full payment for all structural steel, bolts, washers, steel pins, lead plates, molten lead, welding and welding materials, paint and all labor and materials necessary to erect the steel in accordance with the Plans and Specifications. The approximate weight of structural steel shown in the Estimate of Quantities does not include overrun or weld material.

CONNECTIONS: Unless otherwise provided on the plans, all connections shall be 7/8" diameter high strength bolts. Open holes shall be 15/16" diameter. All joints are designed as friction-type connections. Tightening shall be done with properly calibrated wrenches.

CAMBER: The beams of spans composed of wideflange members shall be cambered to mid-ordinates specified on the plans. If these beams do not conform to plan camber for the erected position, either an adjustment in depth of concrete haunch over the supporting members, or a reworking of the beam camber to meet plan camber and slab thickness will be required. If either type of adjustment is required, there will be no additional cost to the state. However, in no case shall the shear connectors be allowed to penetrate the slab less than two inches. Girder web plates shall be cut to provide for the camber of the girder. Provide for possible warpage due to extra heat in top flange by virtue of shear connectors. Girders which do not conform to plan camber and grade in the erected position shall be considered as requiring, at no additional cost to the state, either an adjustment in depth of the concrete slab haunch over the steel supporting members or a reworking of the girder camber to meet the plan grade and slab thickness. However, in no case shall the shear connectors be allowed to penetrate the slab less than two inches.

MILL TEST REPORTS: Notarized Mill test Reports shall be furnished in triplicate to the Department in accordance with Article 408.3.1 L4 of the Standard Specifications.

WELDING: All welding shall conform to AWS D2.0-69 "Specifications for Welded Highway and Railway Bridges", 8th Edition, of the American Welding Society with modifications and additions as stated on the Plans and Special Provision No. 101, current edition and the 1970 revision of AWS D2.0-69.

WELDING PROCEDURE: Qualification tests of all welding procedures shall be completed by the Contractor and approved by the Engineer prior to the final approval of the shop drawings and Welding Procedure and the start of the fabrication.

STUD WELDING: Studs shall be welded with approved welding equipment.

CLEANING AND PAINTING: Special Provision 80, current edition, applies to this project and contractors are hereby reminded that in accordance with the special provision ALL steel surfaces to be painted, including exposed surfaces of splice plates, shall be blast cleaned to a near-white condition in accordance with SSPC-SP10 immediately prior to being painted with the first coat of paint, regardless of whether the first coat is applied in the shop or in the field.

PAINTING BURNT AREAS: Areas of new or existing structural steel on which paint has been damaged with weld burns or other means shall be cleaned and spot painted in accordance with the procedure for painting new structural steel. The cost of this touch-up painting is to be included in the lump sum bid for structural steel.

CLEANING EXISTING STEEL: Areas of existing steel that are to be in contact with new steel, including areas under bolt heads, shall be cleaned of dirt, rust, paint, and other foreign matter, before installing the new steel. The cost of this cleaning is to be included in the lump sum bid for structural steel.

OPTIONAL TYPES OF SHEAR CONNECTORS:

(a) The Contractor shall use one of the following options throughout this structure.

Option 1 - 3/4" x 4" studs

Option 2 - 4" channel @ 5.4#

(b) If the Contractor wishes to use other than 3/4" x 4" stud shear connectors shown on the plans, the proposed arrangement shall be submitted for approval with the shop plans.

PAYMENT FOR SHEAR CONNECTORS: "The Lump Sum Bid" for shear connectors shall be full payment for all shear connectors, welding and welding material, and materials necessary to field weld or shop weld the shear connectors in place according to the plans and specifications.

SLOPE PROTECTION: Slope protection shall be 4" concrete slopewall in accordance with Standard Drawing BGX-004, current edition.

EXPANSION JOINT SEALING MATERIAL: See Special Provision No. 12, current edition, for requirements of joint sealing compound. The cost of this item is to be included in the unit price bid for Deck Overlay.

END BENT EXCAVATION AND BACKFILL: See Standard Drawing BGB-001, BGB-002, and BGB-003, current edition.

BLASTING: Blasting will not be allowed for removing existing concrete or excavation for new construction.

BURIED ELECTRICAL WIRES: The exact location of electrical wires for expressway lighting is not known. Work being done in locations where such wiring might exist should be done with caution to prevent serious accidents.

EXISTING SLOPE PAVING: Existing slope paving shall be removed, in areas Approved by the Engineer, where necessary for making structure excavation. The cost of this removal shall be included in the lump sum bid for "Removal of Existing Reinforced Concrete." Replacement in the areas mentioned above shall be 4" concrete slopewall and shall be paid for at the unit price bid for "Slope Protection". The Contractor shall exercise care to prevent damaging the existing slope paving in other areas and shall replace, entirely at his own expense, any areas damaged by careless operations.

FENCE: Existing fence at bridge ends is to be carefully removed and stored during construction. After new piers have been completed, the fence is to be replaced as directed by the engineer. The cost of this work is to be incidental to the project and no direct payment will be made.

ITEMS DAMAGED OR REMOVED: Any items (sidewalks, curbs, etc) damaged or removed to facilitate construction shall be replaced to original condition, as approved by the Engineer, by the Contractor. The cost of this work is to be incidental to the project and no direct payment will be made.

UTILITY LINES: The owners of utility lines shall be responsible for these lines and shall relocate the lines as shown on the plans or as directed by the Engineer. The Contractor shall cooperate with the utility owners during the construction work.

EXISTING UTILITIES: The Contractor is advised to exercise extreme caution in his operations in areas where plans indicate the presence of a gas line or other lines carrying hazardous material. The Contractor shall use all possible care in driving the piling and constructing the foundations in order that the existing utilities shall not be disturbed. Any utility disturbed or damaged by the Contractor's operations shall be replaced or repaired by the Contractor. Any replacement or repair necessary shall be at the Contractor's expense with no extra cost to the Department and such work shall include excavation, sheeting, materials, labor and all incidentals necessary to complete the work to the satisfaction of the Engineer.

EXISTING REINFORCING STEEL: Where indicated on the plans the existing reinforcing steel shall be exposed and cleaned of concrete, excessive rust, and other foreign matter. It shall be straightened and bent to lap, as indicated on the plans, with the new bars. Laps in excess of 40 bar diameters are not required and the excess may be cut off. The cost of cutting, bending, and cleaning existing reinforcing steel is to be included in the lump sum bid for "Removal of Existing Reinforced Concrete."

BONDING NEW CONCRETE TO OLD CONCRETE: All new concrete shall be bonded to the old concrete where noted on the plans with a two-component epoxy resin conforming to Special Provision No. 73, current edition, (Epoxy Grout and Bond Coat). The cost of this work, including all labor, tools, and materials is to be included in the unit price bid for the class of concrete being bonded.

DRILLING HOLES IN CONCRETE: The cost of drilling holes in existing concrete and grouting dowel bars is to be included in the unit price bid for the class of concrete adjacent to the dowel bars. This work is to be performed in accordance with Special Provision No. 73, current edition.

DAMAGE TO THE STRUCTURE: The Contractor is responsible for any and all damage to the structure during reconstruction, even to the replacement of the entire structure and removal of the fallen structure at his expense, should it be allowed to fall for any cause whatsoever.

ELEVATION DATUM: The Contractor shall establish an elevation bench mark in a convenient location that will not be disturbed by the construction. It shall be durable and adequately identified as to be useful for the entire duration of the project. The elevation shall be based on the values shown on these plans for the bearing seats of the existing structure. The end bearing seat from each existing end bent and pier shall be considered before assigning the elevation. The bench mark shall be acceptable to the Engineer and its location and elevation and the measurements used to establish the elevation furnished to him in writing.

ALIGNMENT, STATIONING, PROFILE, AND CROSS SLOPE: Centerline alignment, bridge stationing, profile grade, and cross slope data shown on these plans is theoretical and based on original construction plans. New construction shall be adjusted as required to match the existing structure as shown.

REMOVAL OF EXISTING REINFORCED CONCRETE: This work includes removing portions of reinforced concrete in wing walls, wing wall footings, end bents and bridge floor slabs, curbs. Concrete removal shall be done in such a manner that the edges of the remaining concrete shall be as square and straight as possible. Feather edges of remaining concrete or new concrete will not be allowed. Where called for on the plans or where directed by the Engineer saw cuts shall be made in the existing concrete to aid in securing square and straight edges. No additional payment will be made for saw cuts. Proper care shall be taken to protect and leave exposed existing reinforcing steel at locations indicated on the plans. Waste shall be removed from the Right of Way and disposed of in a place acceptable to the Engineer. This work shall be paid for at the contract lump sum bid for "Removal of Existing Reinforced Concrete" which payment shall be full compensation for removal and disposal of reinforced concrete and accessory items and all labor, equipment, tools, and incidentals necessary to complete the work.

CONTINUOUS STEEL GIRDERS - NEW CONSTRUCTION: The longitudinal steel girders are designed as continuous spans between the pier girders for steel dead load, as continuous steel girders under weight of the concrete slab, and as continuous girders composite in positive moment for live load and the weights of curbs, plinth, railing, future wearing surface, etc.

STRUCTURAL STEEL: All new structural steel shall be ASTM A36, except where indicated. All the flange and web material supplied for the longitudinal plate girders, shall meet the longitudinal Charpy V-notch toughness test, applicable to Group 2 minimum service temperature from minus 1° F, to minus 30° F, in accordance with the following listing: For A36 steel (up to 4") - 15 ft. lbs. @ 40° F. Sampling and testing procedures shall be in accordance with ASTM A873, current edition, utilizing (H) frequency testing.

SHOP ASSEMBLY: General reaming of holes for each bolted splice connection of each longitudinal girder line shall be required. Each continuous longitudinal girder unit shall be progressively shop assembled with at least three contiguous shop sections adjusted to line, elevations, camber and fit for drilling or reaming. At least one shop section shall be added at the advancing end of the assembly before any shop section is removed from the rearward end so that the assembled portion of the structure is never less than three contiguous shop sections. Other major bolted connections to the longitudinal girders shall be drilled or reamed in the shop with connecting parts assembled or shall be reamed to metal template without assembly. Girder Sections shall remain assembled for inspection by the Bureau of Highways Inspector and are to be matchmarked while assembled. Connections for the cross frames, diaphragms, expansion joints and other minor members may be punched or drilled full size without assembly subject to the requirements in the specifications for general reaming.

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

FRANKFORT

COUNTY OF

JEFFERSON

LOUISVILLE-LEXINGTON

ROAD

P. E. PROJECT NO. 164-2(92)

STATION 267+00

CONSTRUCTION PROJECT NO.

MAINTENANCE PROJECT NO.

DRAWING NO.

19238

UPDATE DATE
LETTING DATE

DESIGNED BY	CHECKED BY	DATE	7/75	
			REVISED	DATE
			RETRACED BY	DATE
			DATE	DATE
Δ Indicates Revision for GM Barrier				

STRUCTURAL STEEL ERECTION: The existing girder "G" is designed as simple spans between the pier girders for steel dead load, as a continuous steel girder under weight of the concrete slab, and as a continuous girder composite in positive moment for live load and the weights at curb, plinth, railing, future wearing surface, etc. Where girder "G" is continuous thru the pier girder the flanges are designed to take the entire moment and the web the entire shear.

In all cases where girder "G" is disconnected from the pier girders, girder "G" must be jacked in such a manner as to remove the load from the connecting bolts. It is anticipated that in order to accomplish the above, girder "G" will have to be jacked at the pier girder and in some cases the adjacent spans. At all jacking points, provisions for jacking against the girder shall be provided.

The jacking procedure and details for reinforcing the girder at the jacking point must be approved by the Engineer prior to jacking.

FIELD WELDING ON EXISTING STRUCTURE: All field welding on existing structure should be indicated on the Shop Drawings along with the method of support where necessary. No other welding of any nature will be permitted on the load carrying members of the bridge without written consent of the Director, Division of Bridges, or his authorized representative, and then only in the manner and at the locations designated in the authorization.

FOUNDATION PILE LOADS: Piles are designed for a maximum load of 60 tons per pile. This maximum is for Group 1 loads with increases allowed for other loading groups in accordance with AASHTO Article 1.2.22 current edition. This load and the pile loads for End Bent 101E are the same as for the existing structure.

TYPES OF PILES: The Contractor shall use 14" Cast-In-Place Concrete Piles Standard Drawing BPC-005-01.

PILING: Piling shall be driven to the maximum tip elevation (minimum penetration) shown on the Pile Record Sheet and determined from driving records for the existing structure, but the driving resistance for the last foot of penetration shall be not less than that required by the Specifications for a 75 ton load.

DRIVING PILES FOR END BENT: Cored pilot holes thru the end bent embankment will be required for starting piles. The cost of this work is to be included in the unit bid per linear foot for driving piles.

EXISTING APPURTENANCE: Existing  Light Standards and Luminaires shall be removed without damage, preserved and salvaged for reuse on the new plinth. All anchor bolts for the above relocated items shall be new.

SHOP DRAWINGS: Complete shop drawings in accordance with Article 408.3.1A.1 of the Standard Specifications shall be submitted to the Bridge Engineer for approval.

SHEETING FOR STRUCTURE EXCAVATION: Steel sheet piling to retain the earth is required at Pier 125A, 129A and 129B footing excavation. The cost of materials and labor is to be incidental to and included in the unit price bid for "Structure Excavation, Common."

CLEARANCE: All falsework, bracing or forms shall have a minimum vertical clearance of 20 feet above the top of the highest rail, and a minimum horizontal clearance of 8 feet measured perpendicular to the centerline of the nearest track.

CONSTRUCTION NOTE: The bridge being widened is over Louisville and Nashville Railroad Company tracks. The Contractor shall arrange to do the work without any interference to the operation of the Louisville and Nashville Railroad Company tracks at any time unless the Contractor has made arrangements with the Railroad Company. The Contractor shall arrange with the Railroad Company to provide flag protection at the Contractor's expense. Plans for any temporary supports adjacent to or over a railroad track shall be submitted to the Bureau of Highways and the Railroad Company for approval. The Contractor's attention is referred to Article 1.7.15 and 1.7.18 of the Specifications.

TEMPORARY SUPPORTS: Temporary supports or shoring will not be permitted under girders when pouring the concrete floor slab or when taking "Top of Beam" elevations.

DRAIN DETAILS: Foundry Note - All drains shall be gray iron castings, ASTM A48, current edition, Class 30A. Report of Field Inspection of Castings, current form, shall be submitted to the Division of Materials

The drain pipe shall be 6" standard weight in accordance with Article 411.3.5 of the Specifications.

Lump sum bid for "Deck Drainage" shall be full payment for the new drain casting, relocating the existing drain casting, all necessary 6" pipe and materials including weld material and welding, brackets, pipe clamps and hangers, fittings, connections, hardware, tools, paint and painting equipment, labor and all incidentals necessary to complete the work as shown on the drain details for deck drains and expansion dams.

CURING DECK CONCRETE: The deck concrete shall be cured as described in Article 403.3.7 of the Standard Specifications, except earth or straw shall not be used. Membrane Curing Compound will not be allowed.

INCIDENTAL MATERIAL: Quantities shown in Bill of Incidental Material are approximate only and the Contractor is responsible for furnishing enough material to complete the work in accordance with the Plans and Specifications.

ALTERNATE TYPES OF CONCRETE OVERLAY: The Contractor shall use, at his option, one of the following alternates throughout the project.

Alternate A - Concrete Overlay, Latex, in accordance with Special Provision No. 108, current edition.

Alternate B - Concrete Overlay, Portland Cement, in accordance with Special Provision No. 112, current edition.

SAFETY NETS: Safety nets will be required during superstructure construction. The nets shall conform to the Occupational Safety and Health Act requirements.

SURFACE FINISH OF CLASS "AA" CONCRETE IN THE BRIDGE DECK: The deck of the new structure is to receive an overlay, therefore, the burlap drag finish specified by Article 411.3.34-5 of the Standard Specifications for Road and Bridge Construction, 1965 Edition, is eliminated, nor will any surface texturing beyond 10-ft. straightedge (Article 411.3.3-H.4.) be permitted.

The contractor is cautioned that the proposed deck overlays are very sensitive to oils, grease, paints, waxes and similar substances and if these substances are deposited on the deck, he will be responsible for their complete removal which may include removal and replacement of the affected concrete to the depth the substances have penetrated. Any expense of this removal will be borne by the Contractor.

CURING OF CLASS "AA" CONCRETE IN THE BRIDGE DECK: The deck of the new structure is to receive an overlay, therefore, membrane curing as specified in Special Provision No. 30-B is not to be applied to the bridge deck. The concrete deck, including portions of the curbs and/or barrier walls designated, must be cured in conformity with Article 403.3.7 of the Standard Specifications for Road and Bridge Construction, 1965 Edition, with the following additional requirements:

1. No curing compound, Styrene Butadiene, linseed oil, masonry coating, form oil or similar substances will be permitted on the Class "AA" Concrete deck surface nor for 6" vertically above the Class "AA" Concrete gutter line of the interior faces of curbs.

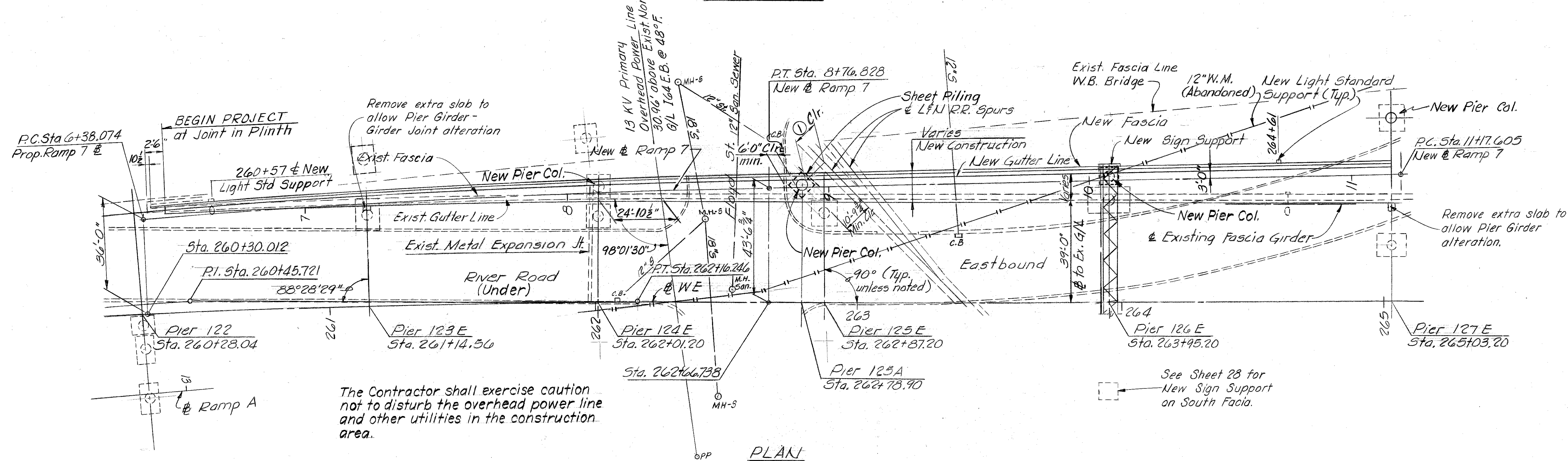
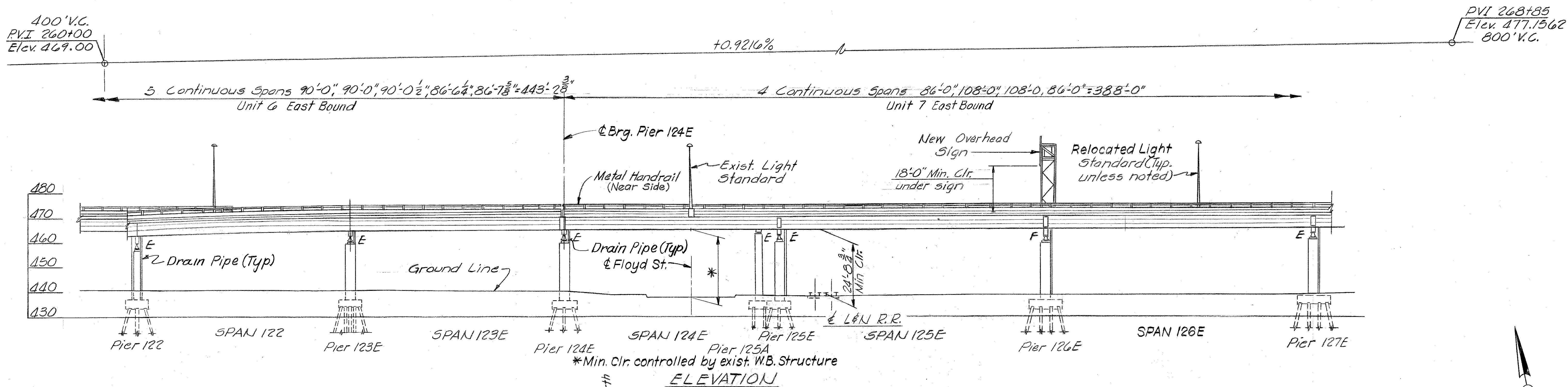
2. As soon after final finish of the concrete as possible one thickness of pre-wetted burlap must be applied and kept moist by a fog spray application of water. This one thickness of burlap must be followed, as soon as is possible, without damaging the surface of the concrete, with an additional layer of wet burlap, cotton mats, etc., as required by Article 403.3.7.

3. The required initial layer of burlap shall be pre-wetted by complete saturation with water except that it shall be squeezed free of excess water that will damage the concrete surface by dripping. The pre-wetted burlap must be applied from work bridges and walkways in a manner that will preclude workmen or tools damaging the fresh concrete surface. The pre-wetted burlap must be applied in a manner that will prevent it from being dragged, untangled, or unrolled on the fresh concrete surface and damaging it.

GENERAL NOTES

SHEET 2A		
COMMONWEALTH OF KENTUCKY		
BUREAU OF HIGHWAYS		
FRANKFORT		
COUNTY OF		
JEFFERSON		
LOUISVILLE - LEXINGTON		
ROAD		
P. E. PROJECT NO. 164-2(92)		
STATION 267+00	CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.
		DRAWING NO. 19238

UPDATE DATE
LETTING DATE



CURVE DATA

WE	New Ramp 7
P.I. = 260+45.721	P.I. = 7+57.488
Δ = 5°07'09" Rt.	Δ = 3°30'49.9 Rt.
D = 1°30'	D = 1°28'18.3"
L = 341.278'	L = 238.753'
R = 3819.719'	R = 3893.029'
T = 170.752'	T = 119.414'

See Sheet 8 for Horizontal Alignment Detail

Designed for HS20A loading. New construction of superstructure and substructure is to alter bridge for the addition of New Ramps 7 and 5. For original Bridge Plans see Dwg. No. 15754. Elevations, dimensions, WE & stationing are from original Bridge Plans and are not field measurements.

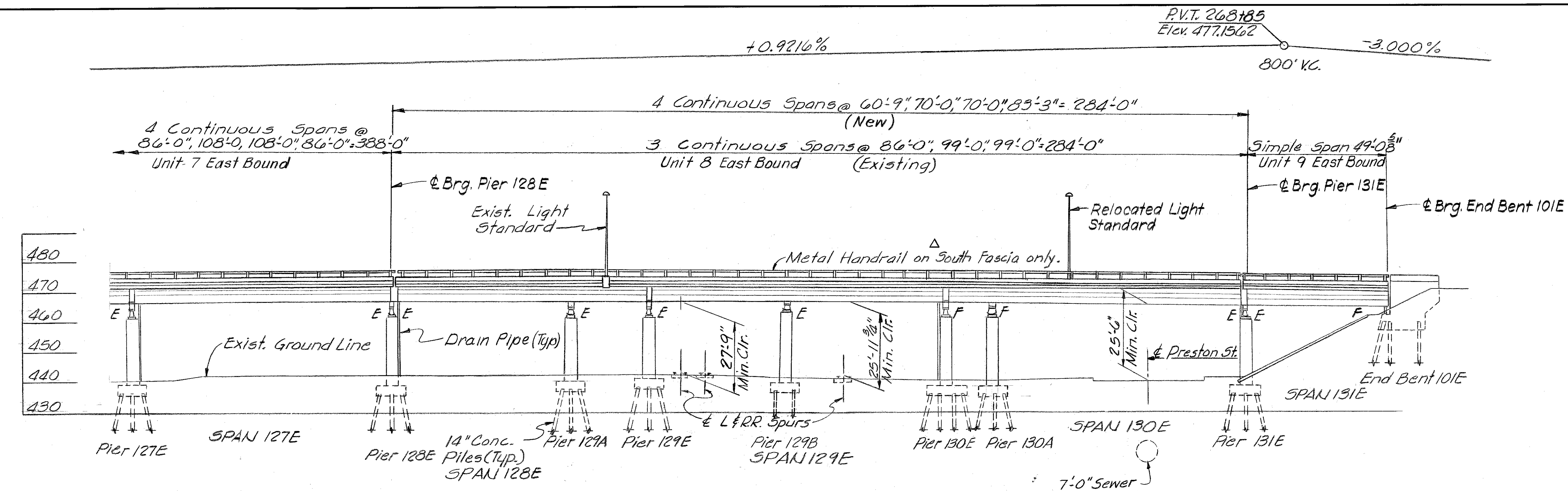
① 8'-6" plus allowance for overhang and tilt on curves (85' car, 60' between trucks and 14' high)

Approx. length of project = 892'

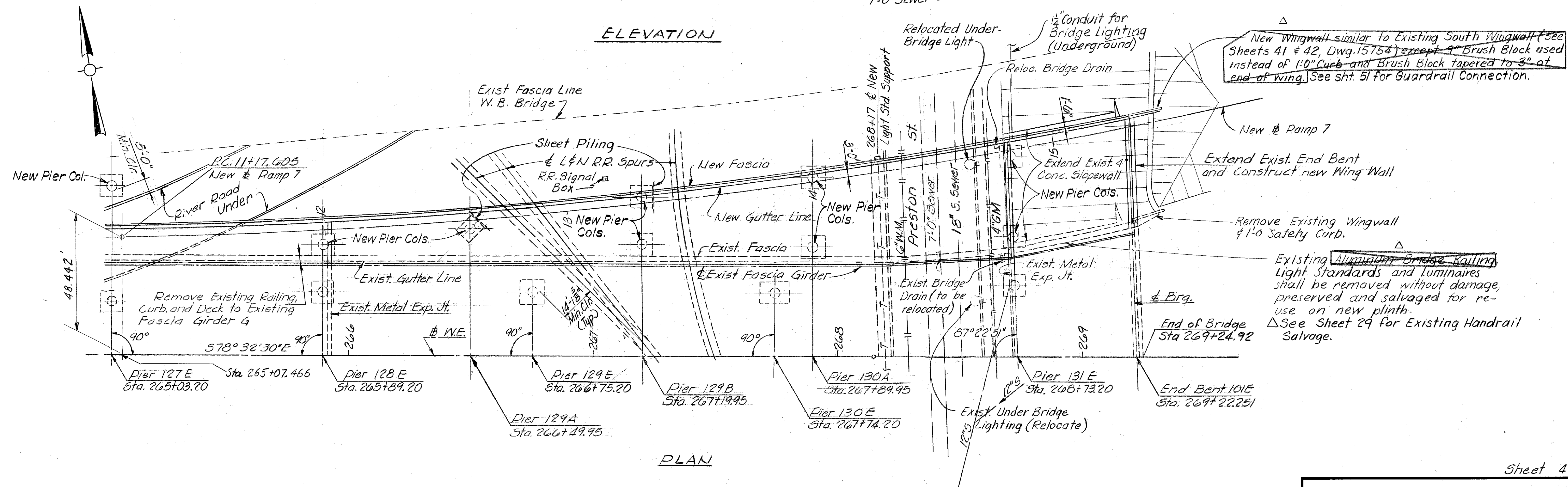
LAYOUT

COMMONWEALTH OF KENTUCKY BUREAU OF HIGHWAYS FRANKFORT COUNTY OF JEFFERSON LOUISVILLE-LEXINGTON ROAD		
STATION 267+00 CONSTRUCTION PROJECT NO.	P.E. PROJECT NO. 164-2(92) MAINTENANCE PROJECT NO.	DRAWING NO. 19238

Sheet 3



ELEVATION



RAMP 7
CURVE DATA

$P.I. = 14 + 25.204$
 $\Delta = 15^{\circ} 17' 19.1'' \text{ Lt.}$
 $D = 2^{\circ} 30'$
 $R = 2291.83'$
 $L = 611.54'$
 $T = 307.60'$

Sheet 4

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

FRANKFORT
COUNTY OF

JEFFERSON
LOUISVILLE-LEXINGTON

ROAD

STATION 267+00
CONSTRUCTION PROJECT

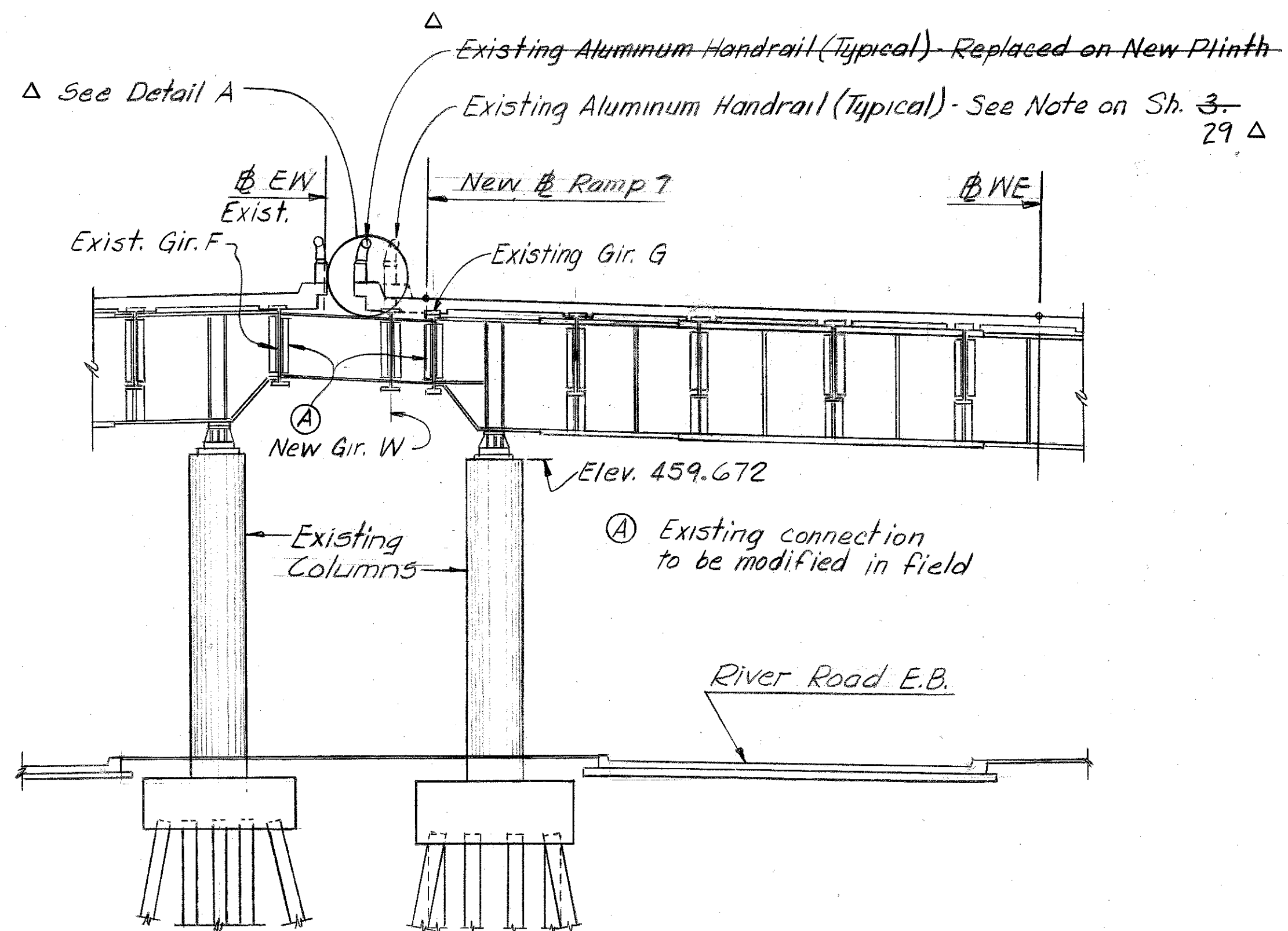
P. E. PROJECT NO. 164-2(92)

404	DRAWING
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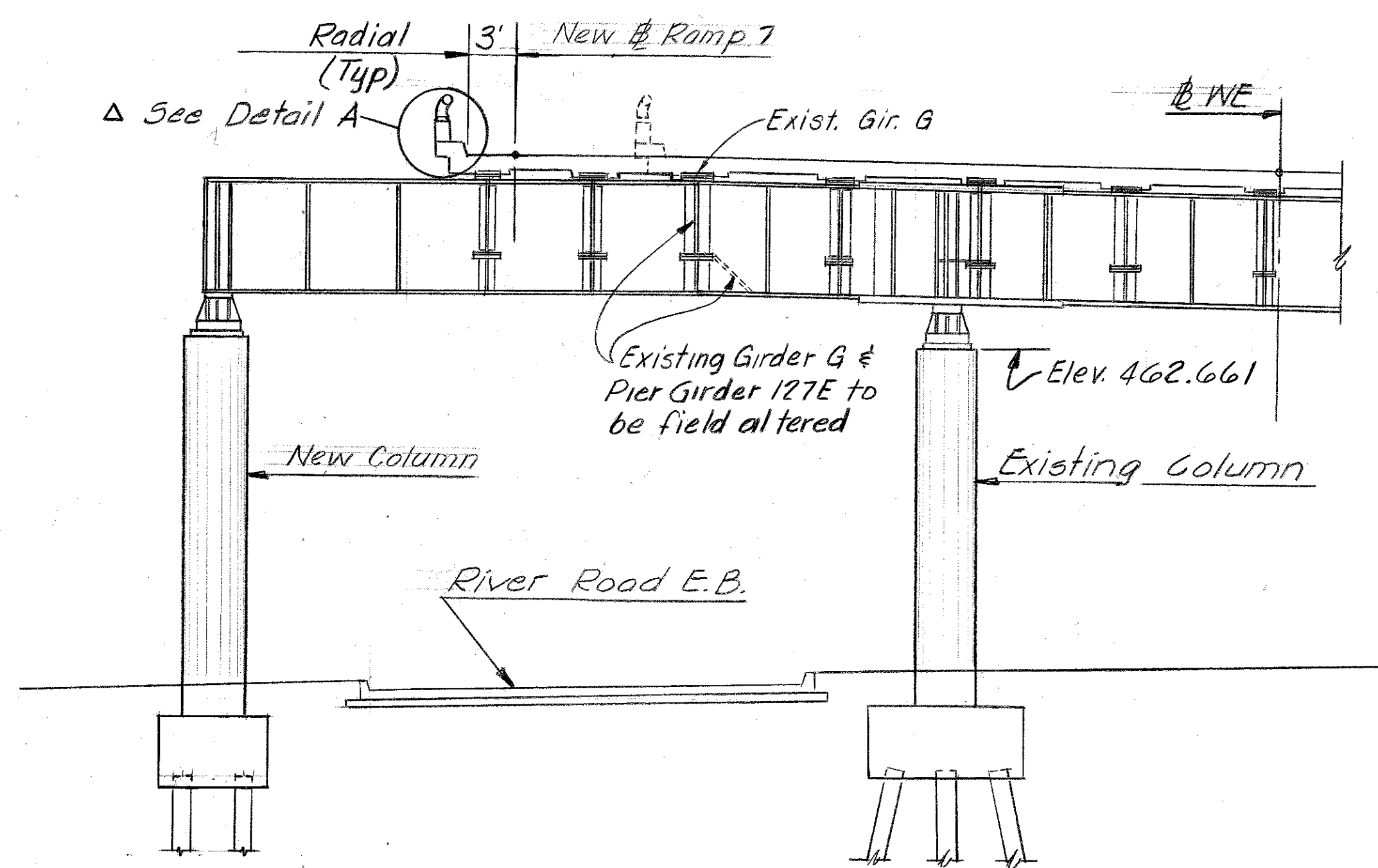
LAYOUT

UPDATE DATE
LETTING DATE

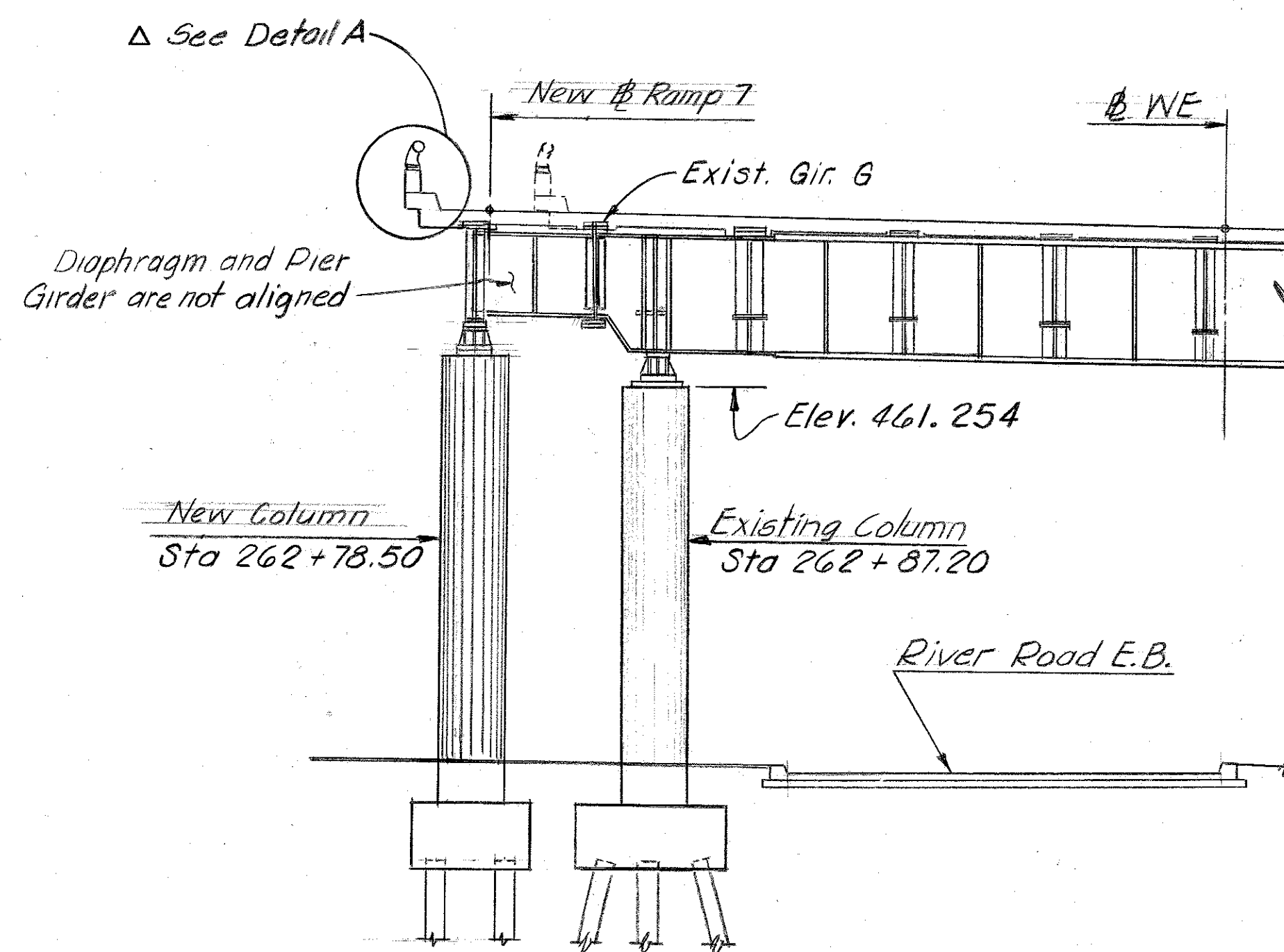
DESIGNED BY: RHMC
CHECKED BY: ELU
DATE: 3/74
REVISION: 1/75
DATE: 7/75



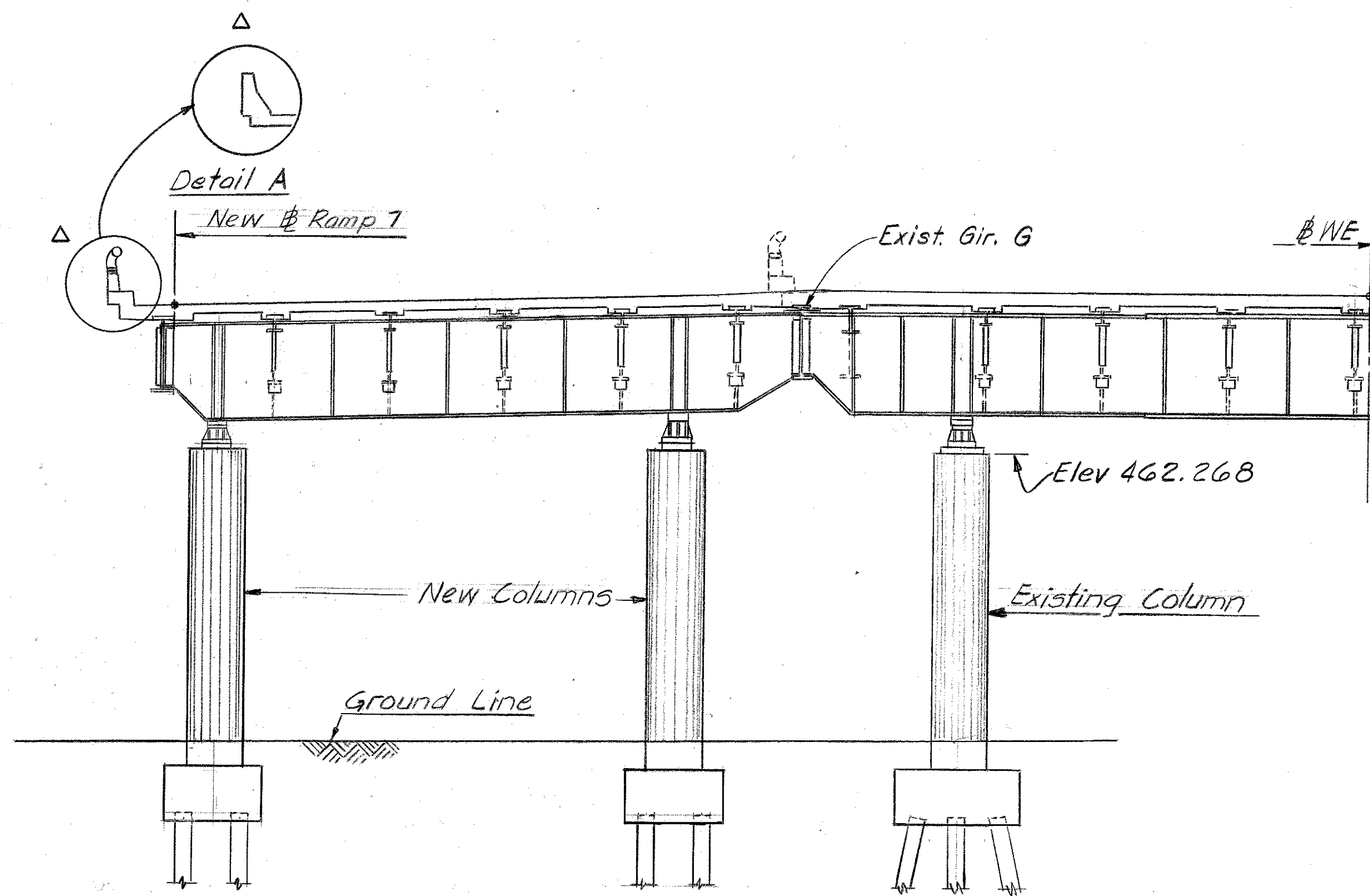
SECTION @ PIERS 123E & W



SECTION @ PIER 127E



SECTION @ PIER 125E



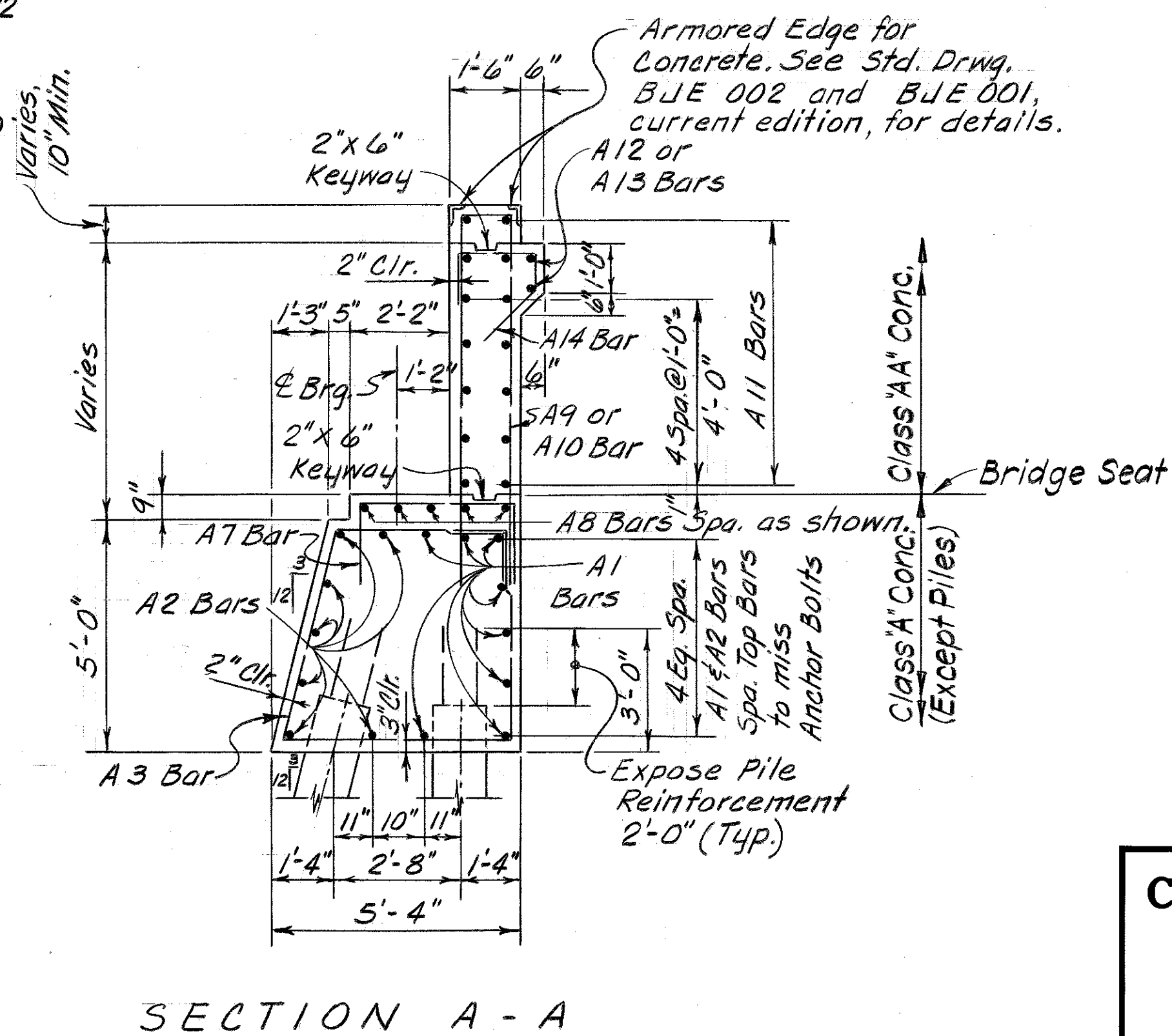
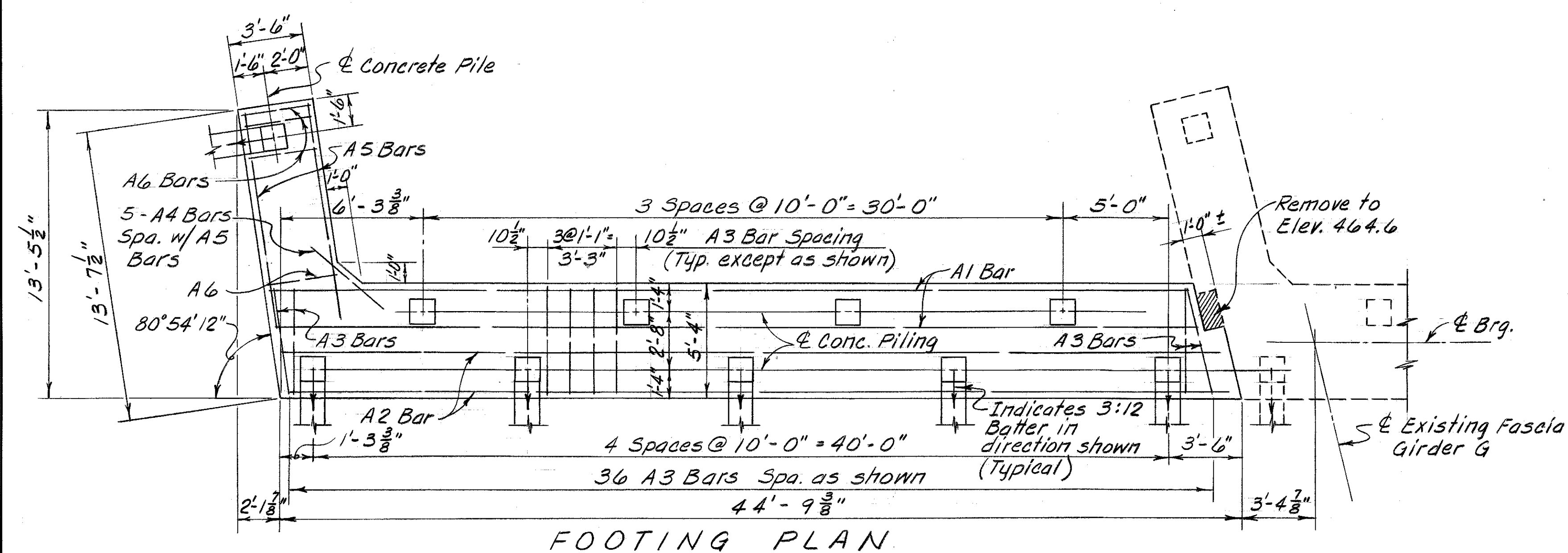
SECTION @ PIER 131E

TYPICAL SECTIONS

Sheet 5

COMMONWEALTH OF KENTUCKY		
BUREAU OF HIGHWAYS		
FRANKFORT		
COUNTY OF		
JEFFERSON		
LOUISVILLE-LEXINGTON		
ROAD		
STATION 267+00	P. E. PROJECT NO. 164-2(92)	DRAWING NO.
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.	19238

DESIGNED BY <i>R.B.S</i>	CHECKED BY <i>PMF</i>	DATE <i>4-15</i>	REVISED	DATE <i>7/75</i>
DESIGNED BY <i>E.L.U.</i>	CHECKED BY <i>Bloch</i>	DATE <i>4-15</i>	REVISED	DATE
DESIGNED BY	CHECKED BY	DATE	REVISED	DATE



COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

FRANKFORT COUNTY OF

JEFFERSON

LOUISVILLE-LEXINGTON

ROAD

STATION 267+00

CONSTRUCTION PROJECT NO.

P. E. PROJECT NO. 164-2(92)

MAINTENANCE PROJECT NO.

DRAWING NO.

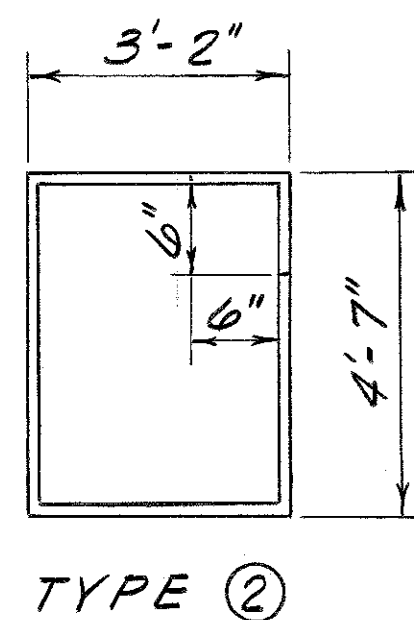
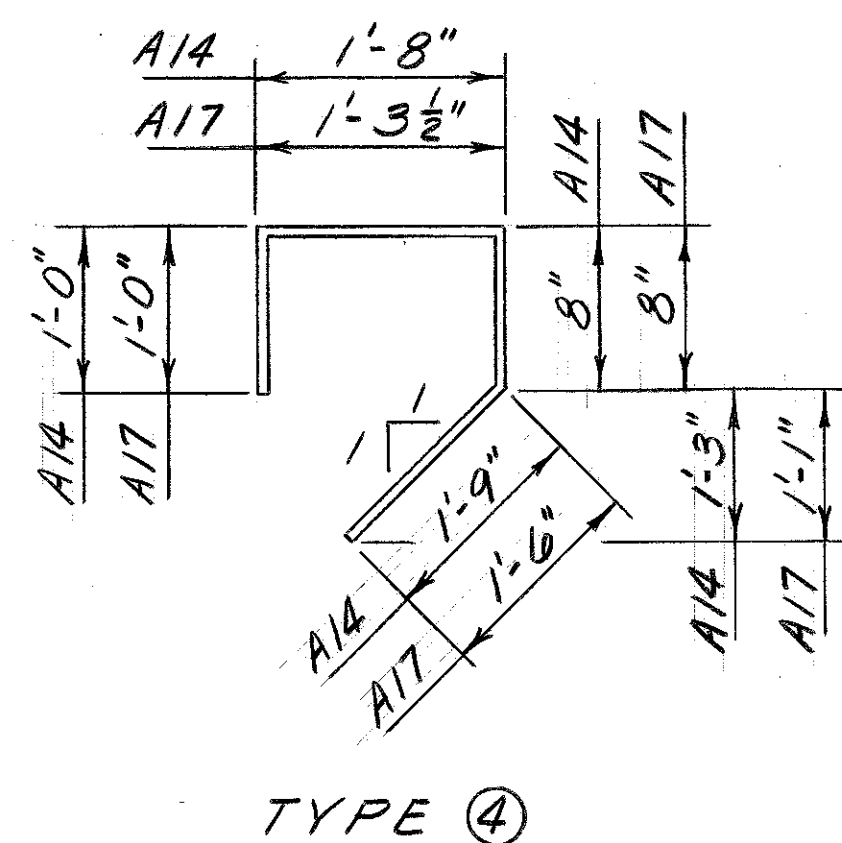
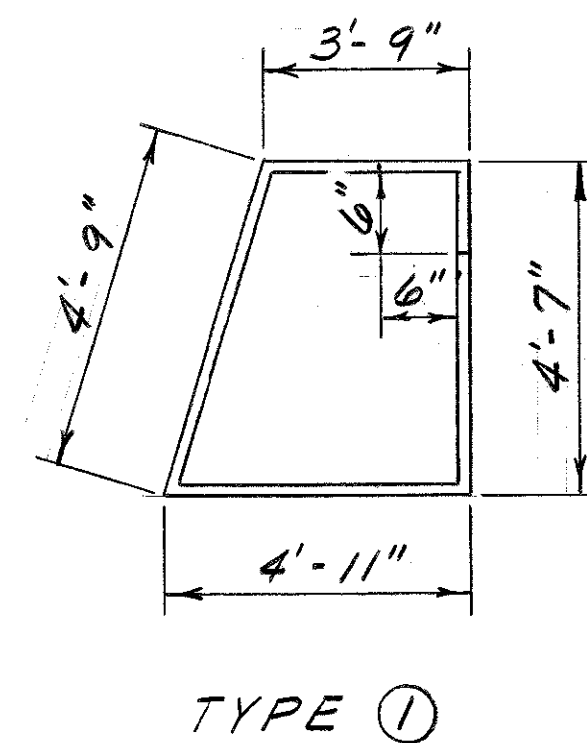
19238

The diagram shows a 6x6 grid of squares. The dimensions of the squares are as follows:

Row	Col 1	Col 2	Col 3	Col 4	Col 5	Col 6
1	4"	A7	1'-4"	6'-10"	6'-7"	1'-4"
2	3'-4"	A7	1'-4"	6'-10"	6'-7"	1'-4"
3	1'-2"	A9	1'-2"	6'-10"	6'-7"	1'-4"
4	1'-2"	A10	1'-2"	6'-10"	6'-7"	1'-4"
5	11"	A15	11"	6'-10"	6'-7"	1'-4"
6	11"	A16	11"	6'-10"	6'-7"	1'-4"
7	6'-2"	A18	6'-2"	6'-10"	6'-7"	1'-4"
8						

Below the grid, the types of the squares are listed:

Row	Col 1	Col 2	Col 3	Col 4	Col 5	Col 6
1	A7	A7	A10	A15	A16	A20
2	A7	A7	A10	A15	A16	A20
3	A9	A9	A10	A15	A16	A20
4	A9	A9	A10	A15	A16	A20
5	A15	A15	A15	A15	A16	A20
6	A16	A16	A16	A16	A16	A20
7	A18	A18	A18	A18	A18	A20
8						



Preformed Compression Joint Seal

5" x 3" x $\frac{3}{8}$ " Angle

5" x 3" x $\frac{3}{8}$ " Angle

10" Min. Print Seal

3"

1" Preformed Joint Filler

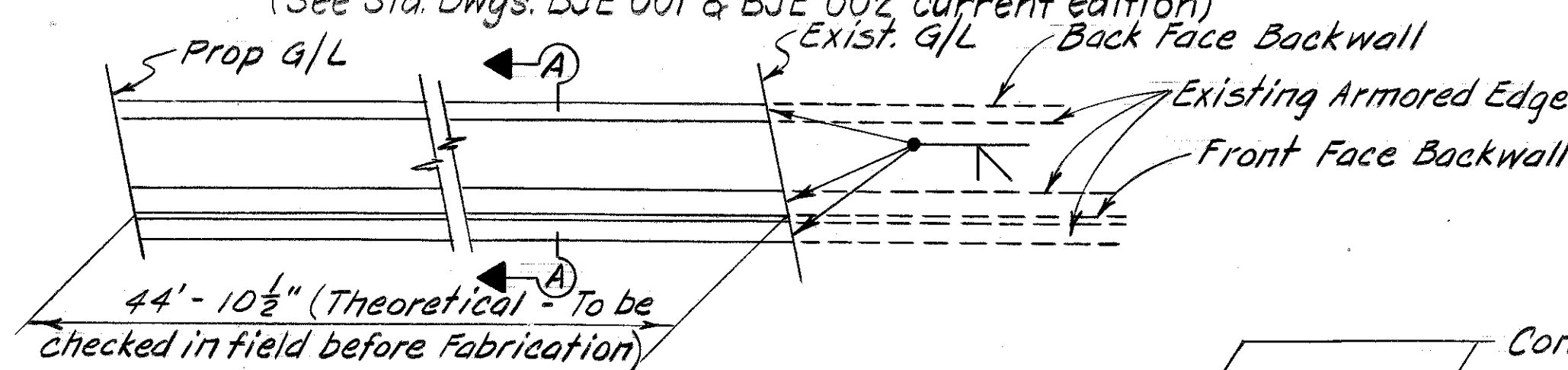
1'6"

1 1/4" Deck Overlay

Overlay

Secure preformed joint filler with approved adhesive. No nails will be permitted.

(See Std. Dwg. BJE 001 & BJE 002 current edition)



checked in field before Fabrication

ARMORED EDGE DETAILS

Concrete to be removed

Exposed Reinforcement in Backwall is to be cleaned and straightened to embed in new concrete.

12-D1 Bars - Grout in 2" ϕ Holes x 1'-3" deep. See General Notes

Slope Protection where called for in Plans.

End comp. placin. for E

Remove this part of Br. Seat
El. 464.6

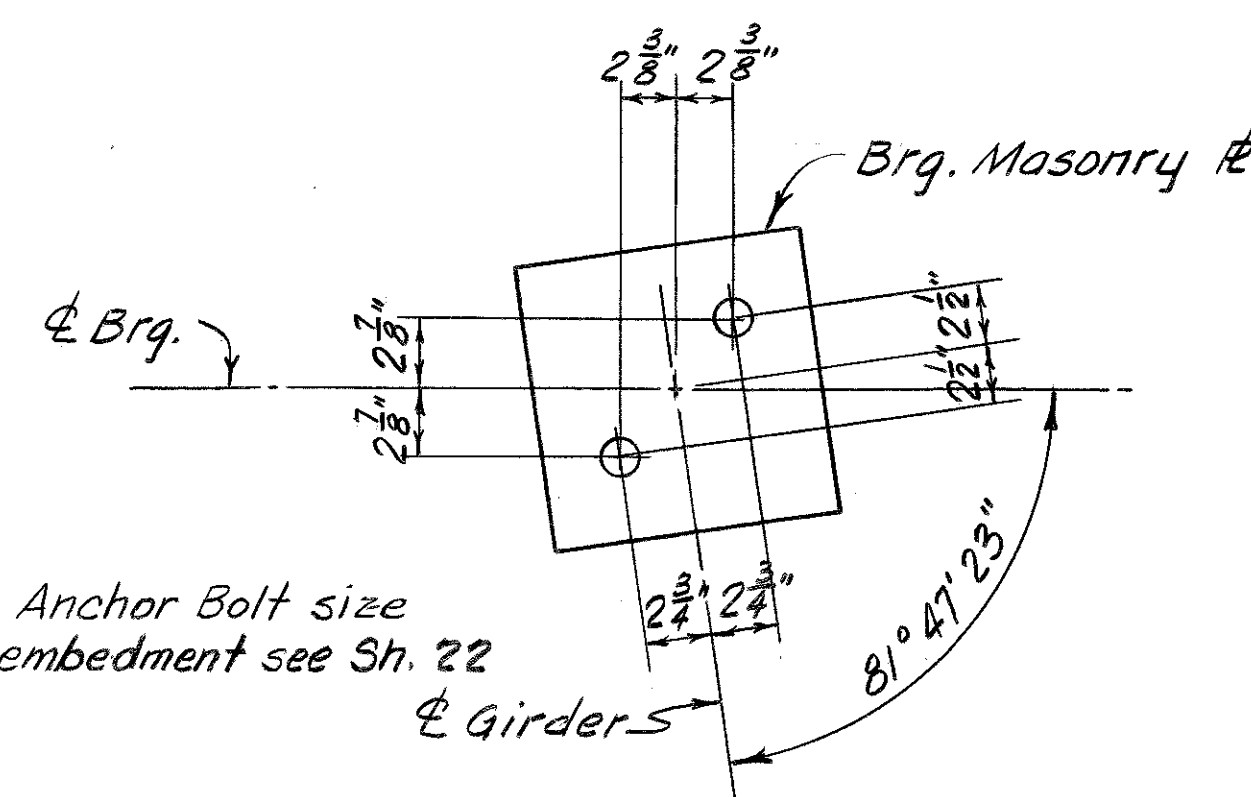
Use Bonding System on old End Bent before placing new concrete. See General Notes.

TYPE ⑤

Dimensions and labels in drawing:

- Vertical dimensions: 1'-3", 1'-1", 1'-7 1/2", 2'-7", 6"
- Horizontal dimensions: 6 1/2", 1'-6", 6", 6", 3 @ 1'-4"=, 4'-0", 6"
- Labels: a, b, d, AC

BILL OF REINFORCEMENT														
MARK	SIZE	TYPE	NO	a		b		c		d		LENGTH		LOCATION
				FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	FT.	IN.	
A22	5	⑤	5	0	0	0	0	1	1	1	3	7	5	Wingwall
A23	5	⑤	5	0	3	0	1	1	1 1/2	1	3	7	6	Wingwall
A24	5	⑤	5	0	6	0	2	1	2 1/2	1	3	7	7	Wingwall



Measured Limits of Structure Excavation and End Bent Backfill.

ing 2:1 Slope

vious Earth backfill

Backfill to be removed prior to forming Beams or Spans

Roadway Subgrade

1-6"

1-6"

2'

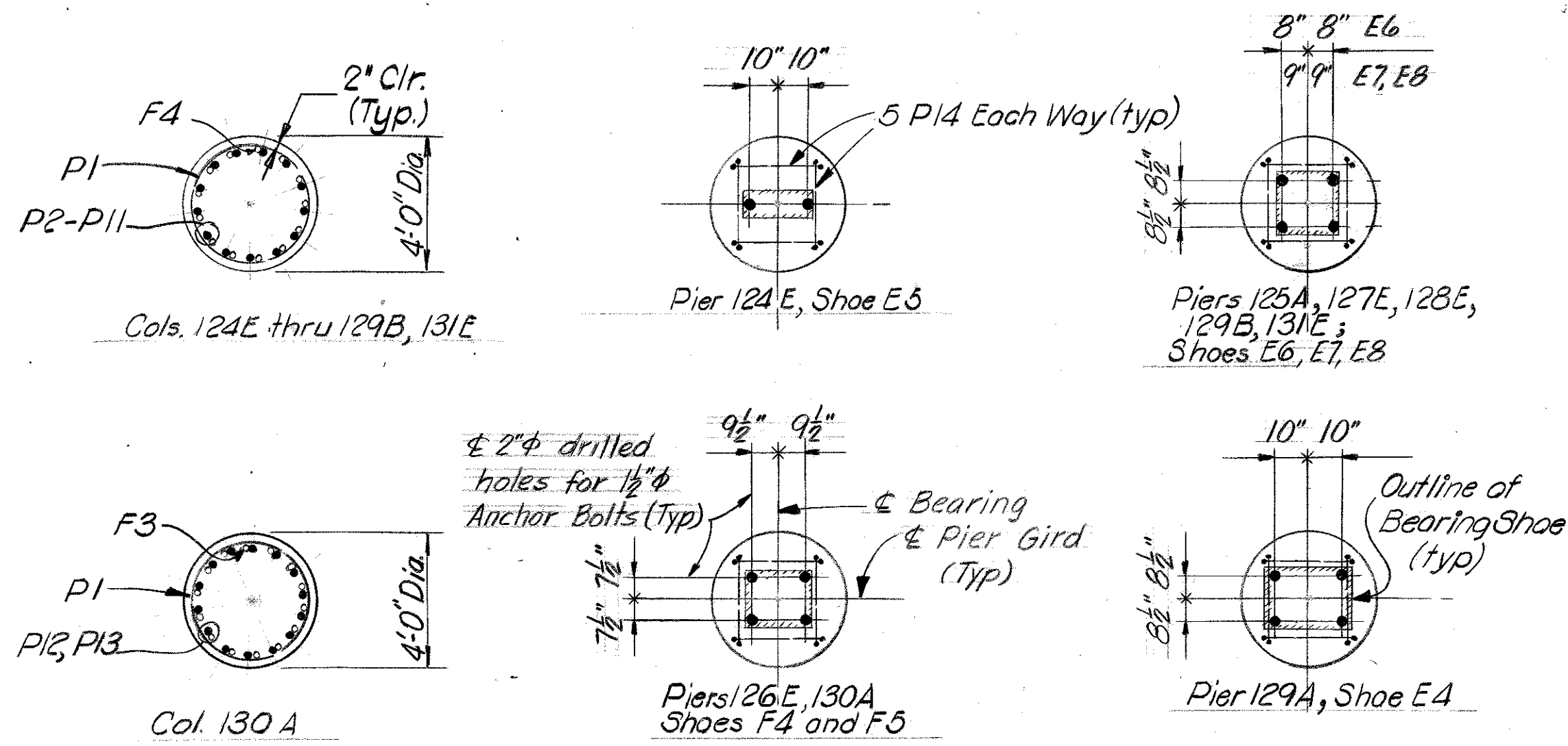
(Earth Core not required)

For Notes and Details not shown, see Standard Drawings BGB-001, BGB-002 and BGB-003, current edition.

- ① Paid for as "End Bent Backfill, Lump Sum"
- ② Incidental to "Deck Overlay, Cu. Yds"
- ③ Incidental to "Concrete Class "A A" Cu. Yds"

UPDATE DATE
LETTING DATE

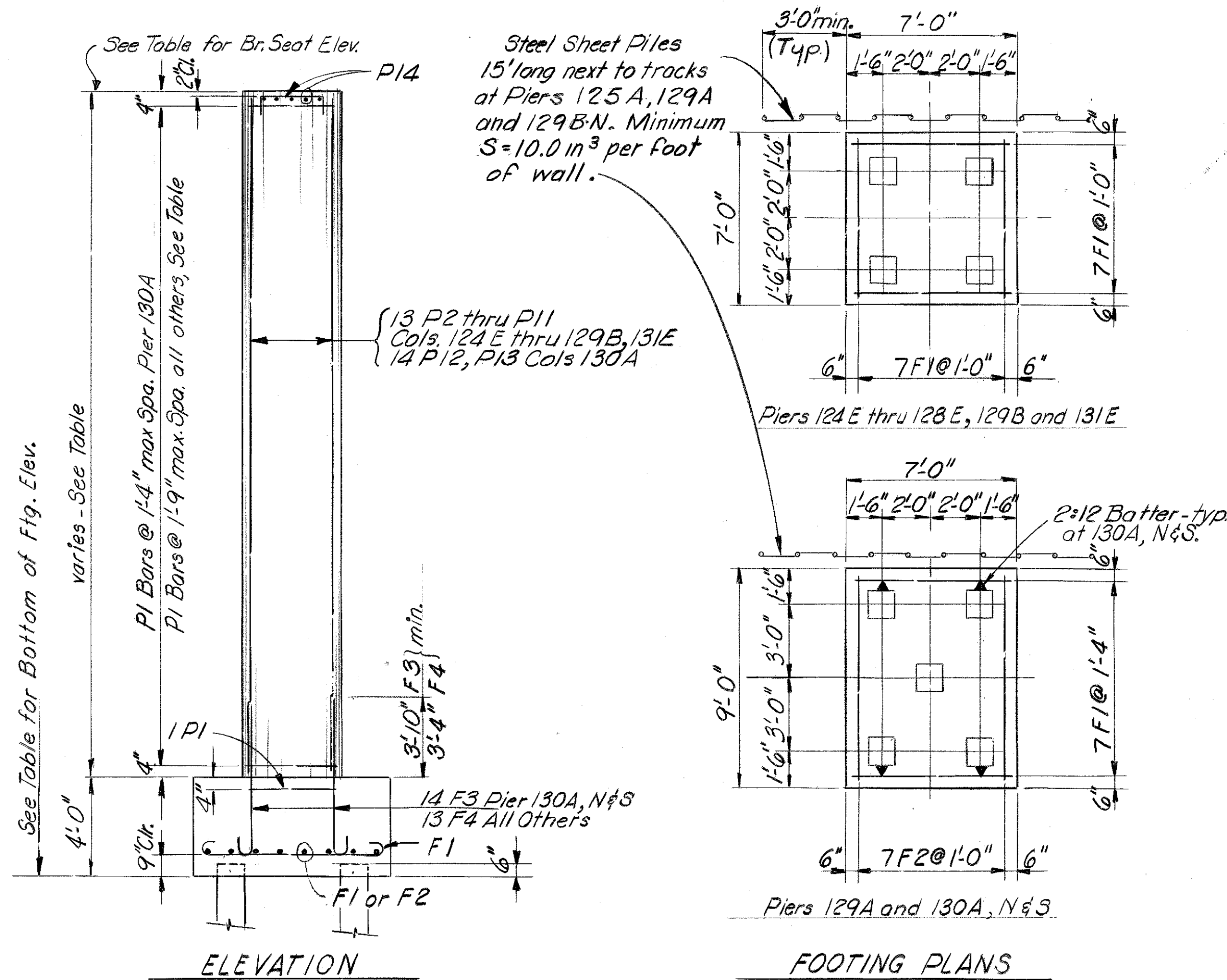
DESIGNED BY: DMS CHECKED BY: DMS DATE: 3-75
DETAILED BY: DMS CHECKED BY: DMS DATE: 4-75
TRACED BY: DMS CHECKED BY: DMS DATE: 4-75



COLUMN SECTIONS

ANCHOR BOLT LOCATIONS

See Framing Plans for orientation
The P14 Bars must be accurately located so that they will not interfere with drilling Anchor Bolt holes.

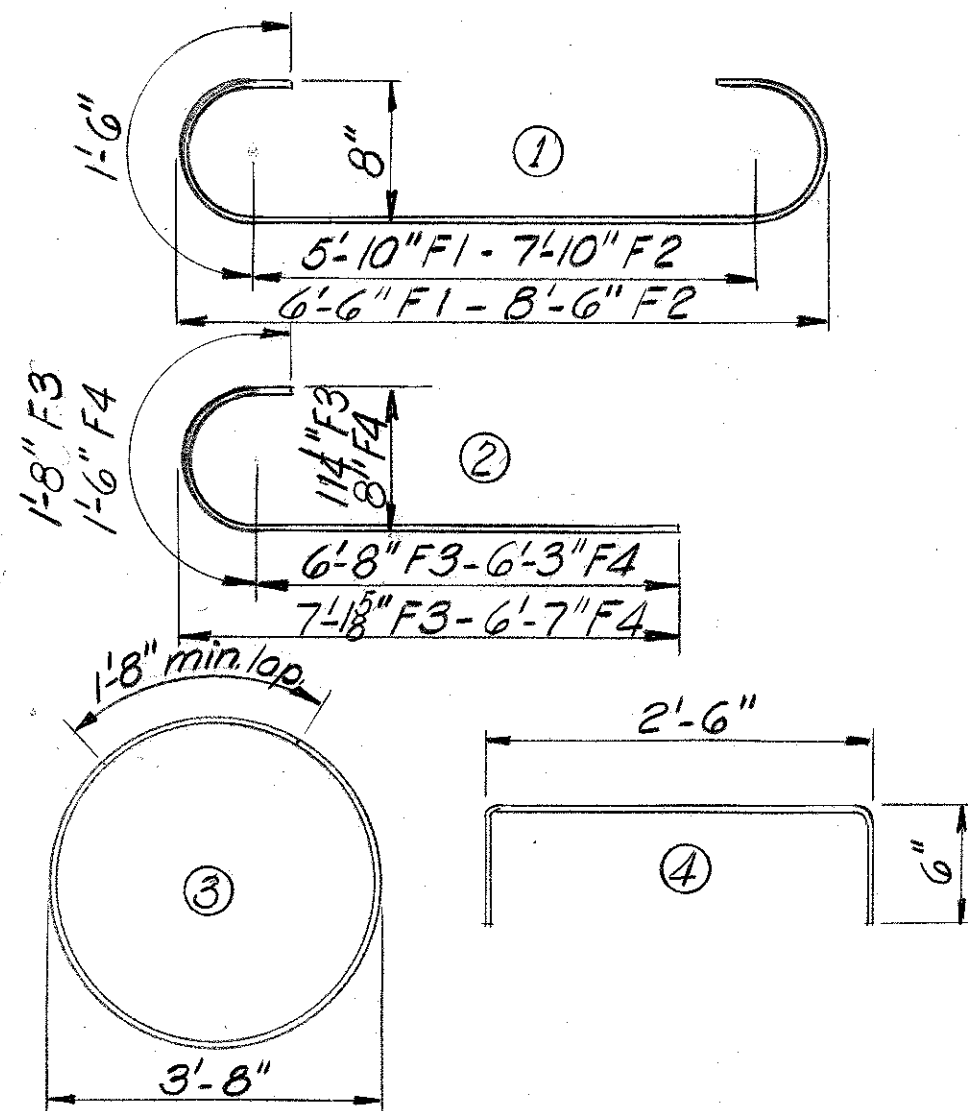


FOOTING PLANS

See Pile Record Sheets for orientation

BILL OF REINFORCEMENT

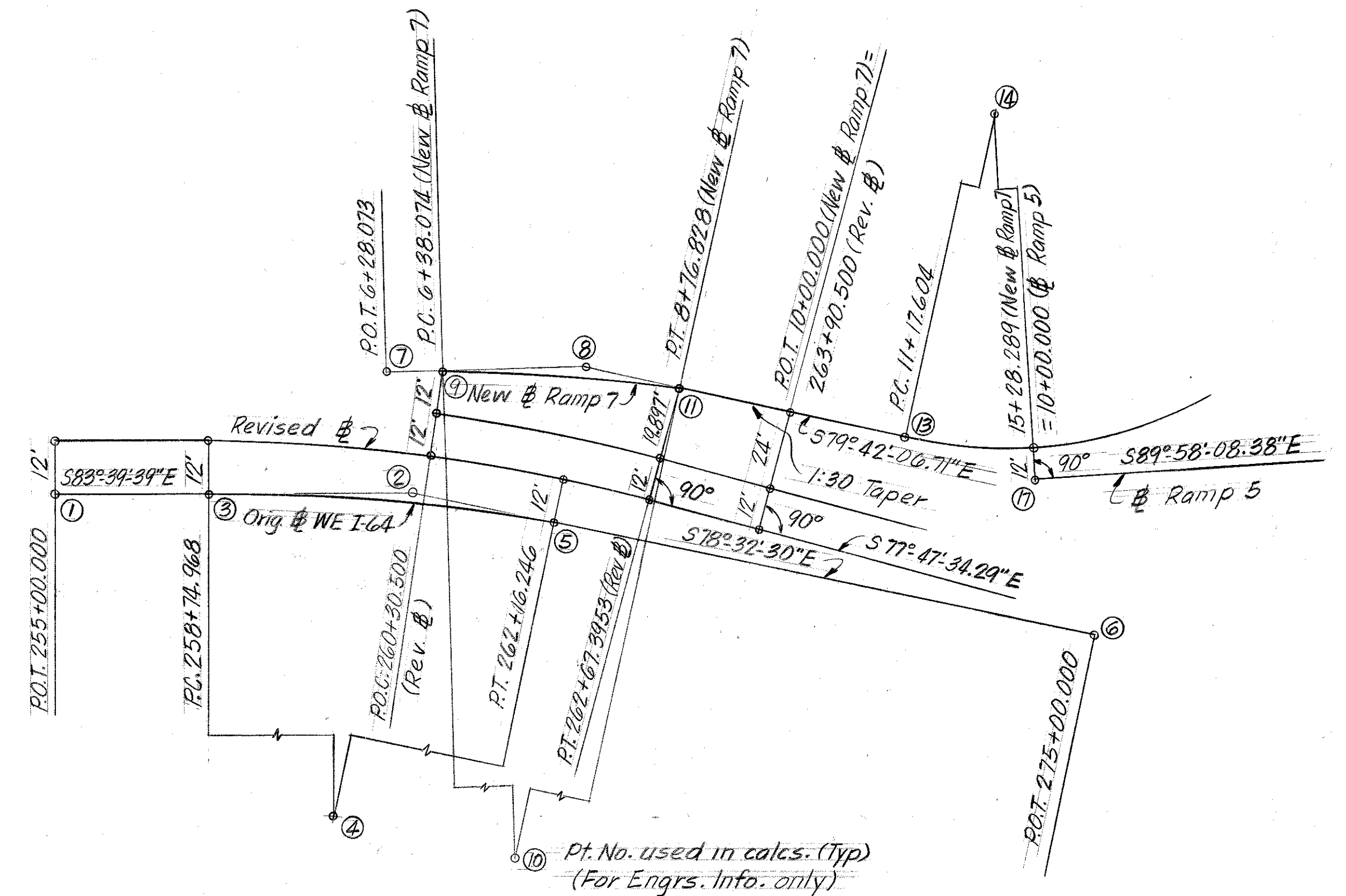
Mark	Size	Type	Nº	Length	Location
F1	8	①	147	8'-10"	Footings
F2	8	①	21	10'-10"	do
F3	9	②	28	8'-4"	do
F4	8	②	130	7'-9"	do
P1	4	③	213	13'-1"	Columns
P2	8	Str	13	26'-10"	Col 124E
P3	8	"	13	28'-6"	" 125A
P4	8	"	13	28'-0"	" 126E
P5	8	"	13	24'-5"	" 127E
P6	8	"	13	26'-6"	" 128E
P7	8	"	13	26'-11"	" 129A
P8	8	"	13	27'-4"	" 129B-N
P9	8	"	13	27'-10"	" 129B-S
P10	8	"	13	21'-3"	" 131E-N
P12	9	"	14	25'-10"	" 130A-N
P13	9	"	14	26'-10"	" 130A-S
P14	4	④	120	3'-5"	Col Tops



BENDING DIAGRAMS

Pier Nº	Bot Ftg	Br. Seat	Col. Ht.	Nº PI Bars
124E	433.003	464.003	27'-0"	17
125A	431.730	464.397	28'-8"	17
126E	433.466	465.633	28'-2"	17
127E	435.410	463.993	24'-7"	15
128E	435.806	466.473	26'-8"	16
129A	435.224	466.307	27'-1"	17
129B-N	434.855	466.355	27'-6"	17
129B-S	434.904	466.904	28'-0"	17
130A-N	435.372	465.372	26'-0"	20
130A-S	435.226	466.226	27'-0"	21
131E-N	435.897	461.314	21'-5"	13
131E-S	435.784	462.284	22'-6"	14

COLUMN DATA TABLE



HORIZONTAL ALIGNMENT

For # WE I-64 and New # Ramp 7
Curve Data, see Layout Shts 334

ESTIMATE OF QUANTITIES

ITEM	UNIT	AMOUNT
Class "A" Concrete	Cu. Yds.	240.0
Steel Reinforcement	Lbs.	21,222
Structure Excavation - Common	Cu. Yds.	280
Piles - 14" Cast In Place - Furnished	Lin. Ft.	1571
"do" - Driven	Lin. Ft.	1571
Sheet Piling *	Sq. Ft.	585

*Sheet Piling is Incidental to Structure Excavation, Common

NOTE: Mr. W. T. Dudley, Superintendent, L & N Railroad Company, P. O. Box 1198, Louisville, Kentucky 40201, should be notified when the work on the piers near the railroad tracks is started so that affected Railroad Company employees can be alerted.

SHEET 8

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON
ROAD

STATION 267+00
CONSTRUCTION PROJECT NO.
P. E. PROJECT NO. 164-2(92)
MAINTENANCE PROJECT NO.
DRAWING NO.
19238

PIERS

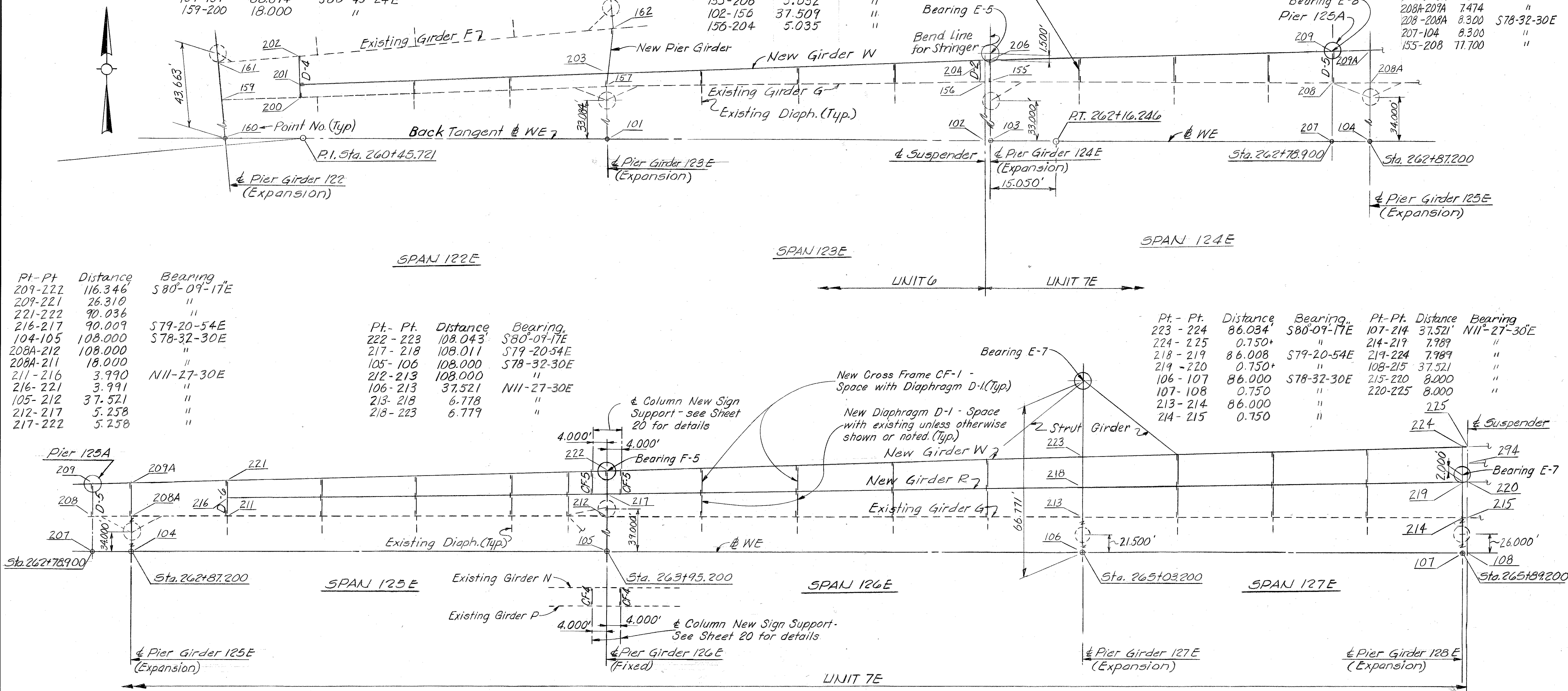
UPDATE DATE
LETTING DATE

Pt. - Pt.	Distance	Bearing
101 - 160	86.837'	N 78°-32'-30" W
159 - 160	32.902	S 6°-20'-21" W
161 - 159	8.469	"
157 - 101	36.167	S 9°-55'-59" W
162 - 157	13.082	S 11°-47'-22" W
157 - 203	3.038	"
161 - 162	90.000	S 83°-39'-39" E
159 - 157	88.874	S 80°-43'-24" E
159 - 200	18.000	"

Pt. - Pt.	Distance	Bearing
201 - 203	71.007'	S 80°-45'-06" E
200 - 202	9.404	N 9°-16'-36" E
200 - 201	3.000	"
161 - 202	18.458	S 83°-39'-39" E
161 - 162	90.000	"

Pt. - Pt.	Distance	Bearing
101 - 102	85.912'	S 78°-32'-30" E
156 - 157	86.885	S 79°-26'-07" E
155 - 156	0.750	"
203 - 204	86.922	S 80°-45'-06" E
204 - 206	0.751	"
102 - 103	0.750	S 78°-32'-30" E
103 - 155	37.521	N 11°-27'-30" E
155 - 206	5.052	"
102 - 156	37.509	"
156 - 204	5.035	"

Pt. - Pt.	Distance	Bearing
103 - 104	86.000	S 78°-32'-30" E
207 - 104	8.300	"
206 - 209	77.731	S 80°-09'-17" E
209 - 207A	8.303	"
207 - 209	44.761	N 11°-27'-30" E
208 - 209	7.240	"
207 - 208	37.521	"
104 - 208A	37.521	"
208A - 207A	7.474	"
208 - 208A	8.300	S 78°-32'-30" E
207 - 104	8.300	"
155 - 208	77.700	"



Pt. - Pt.	Distance	Bearing
209 - 222	116.346'	S 80°-09'-17" E
209 - 221	26.310	"
221 - 222	90.036	"
216 - 217	90.009	S 79°-20'-54" E
104 - 105	108.000	S 78°-32'-30" E
208A - 212	108.000	"
208A - 211	18.000	"
211 - 216	3.990	N 11°-27'-30" E
216 - 221	3.991	"
105 - 212	37.521	"
212 - 217	5.258	"
217 - 222	5.258	"

Pt. - Pt.	Distance	Bearing
222 - 223	108.043'	S 80°-09'-17" E
217 - 218	108.011	S 79°-20'-54" E
105 - 106	108.000	S 78°-32'-30" E
212 - 213	108.000	"
106 - 213	37.521	N 11°-27'-30" E
213 - 218	6.778	"
218 - 223	6.779	"

Pt. - Pt.	Distance	Bearing
223 - 224	86.034'	S 80°-09'-17" E
224 - 225	0.750+	"
218 - 219	86.008	S 79°-20'-54" E
219 - 220	0.750+	"
106 - 107	86.000	S 78°-32'-30" E
107 - 108	0.750	"
213 - 214	86.000	"
214 - 215	0.750	"

Pt. - Pt.	Distance	Bearing
107 - 214	37.521'	N 11°-27'-30" E
214 - 219	7.989	"
219 - 224	7.989	"
108 - 215	37.521	"
215 - 220	8.000	"
220 - 225	8.000	"

EXISTING STRUCTURE DIMENSIONS

Dimensions were taken from original Design Drawings. All dimensions should be verified by field measurement before fabrication begins.

Note: All dimensions shown on this sheet are horizontal.

NOTES:

For Girder Details see Sheets 11 thru 14.
For Pier Girder and Diaphragm Details see Sheets 15 thru 18.
For Cross Frame Details see Sheet 18.
For Bearing Details see Sheet 22.

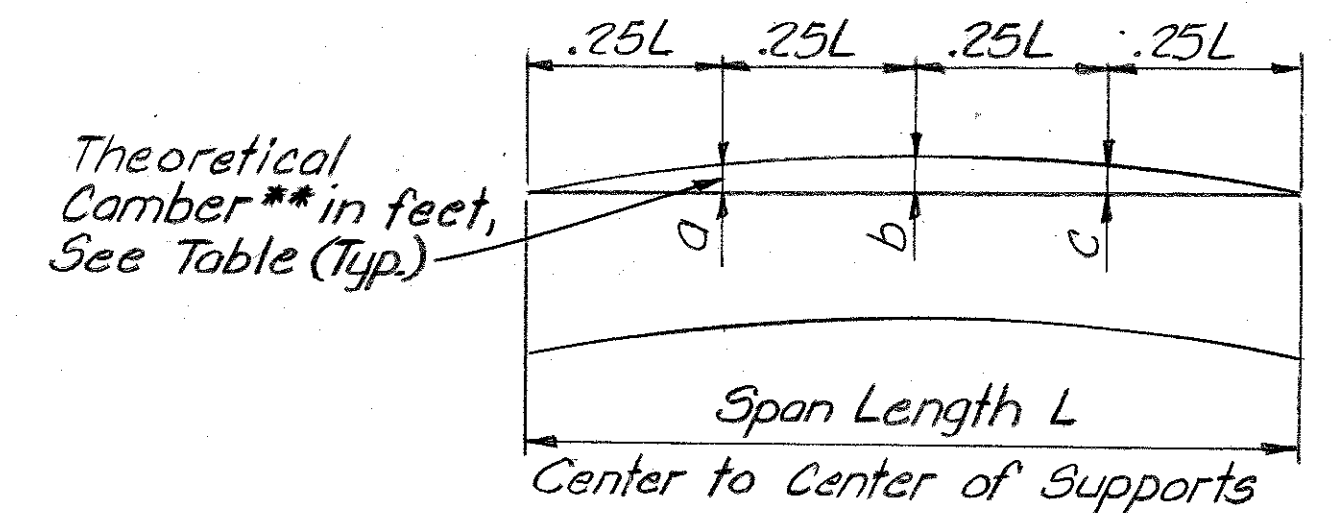
FRAMING PLAN UNITS 6(PT.) & 7E
STRUCTURAL STEEL

Sheet 9

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON
ROAD
STATION 267+00
CONSTRUCTION PROJECT NO.
P.E. PROJECT NO. 164-2(92)
MAINTENANCE PROJECT NO.
DRAWING NO.
19238

19238

DESIGNED BY <u>RBS & DMB</u>	CHECKED BY <u>DMB & RBS</u>	DATE <u>11-74</u>	REVISED	DATE
DRAWN BY <u>R.H.N.</u>	Detailed BY <u>ELW</u>	DATE <u>4/75</u>	REVISED	DATE
TRACED BY	CHECKED BY	DATE	REVISED	DATE



CAMBER					TABLE						
SPAN		GIRDER	a	b	c	SPAN		GIRDER	a	b	c
BEGIN	END					BEGIN	END				
D-4	123E	W	.017	.011	.007	129B	130A	W	.001	.004	.001
123E	124E	W	.062	.110	.088	129B	130A	V		*.010	
124E	125E	W	.030	.028	0	129B	130A	U	.022	.025	.009
						129B	130A	T	-.006	-.008	-.012
125E	126E	W	.089	.143	.087	129B	130A	S	.022	.025	.009
D-6	126E	R	.060	.070	.030	129B	130A	R	-.001	-.001	-.006
126E	127E	W	0	.030	.016	130A	131E	W	.035	.061	.050
126E	127E	R	.040	.070	.033	130A	131E	V	.082	.118	.082
						130A	131E	U	.032	.061	.051
127E	128E	W	.061	.095	.075	130A	131E	T	.050	.082	.066
127E	128E	R	.035	.079	.065	130A	131E	S	.032	.061	.051
						130A	131E	R	.045	.077	.061
128E	129A	W	.012	.017	.008						
128E	129A	T	.011	.012	.004	131E	101E	W	.020	.027	.020
128E	129A	R	.016	.019	.008	131E	101E	V	.018	.025	.018
						131E	101E	U	.018	.025	.018
129A	129B	W	.008	.026	.016	131E	101E	T	.018	.025	.018
129A	129B	U		*.010		131E	101E	S	.018	.025	.018
129A	129B	T	.010	.038	.027	131E	101E	R	.013	.019	.013
129A	129B	S		*.010							
129A	129B	R	.009	.032	.022						

SHEET II

STATION 267+00		P. E. PROJECT NO. 164-2(92)
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.	DRAWING NO. <i>19238</i>

GIRDER DETAILS

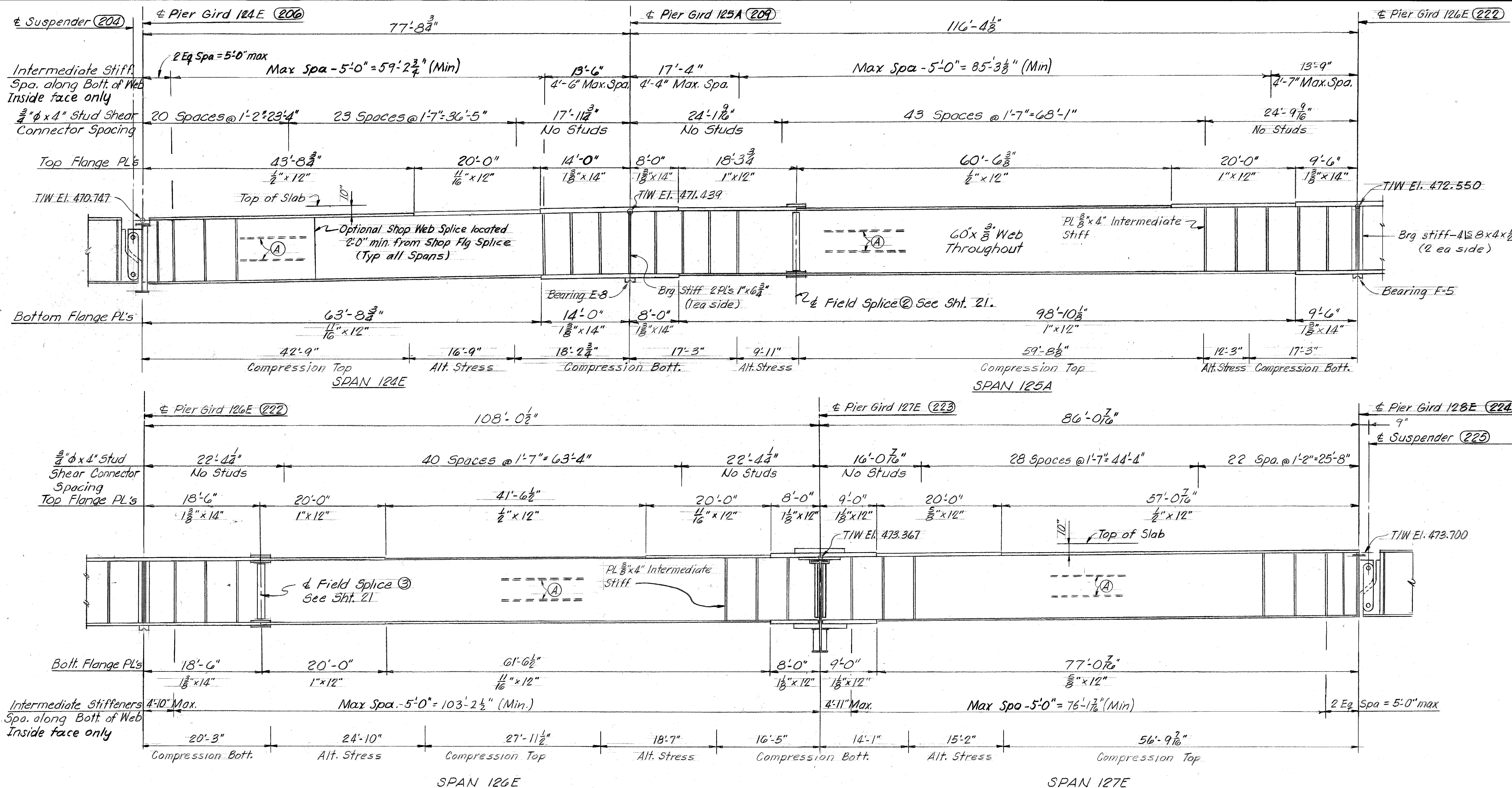
STRUCTURAL STEEL

NOTES

Fillet weld size to be minimum size specified by AWS Specifications unless noted otherwise but not less than $\frac{1}{4}$ ".

UPDATE DATE
LETTING DATE

DESIGNED BY: DME
CHECKED BY: RBS
DATE: 11-74
REVISION: 4/75
DATE: 4/75
TRACED BY: RHM
CHECKED BY: ELU
DATE: 4/75



Ⓐ 4x8" Longitudinal Stiffener Full Length On Outside Face Only.

For Notes see Sht 11.

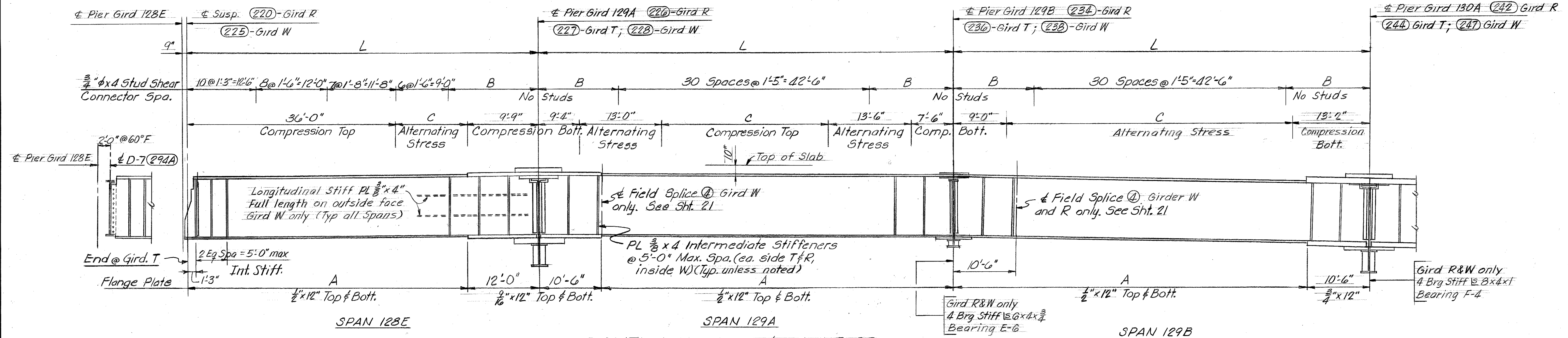
ELEVATION - GIRDER W - UNIT 7E

GIRDER DETAILS
STRUCTURAL STEEL

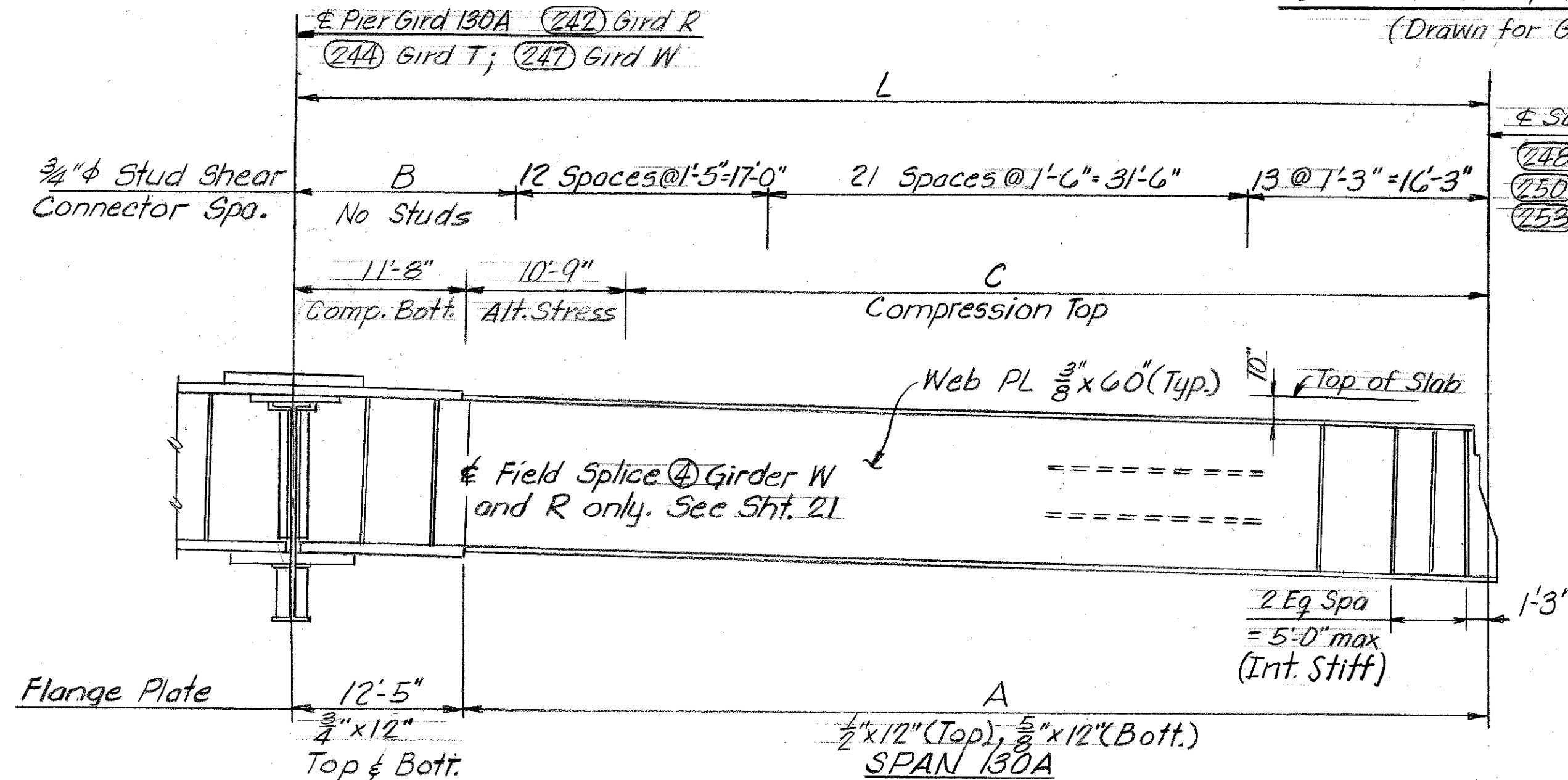
SHEET 12

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON
ROAD
P. E. PROJECT NO. 164-2(92)

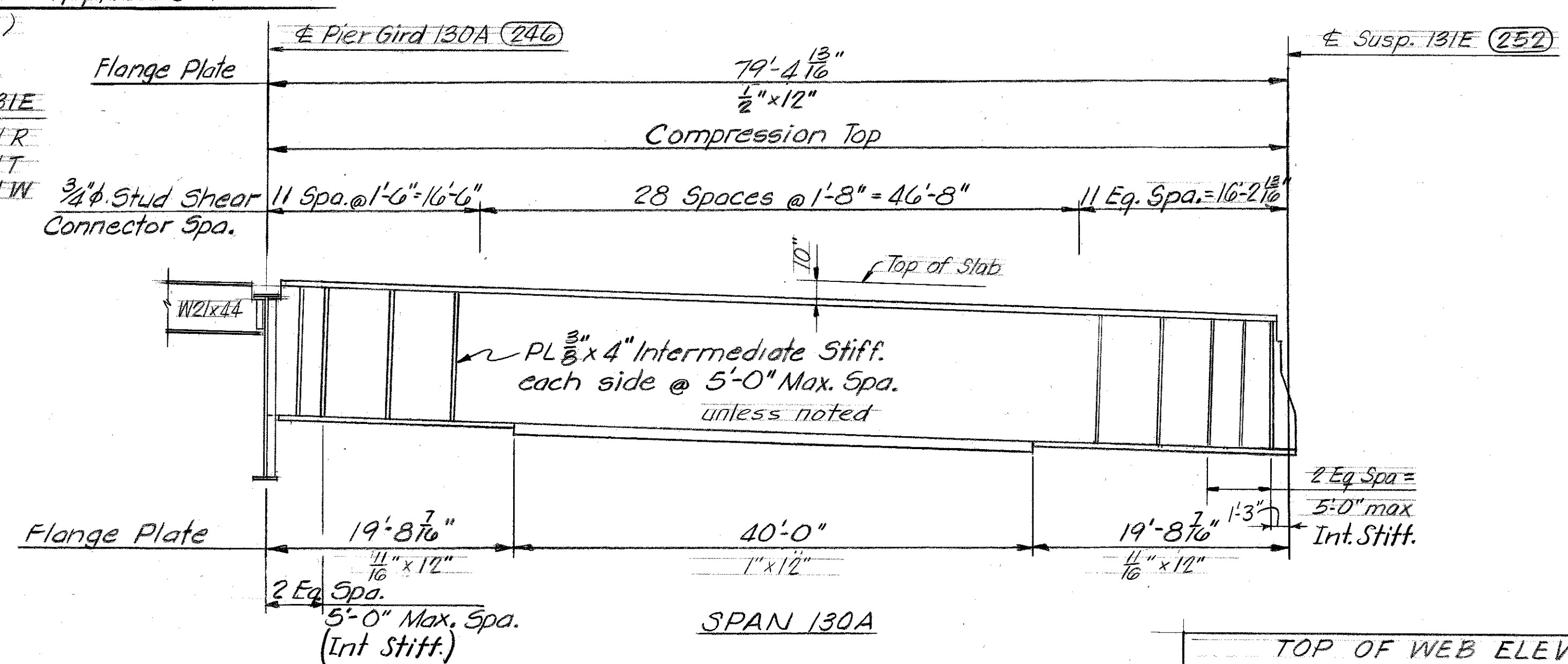
STATION 267+00
CONSTRUCTION PROJECT NO.
MAINTENANCE PROJECT NO.
DRAWING NO.
19238



ELEVATION-GIRDERS W,T,&R-UNIT 8E

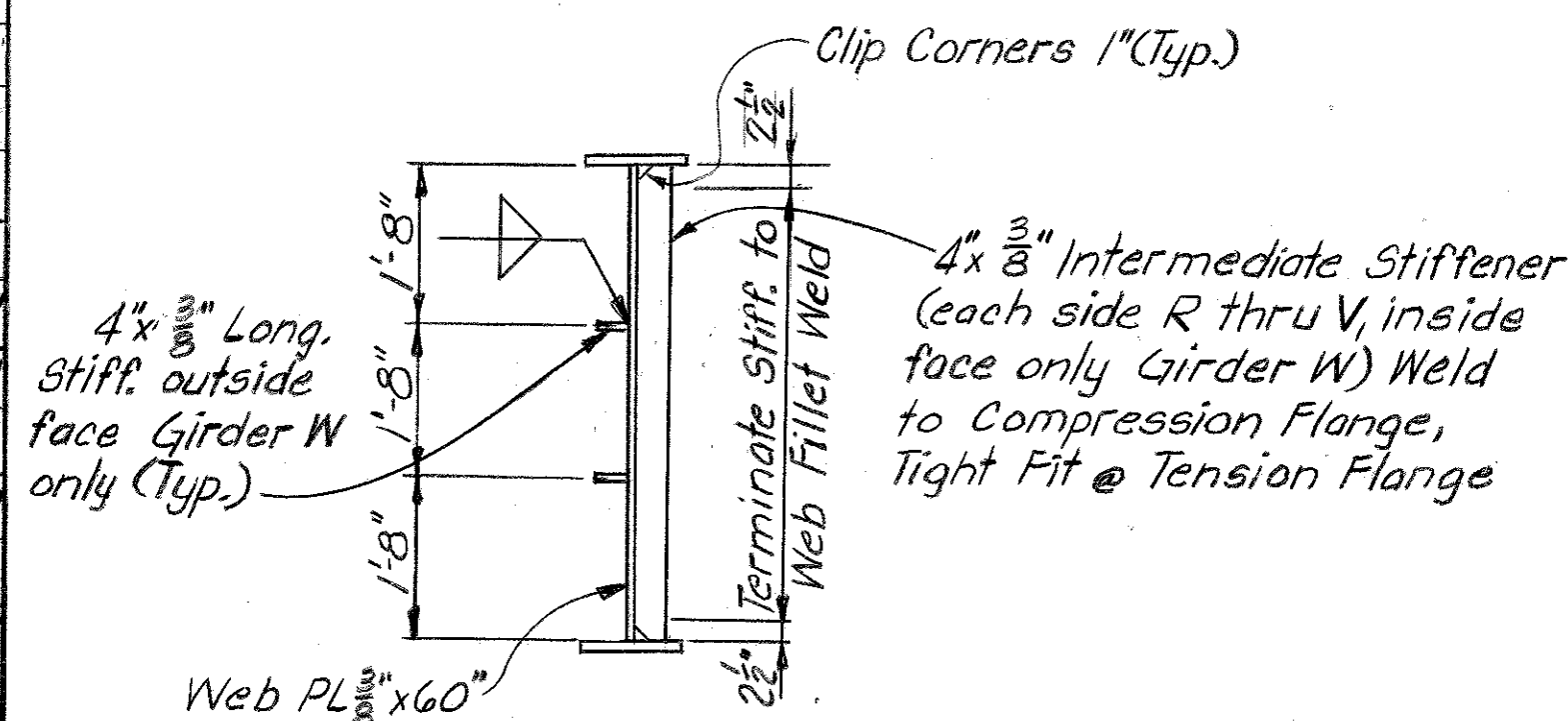


ELEVATION - GIRDERS W, T, & R - UNIT 8E

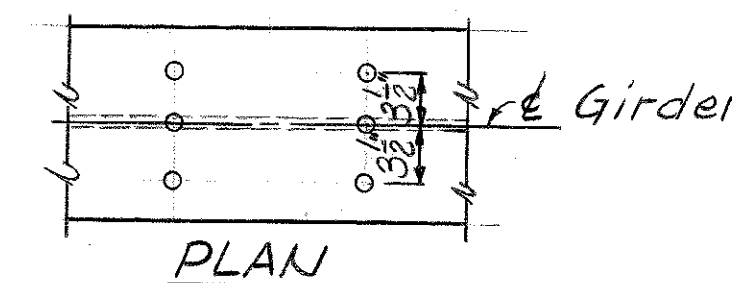


ELEVATION-GIRDER-V-UNIT 8E

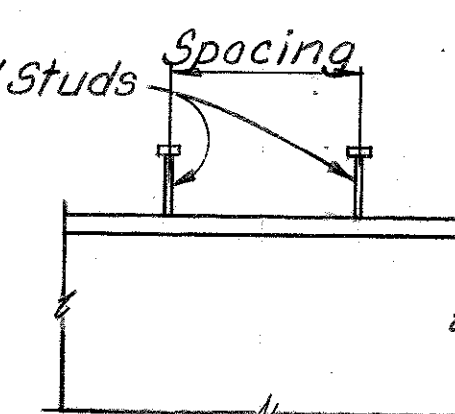
GIRDER	SPAN		L	A	B	C
	PIER	PIER				
W	128E	129A	60'-2 $\frac{1}{2}$ "	48'-2 $\frac{1}{2}$ "	15'-0 $\frac{1}{2}$ "	14'-5"
	129A	129B	70'-4 $\frac{3}{8}$ "	59'-10 $\frac{3}{8}$ "	13'-11 $\frac{1}{2}$ "	27'-0 $\frac{1}{2}$ "
	129B	130A	70'-6 $\frac{15}{16}$ "	60'-0 $\frac{15}{16}$ "	14'-0 $\frac{1}{2}$ "	48'-4 $\frac{15}{16}$ "
	130A	131E	79'-6 $\frac{3}{8}$ "	67'-1 $\frac{3}{8}$ "	14'-9 $\frac{3}{8}$ "	57'-1 $\frac{3}{8}$ "
U	129B	130A	70'-1 $\frac{15}{16}$ "	60'-1 $\frac{15}{16}$ "	17'-1 $\frac{15}{16}$ "	42'-1 $\frac{15}{16}$ "
	130A	131E	79'-5 $\frac{1}{2}$ "	67'-5 $\frac{1}{2}$ "	20'-1 $\frac{1}{2}$ "	54'-5 $\frac{1}{2}$ "
T	128E	129A	60'-0 $\frac{3}{4}$ "	48'-0 $\frac{3}{4}$ "	14'-10 $\frac{3}{8}$ "	14'-3 $\frac{5}{8}$ "
	129A	129B	70'-0 $\frac{3}{4}$ "	59'-6 $\frac{3}{4}$ "	13'-9 $\frac{3}{8}$ "	26'-8 $\frac{3}{4}$ "
	129B	130A	70'-0 $\frac{15}{16}$ "	59'-6 $\frac{15}{16}$ "	13'-9 $\frac{1}{2}$ "	47'-10 $\frac{15}{16}$ "
	130A	131E	79'-9"	67'-4"	15'-0"	57'-4"
S	129B	130A	70'-0 $\frac{1}{2}$ "	60'-0 $\frac{1}{2}$ "	17'-10 $\frac{1}{2}$ "	42'-0 $\frac{1}{2}$ "
	130A	131E	80'-0 $\frac{1}{2}$ "	68'-0 $\frac{1}{2}$ "	20'-8 $\frac{1}{2}$ "	55'-0 $\frac{1}{2}$ "
R	128E	129A	60'-0"	48'-0"	14'-10"	14'-3"
	129A	129B	70'-0"	59'-6"	13'-9"	26'-8"
	129B	130A	70'-0"	59'-6"	13'-9"	47'-10"
	130A	131E	80'-5"	68'-0"	15'-8"	58'-0"



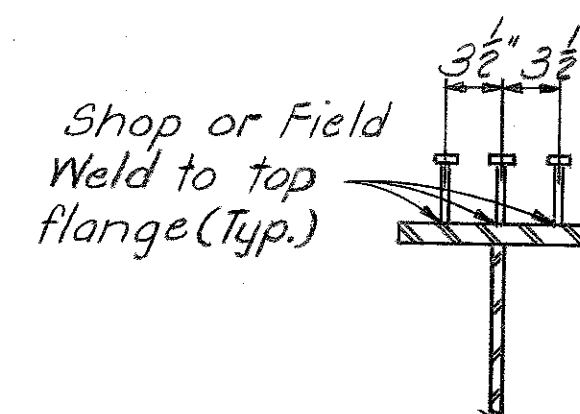
LONGITUDINAL AND INTERMEDIATE STIFFENER DETAILS



PLAN



ELEVATION



SECTION

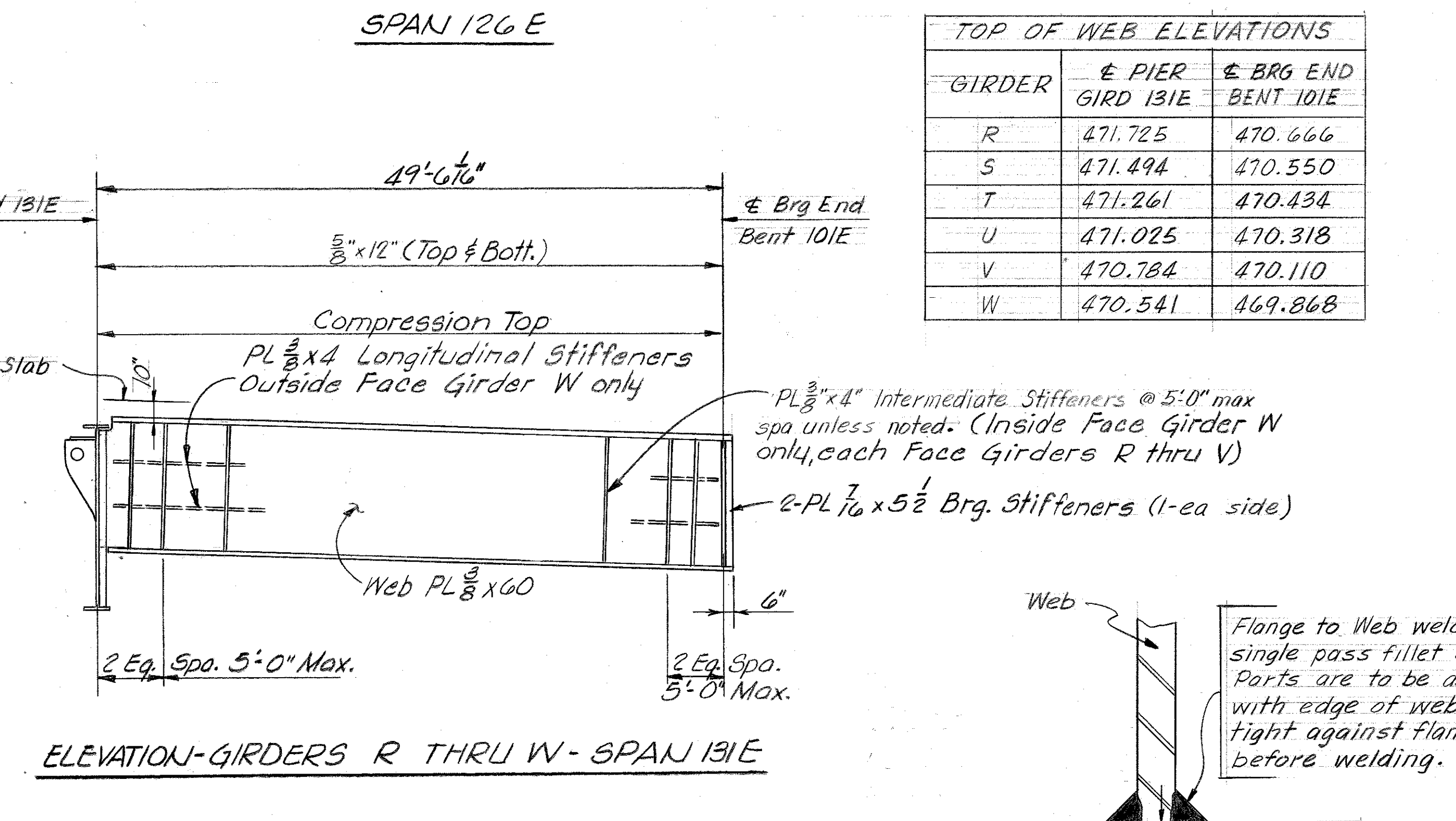
SHEAR CONNECTOR DETAILS
(ASTM A-108)

TOP OF WEB ELEVATIONS				
	GIRDER			
	R	T	W	V
£ Susp 128E	473.834	473.768	473.701	—
£ Pier Gird 129A	473.955	473.812	473.680	—
£ Pier Gird 129B	473.873	473.605	473.324	—
£ Pier Gird 130A	473.174	472.791	472.320	472.451
£ Susp 131E	471.743	471.275	470.552	470.796

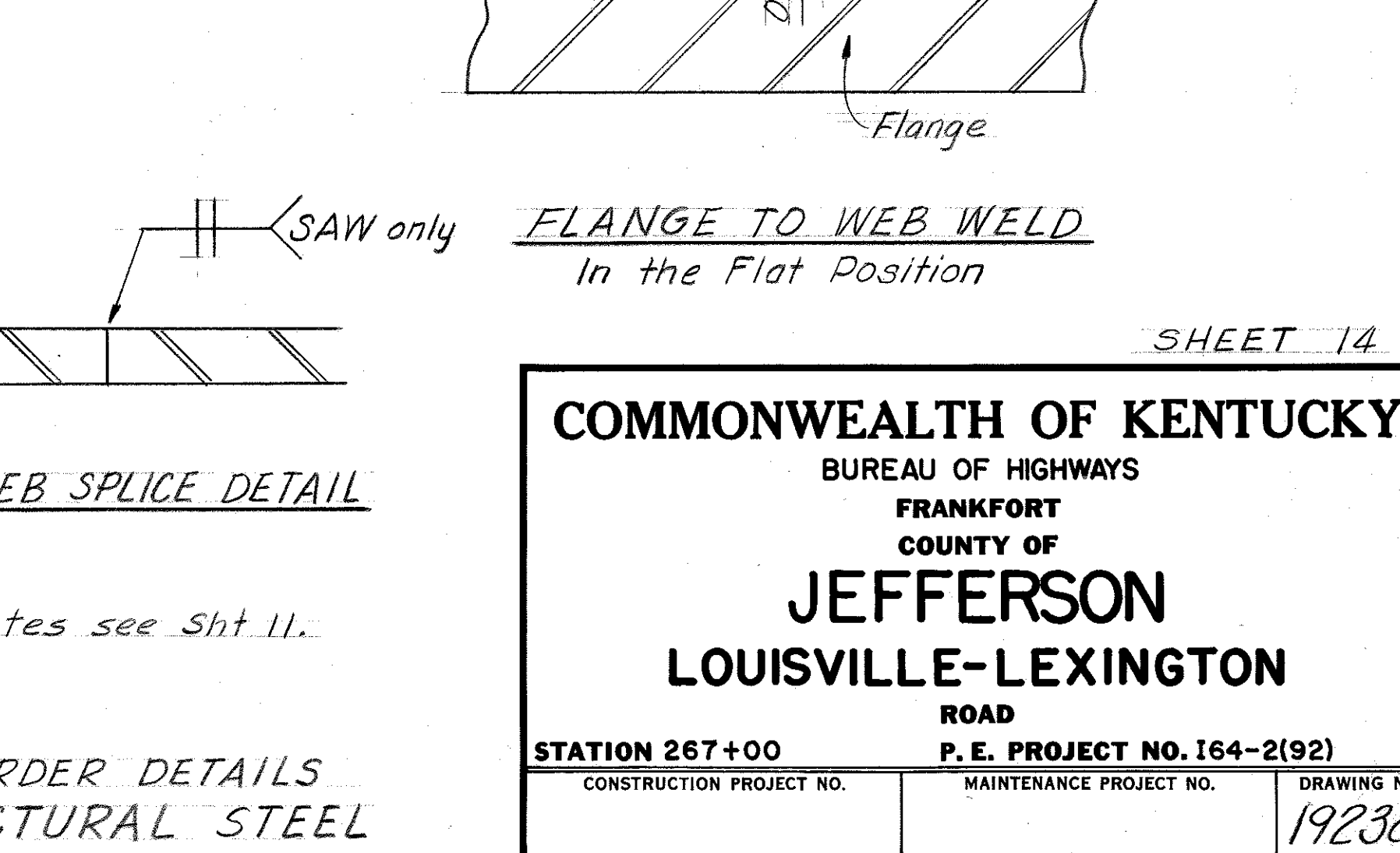
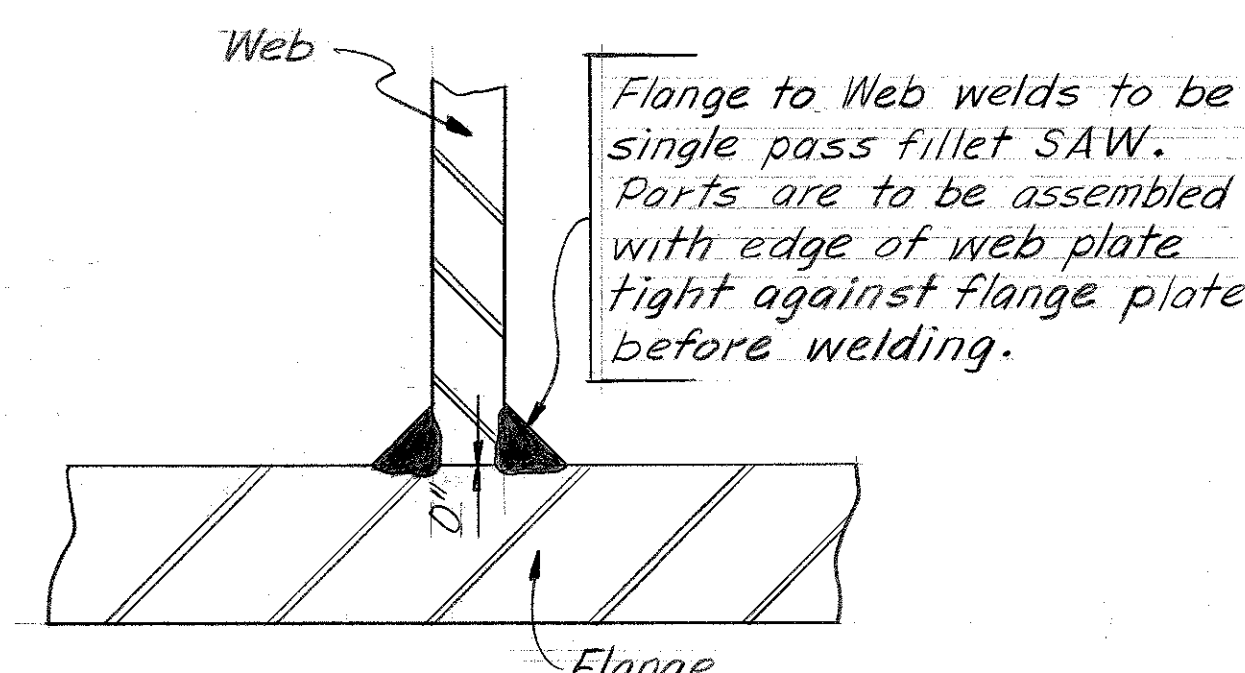
For Notes see Snt 11.

GIRDER DETAILS
STRUCTURAL STEEL

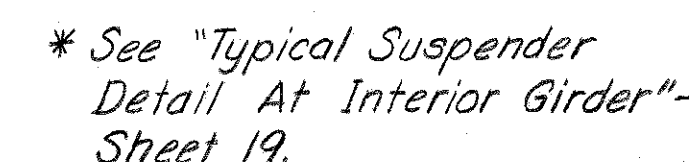
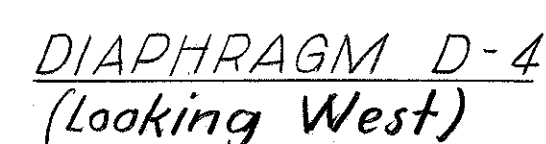
DESIGNED BY	<i>PME</i>	CHECKED BY	<i>RBS</i>	DATE	<i>11-74</i>	REVISED		DATE	
REVIEWED BY		CHECKED BY	<i>ELW</i>	DATE	<i>4/75</i>	REVISED		DATE	
TRACED BY		CHECKED BY		DATE		REVISED		DATE	



TOP OF WEB ELEVATIONS		
GIRDER	£ PIER GIRD 131E	£ BRG END BENT 101E
R	471.725	470.666
S	471.494	470.550
T	471.261	470.434
U	471.025	470.318
V	470.784	470.110
W	470.541	469.868



DESIGNED BY	<u>RDS & DMB</u>	CHECKED BY	<u>CHH & RDS</u>	DATE	<u>2/75</u>	REVISED	_____	DATE	_____
DRAWN BY	<u>ELW</u>	CHECKED BY	<u>DMB</u>	DATE	<u>3-75</u>	REVISED	_____	DATE	_____
DETAILS BY	_____	CHECKED BY	_____	DATE	_____	REVISED	_____	DATE	_____



For Intermediate Stiffener Details see Sht. 17.
For Bearing Stiffener Details see Sheet 14.
(200) - indicates point shown on Framing
Plan, Sheets 9 and 10.
For additional notes see Sheet 11.

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON

STATION 267+00		P. E. PROJECT NO. 164-2(92)
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.	DRAWING NO.

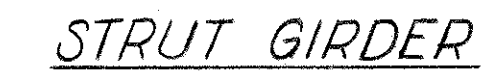
19238

DIAPHRAGMS AND
PIER GIRDERS
STRUCTURAL STEEL

DESIGNED BY	DMB	CHECKED BY	RBS	DATE	2/75	REVISED		DATE	
REFINED BY	ELL	CHECKED BY	DMB	DATE	3-75	REVISED		DATE	
TRACED BY		CHECKED BY		DATE		REVISED		DATE	



PART FRAMING PLAN
@ PIER 127E



For Intermediate Stiffener Details see Sht. 17.
For Bearing Stiffener Details see Sheet 14.
(213) - indicates point shown on Framing
Plan, Sheets 9 and 10.
For additional notes see Sheet 11.

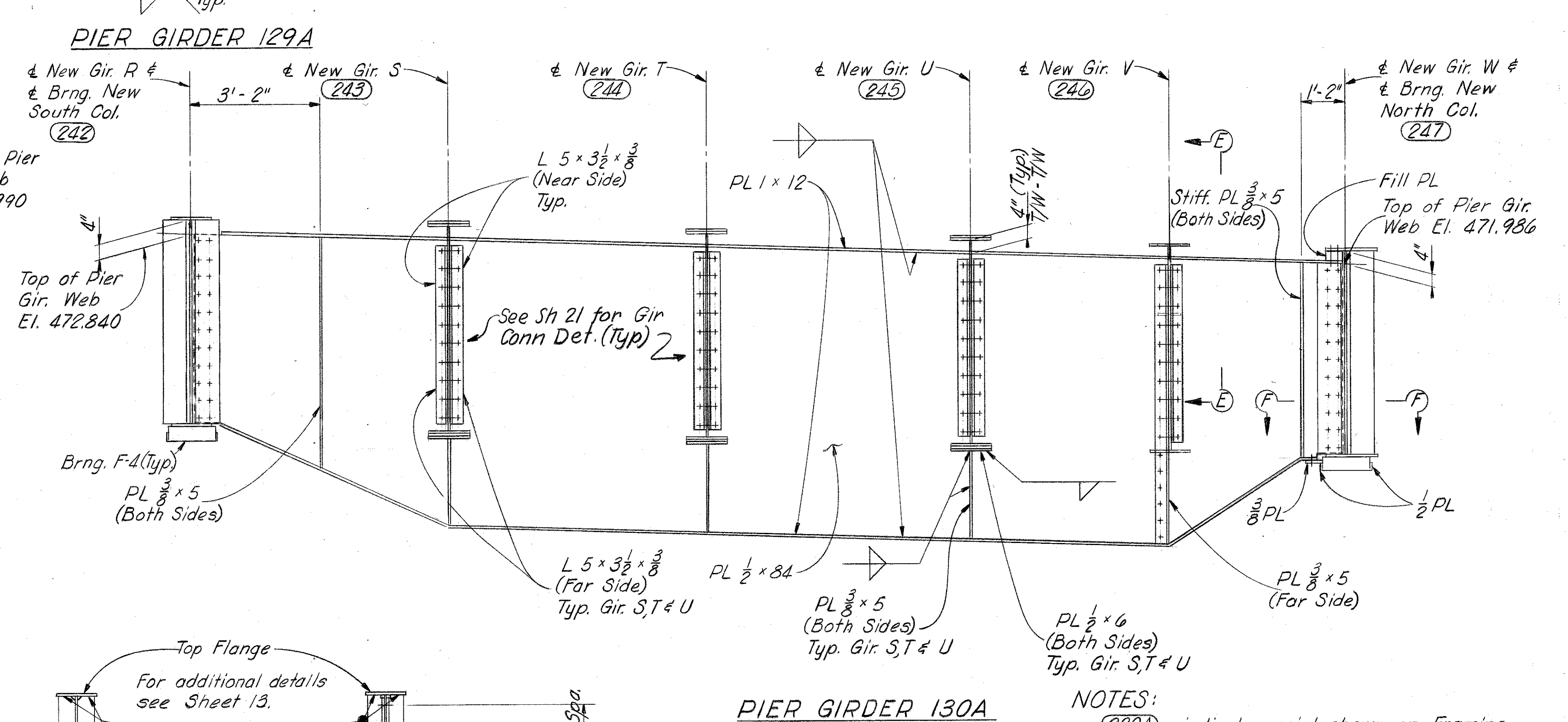
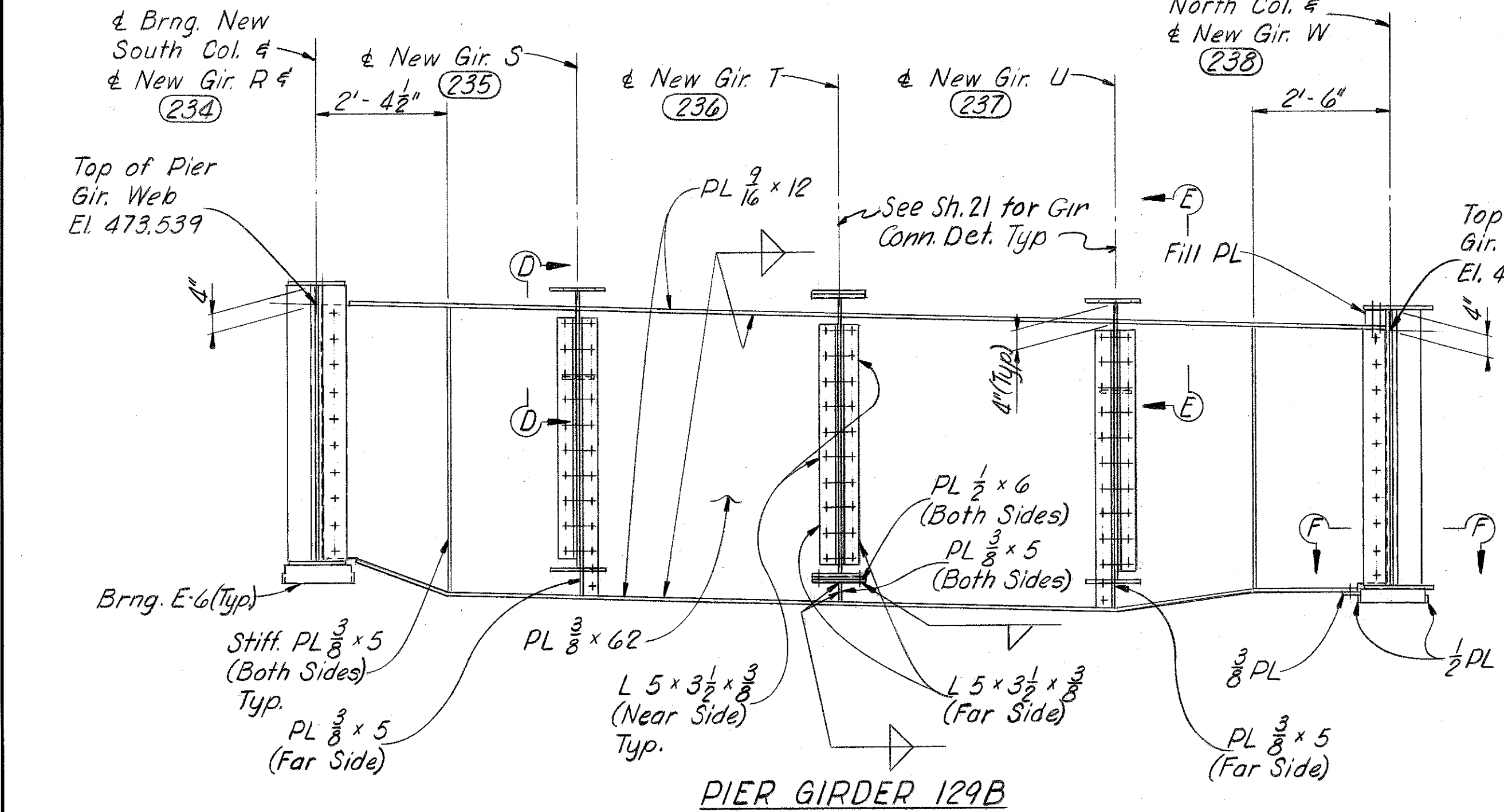
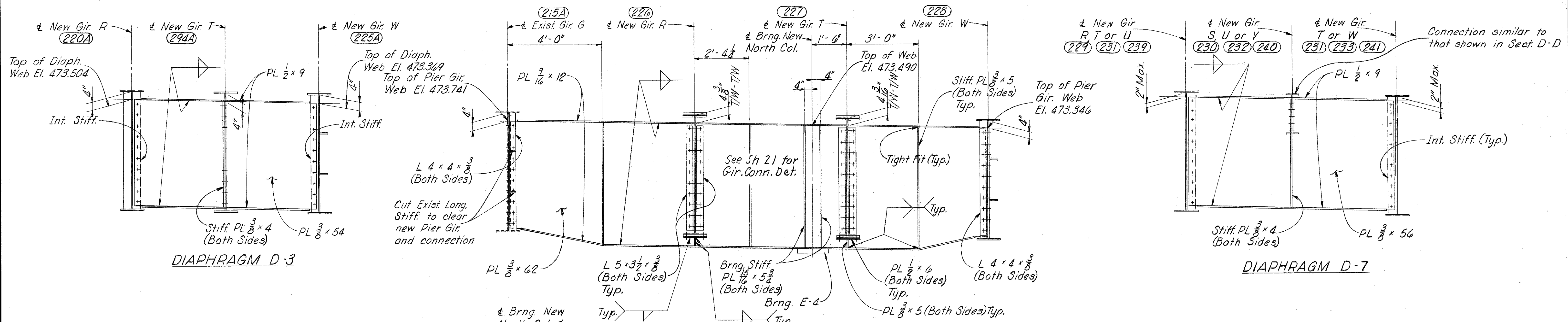
SHEET 16

DRAWING NO.

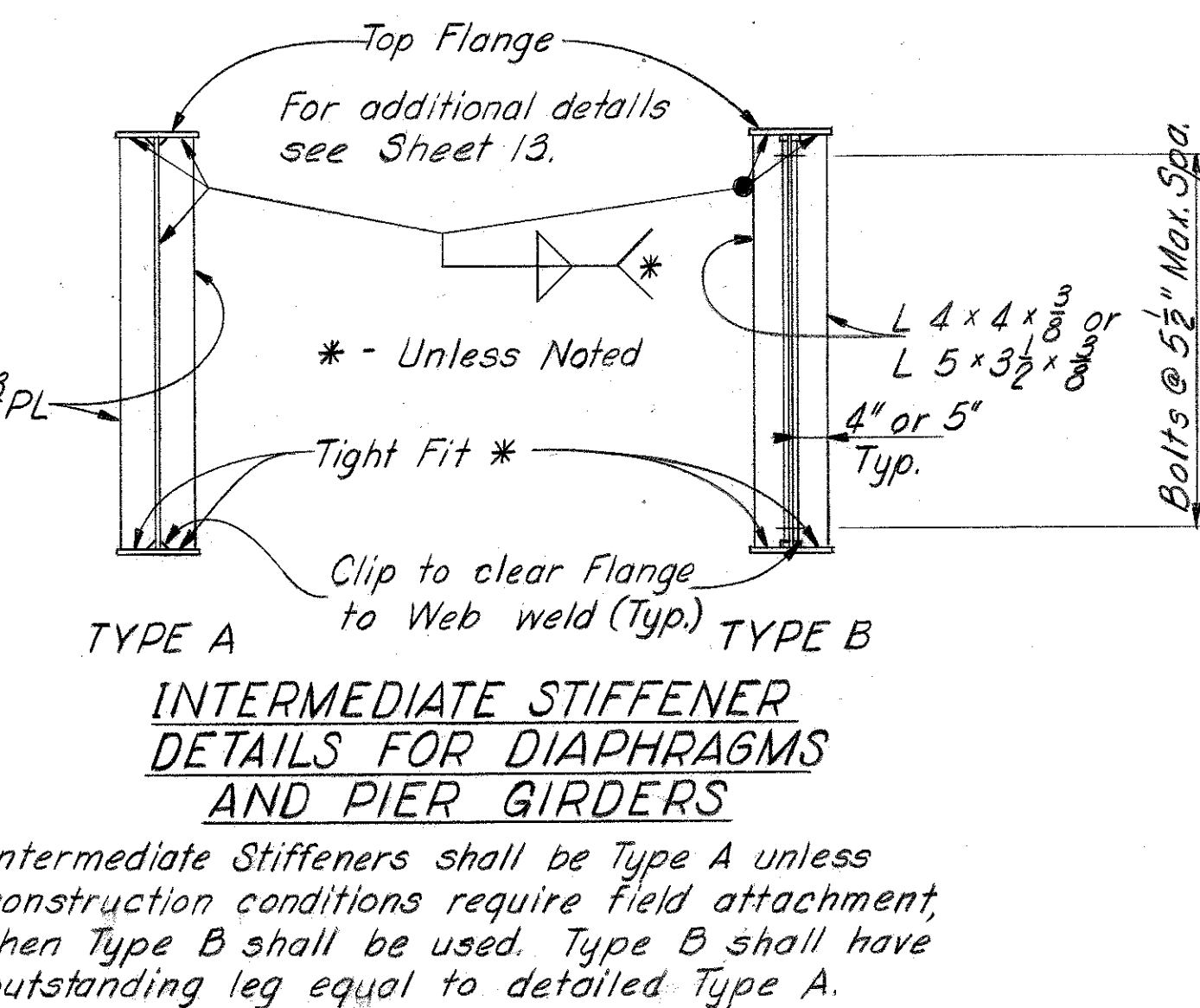
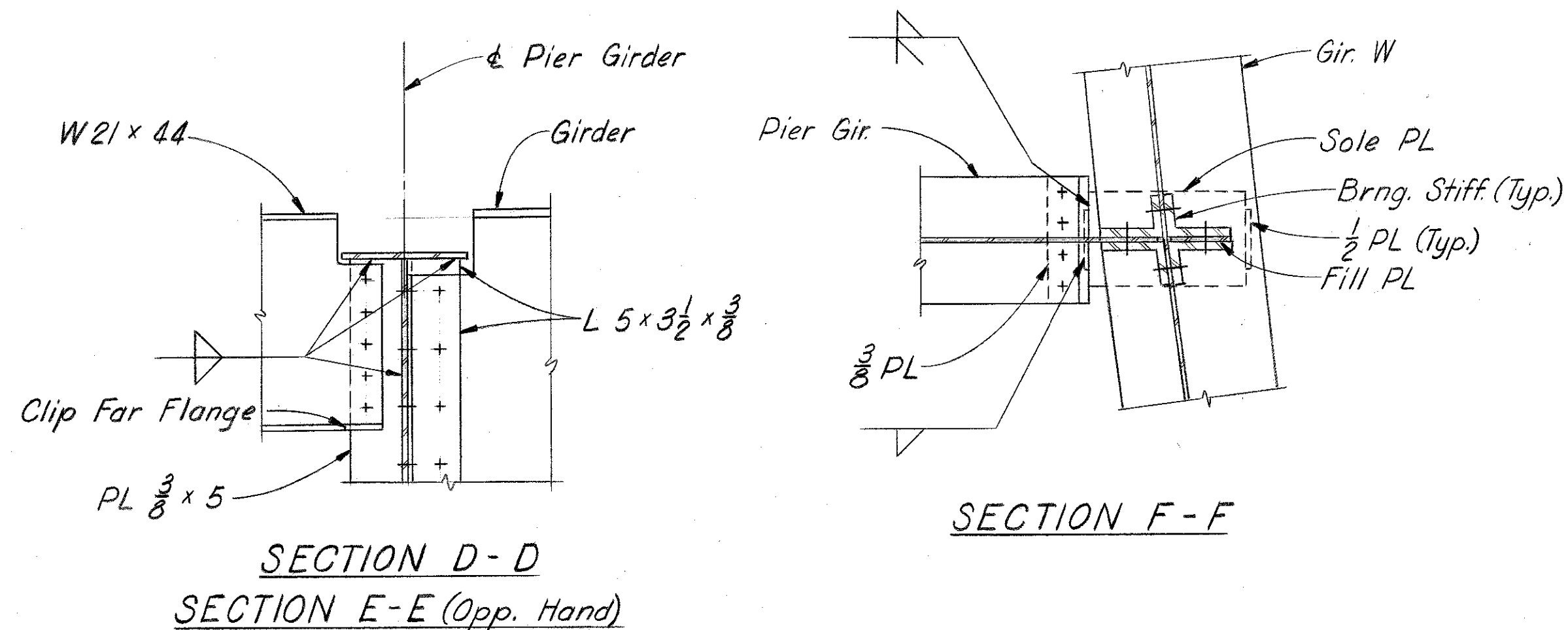
PIER GIRDERS AND
STRUT GIRDER
STRUCTURAL STEEL

UPDATE DATE
LETTING DATE

DESIGNED BY	CHH	DATE	2/15
CHECKED BY	DMB	DATE	3-75
TRACED BY	ELW	DATE	
DESIGNED BY	CHH	DATE	2/15
CHECKED BY	DMB	DATE	3-75
TRACED BY	ELW	DATE	



NOTES:
 (220A) - indicates point shown on Framing Plan, Sheets 9 and 10.
 For Bearing Stiffener Details see Sheet 14.
 For additional notes see Sheet 11.



**DIAPHRAGMS AND PIER GIRDERS
 STRUCTURAL STEEL**

COMMONWEALTH OF KENTUCKY
 BUREAU OF HIGHWAYS
 FRANKFORT
 COUNTY OF
JEFFERSON
 LOUISVILLE-LEXINGTON
 ROAD

STATION 267+00
P. E. PROJECT NO. 164-2(92)

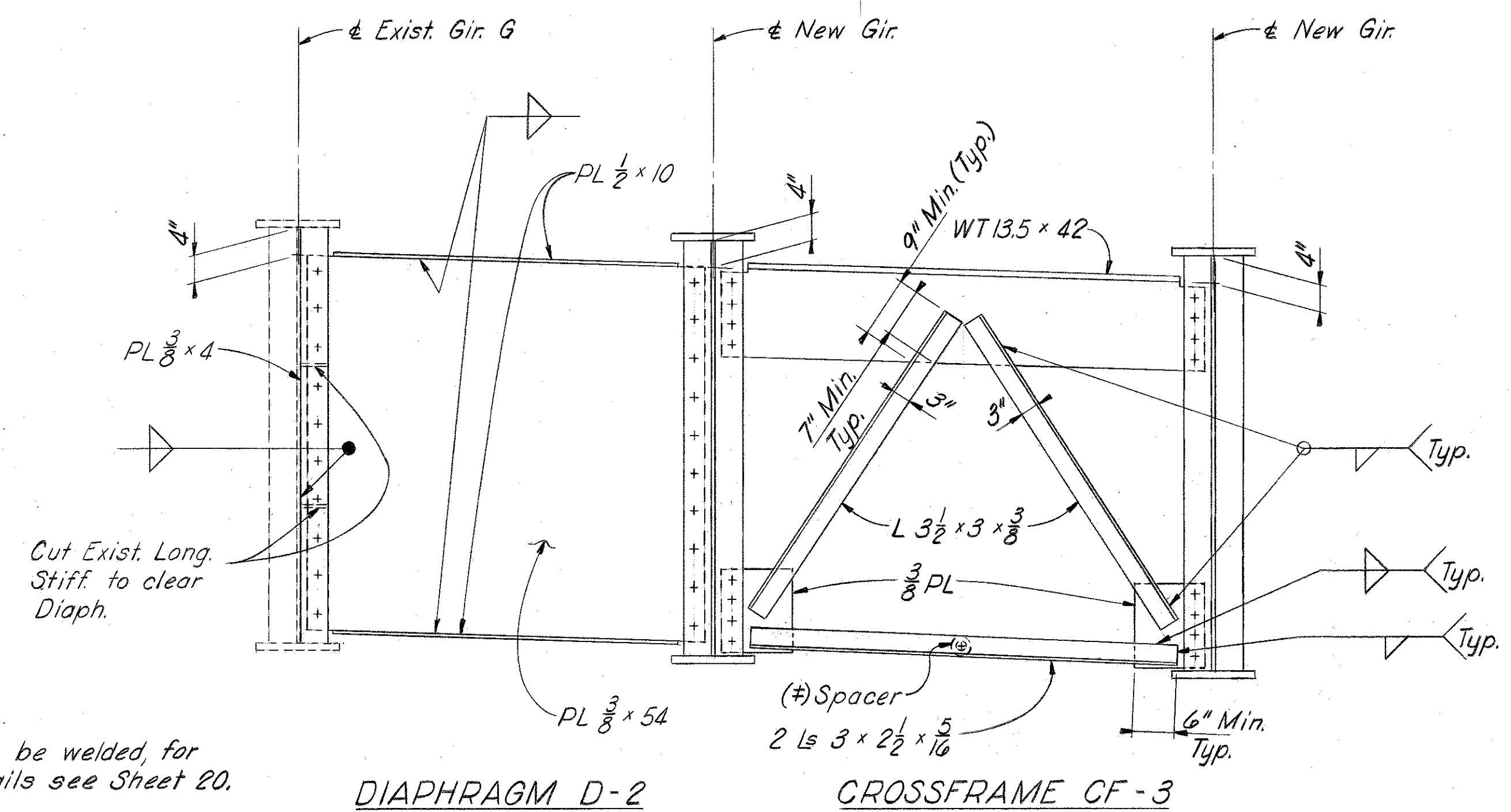
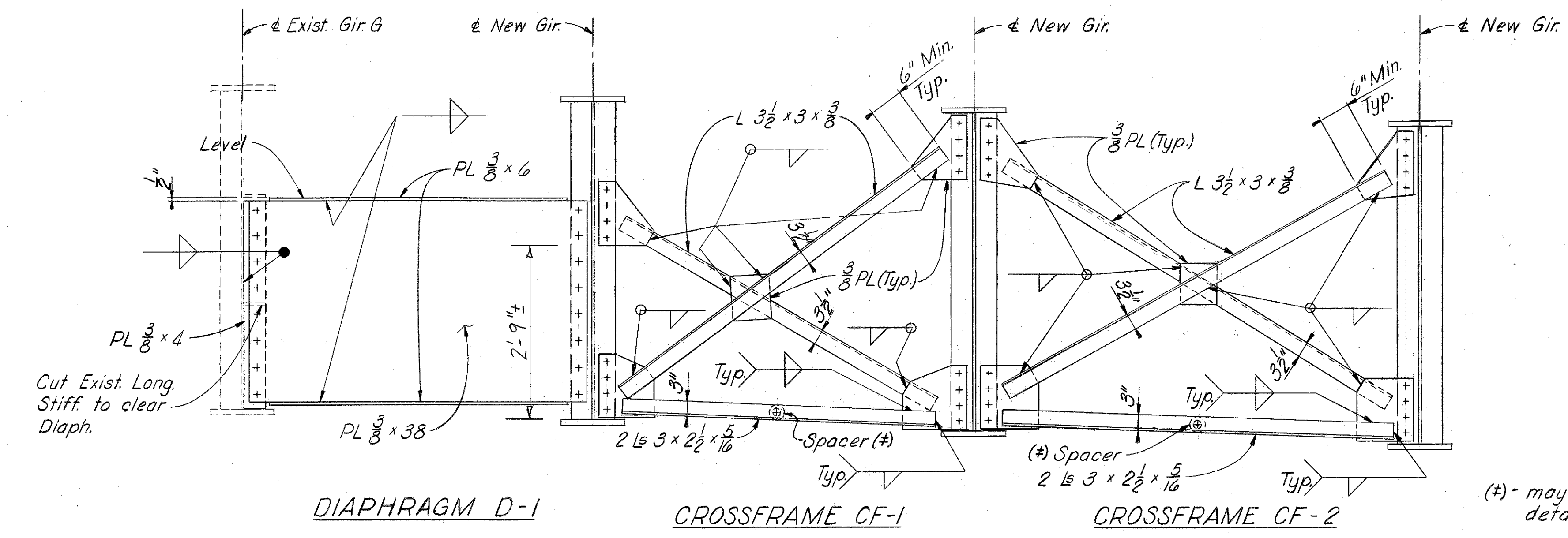
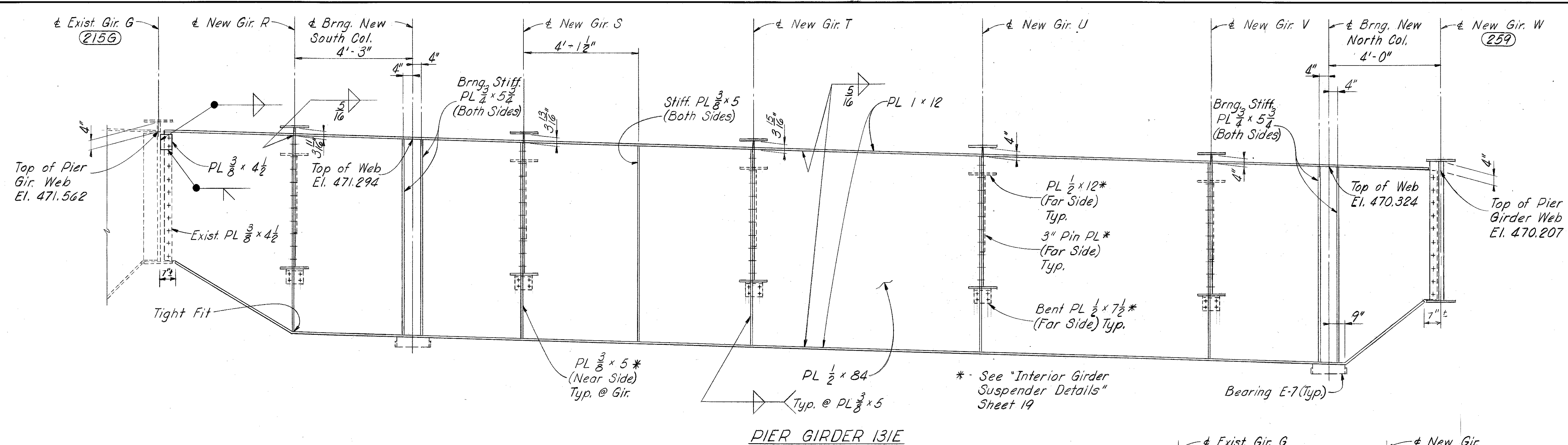
CONSTRUCTION PROJECT NO.
MAINTENANCE PROJECT NO.
DRAWING NO.
19238

SHEET 17

471.562

UPDATE DATE
LETTING DATE

DESIGNED BY	DATE	REVISION	DATE	REVISION	DATE
DMB	2/75				
ELW	3/75				
DMB					
ELW					



(*) - may be welded, for details see Sheet 20.

NOTES:
For Intermediate Stiffener Details see Sht. 17.
For Bearing Stiffener Details see Sheet 14.
(215G) - indicates point shown on Framing Plan, Sheets 9 and 10.
For additional notes see Sheet 11.

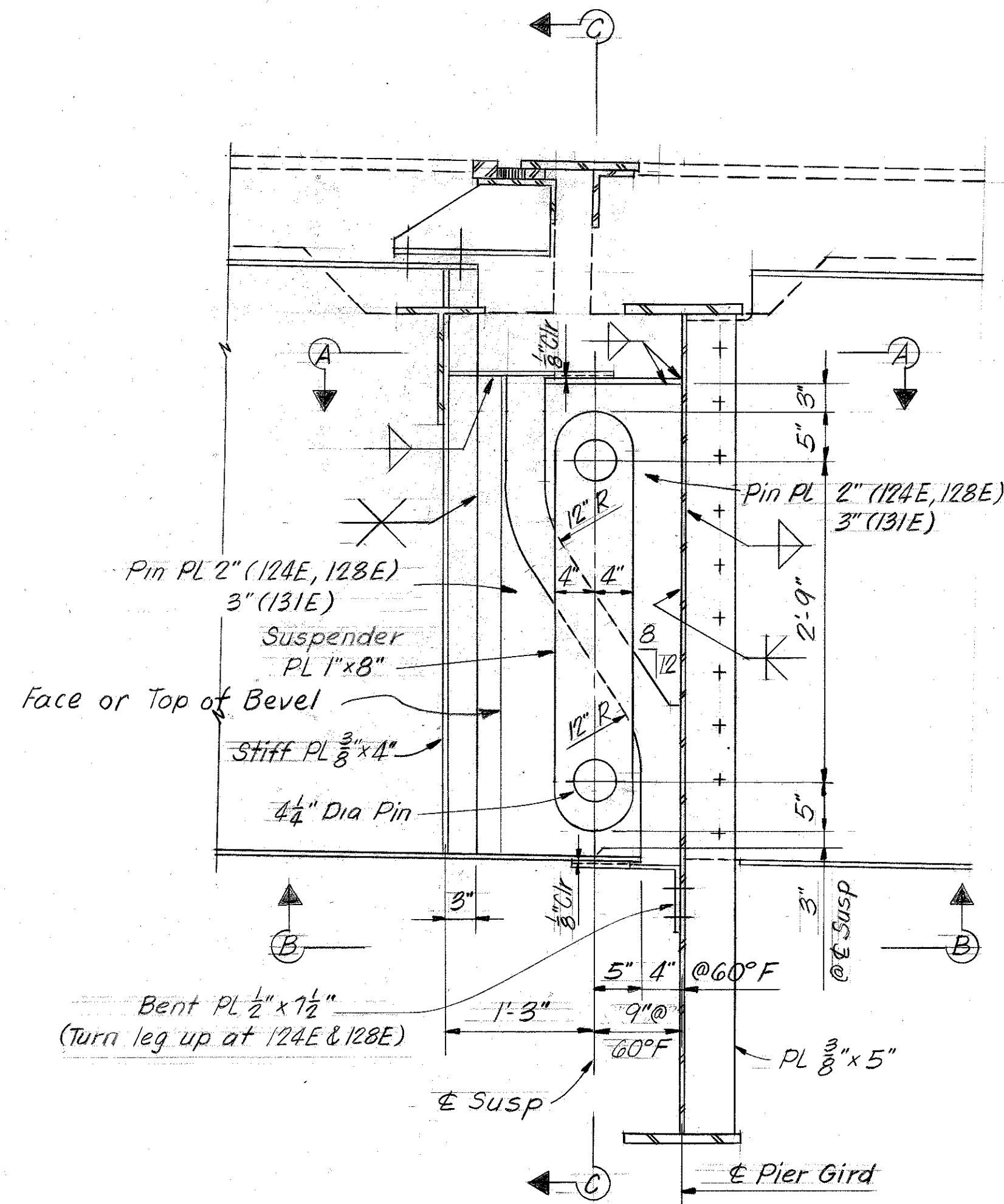
SHEET 18

DIAPHRAGMS AND
PIER GIRDERS
STRUCTURAL STEEL

COMMONWEALTH OF KENTUCKY		
BUREAU OF HIGHWAYS		
FRANKFORT COUNTY OF		
JEFFERSON LOUISVILLE-LEXINGTON		
ROAD		
STATION 267+00	P. E. PROJECT NO. I64-2(92)	DRAWING NO.
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.	19238

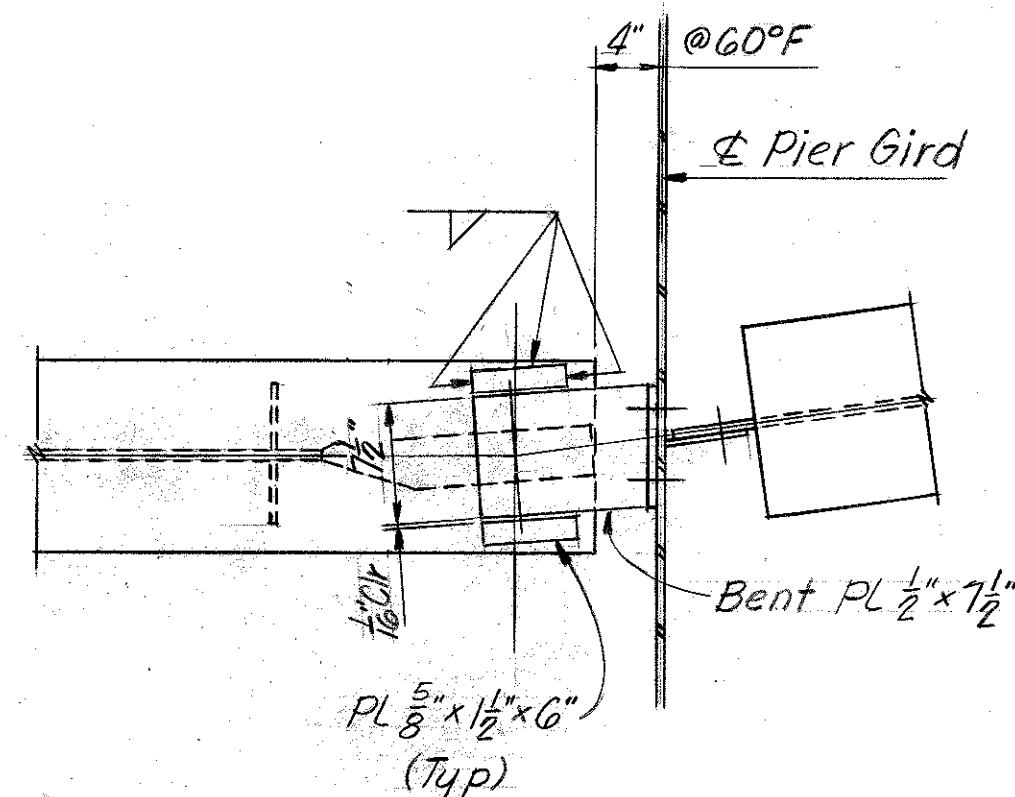
UPDATE DATE
LETTERING DATE

DESIGNED BY	DATE	REVISION	DATE	REVISION	DATE
BY	2-75	BY	4-75	BY	
CHECKED BY	DMB	CHECKED BY	ELU	CHECKED BY	
TRACED BY	DMB	TRACED BY		TRACED BY	

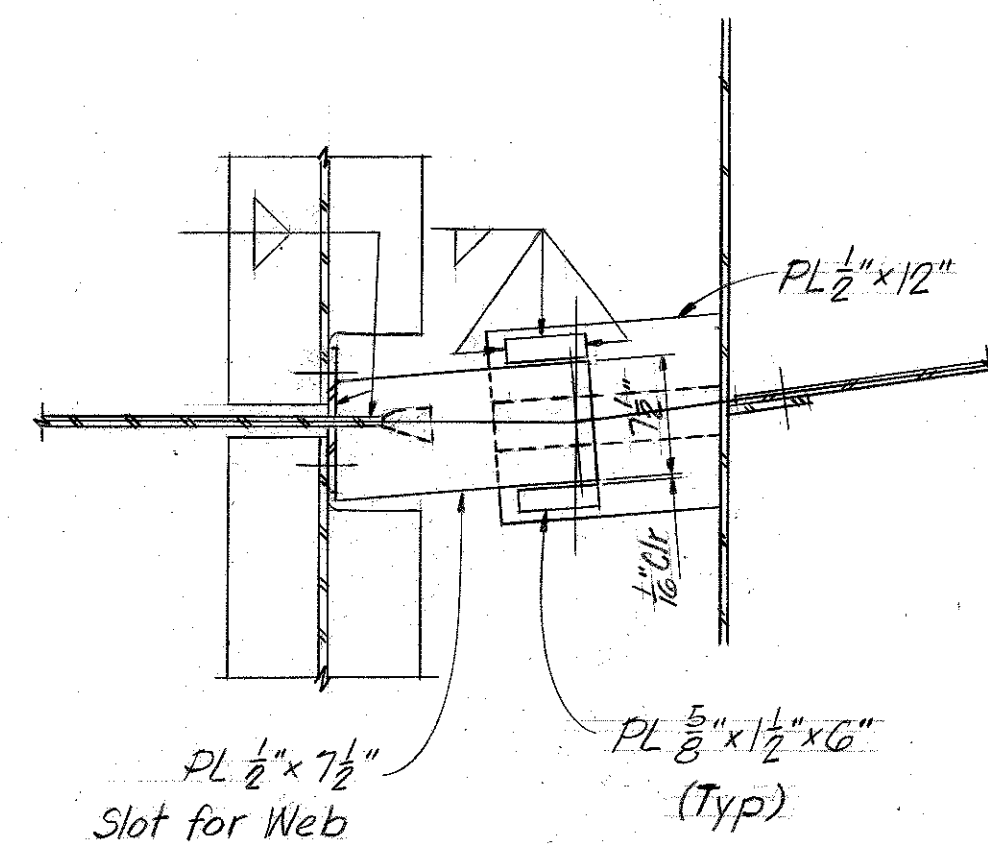


TYPICAL SUSPENDER DETAIL AT INTERIOR GIRDER

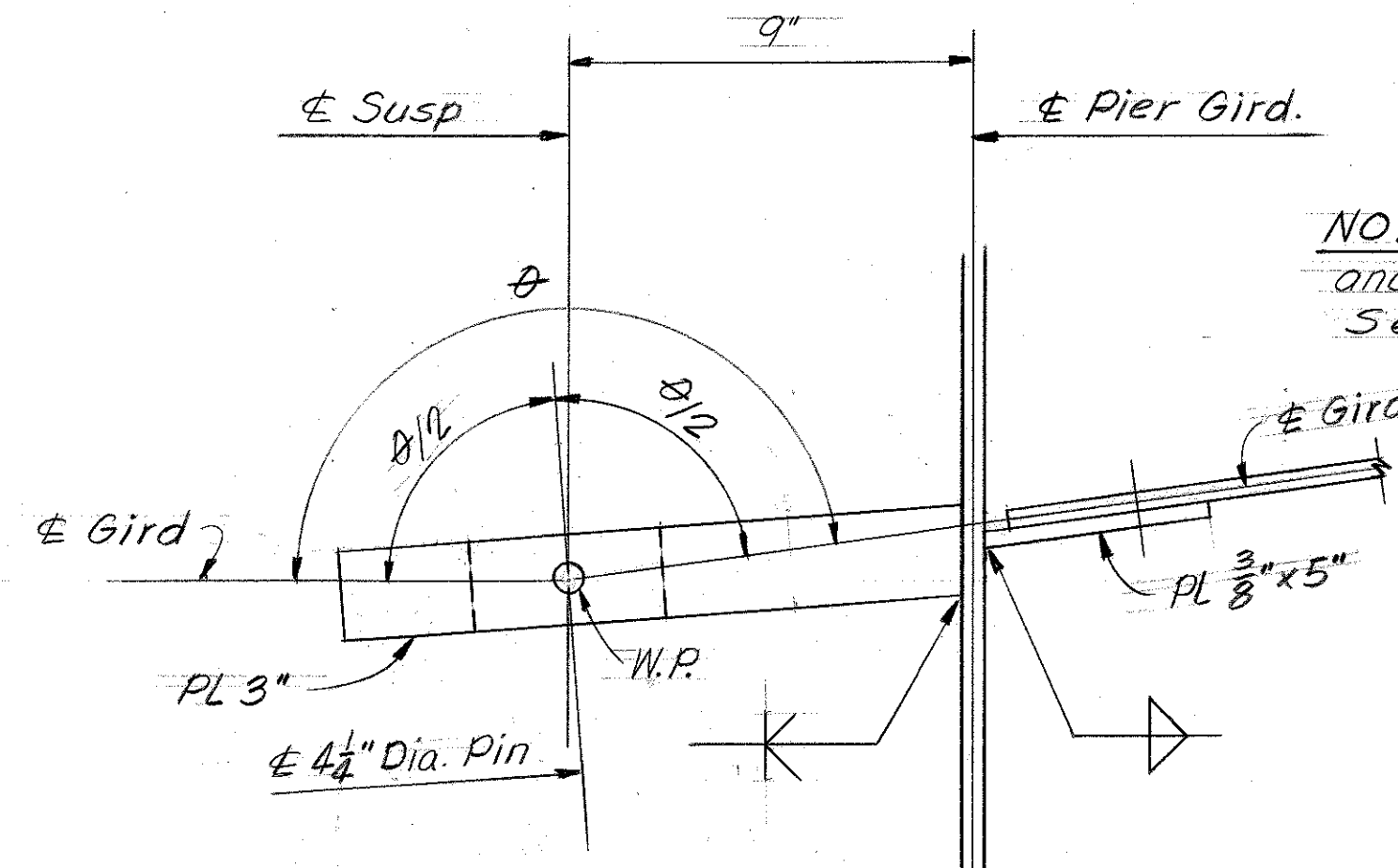
(Drawn at Pier Girder 131E, Girds R thru V
Pier Girder 128E, Girder R similar
Pier Girder 124E, Girder W similar)



SECTION B-B

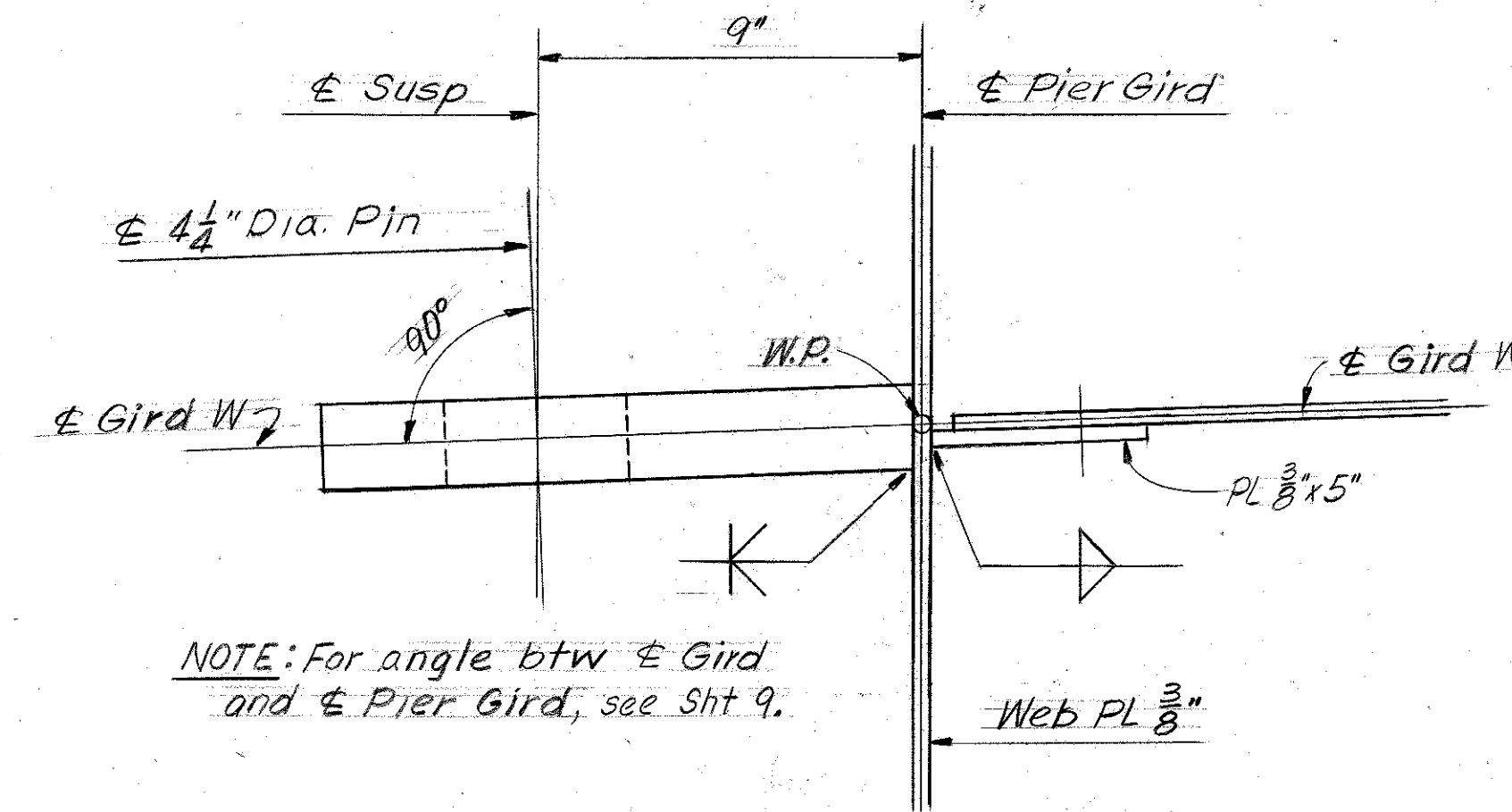


SECTION A-A

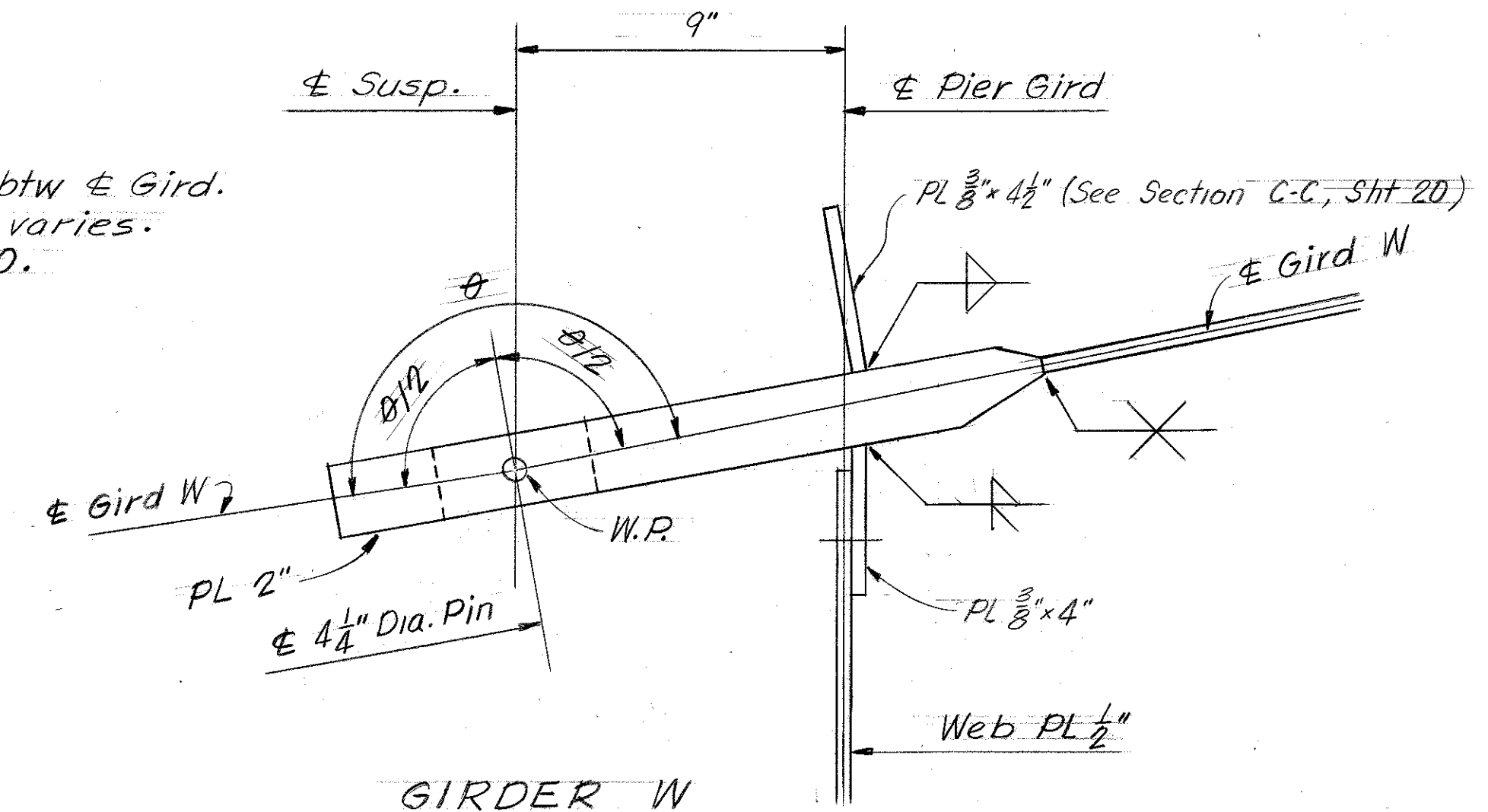


GIRDERS R THRU V
(Pier Girder 128E, Girder R similar)

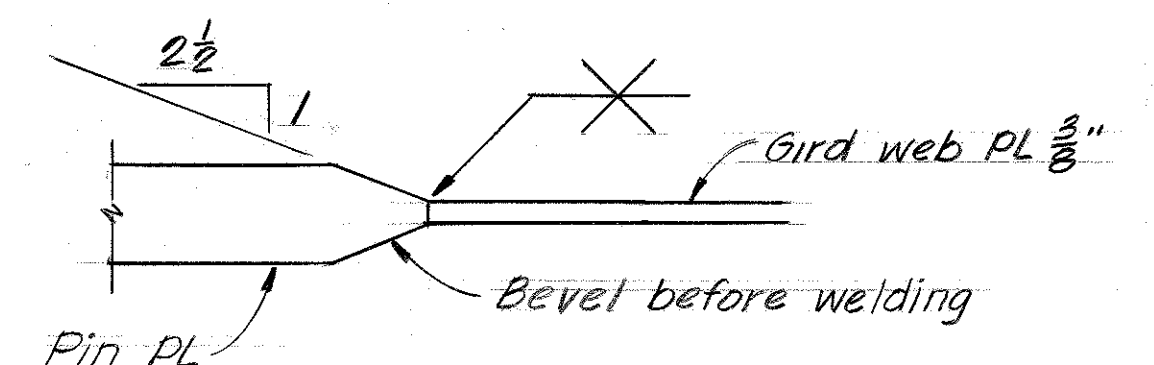
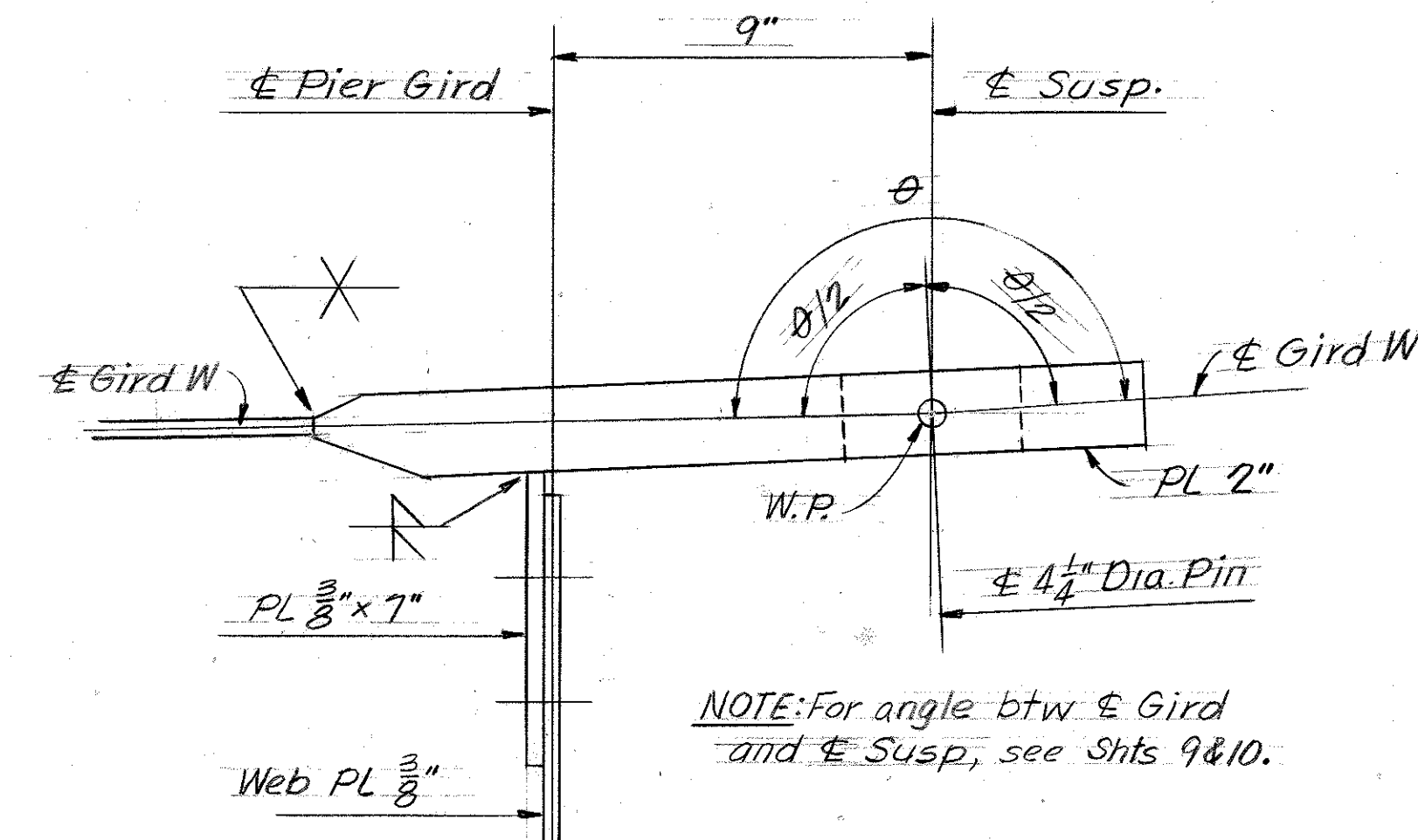
SUSPENDER ALIGNMENT AT PIER GIRDER 131E



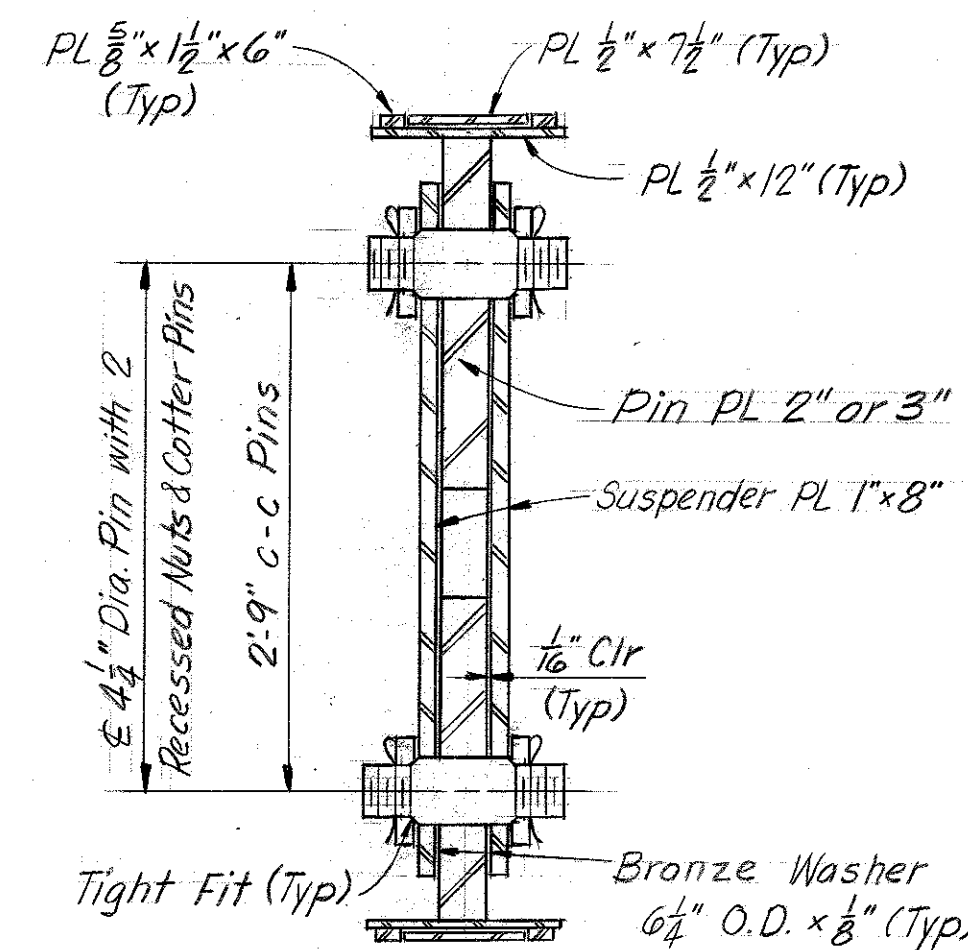
SUSPENDER ALIGNMENT AT PIER GIRDER 124E



SUSPENDER ALIGNMENT AT PIER GIRDER 128E
GIRDER W



SHOP WEB SPLICE DETAIL



SECTION C-C

SUSPENDER DETAILS
STRUCTURAL STEEL

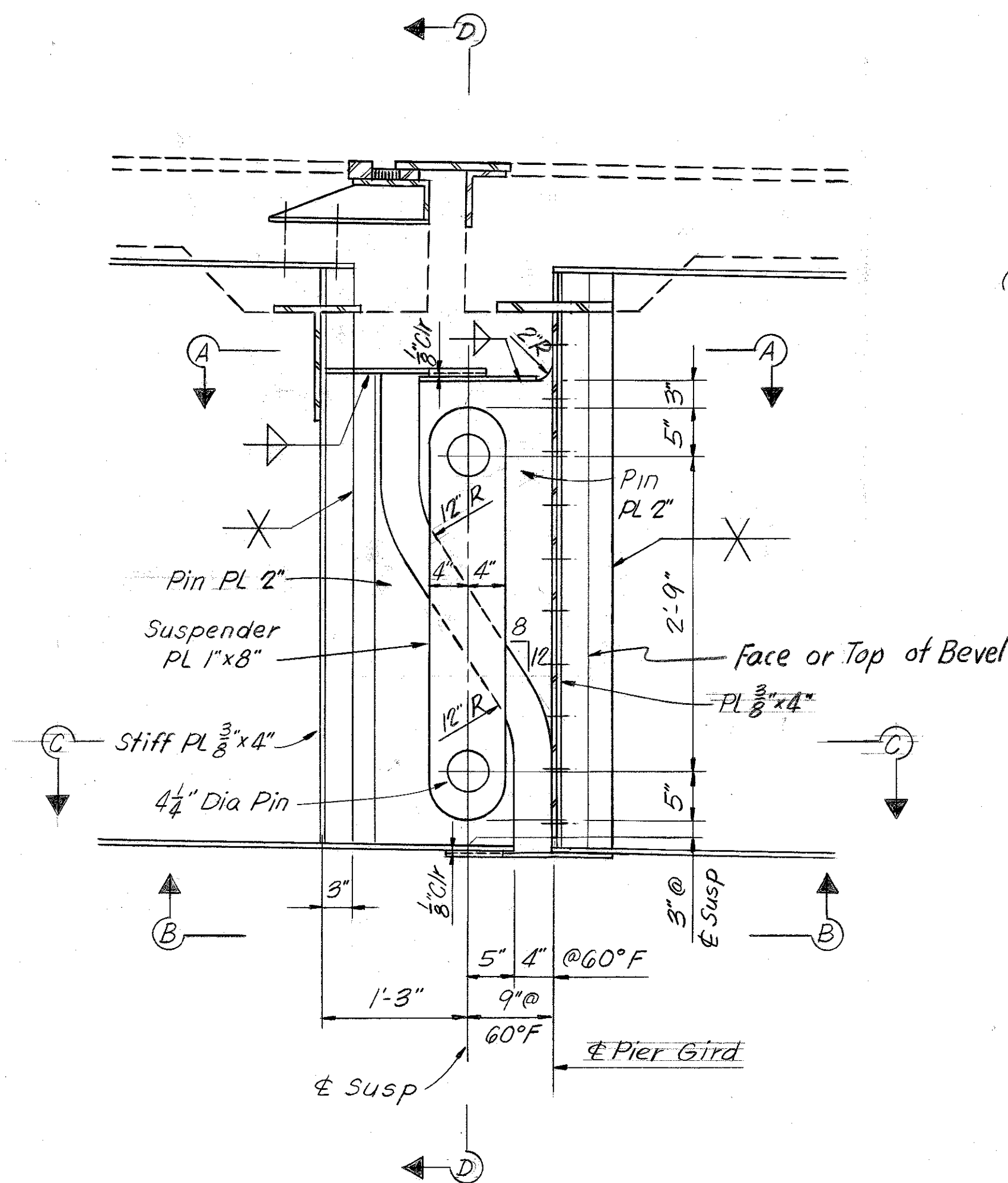
For Notes see Sht 11.

SHEET 19

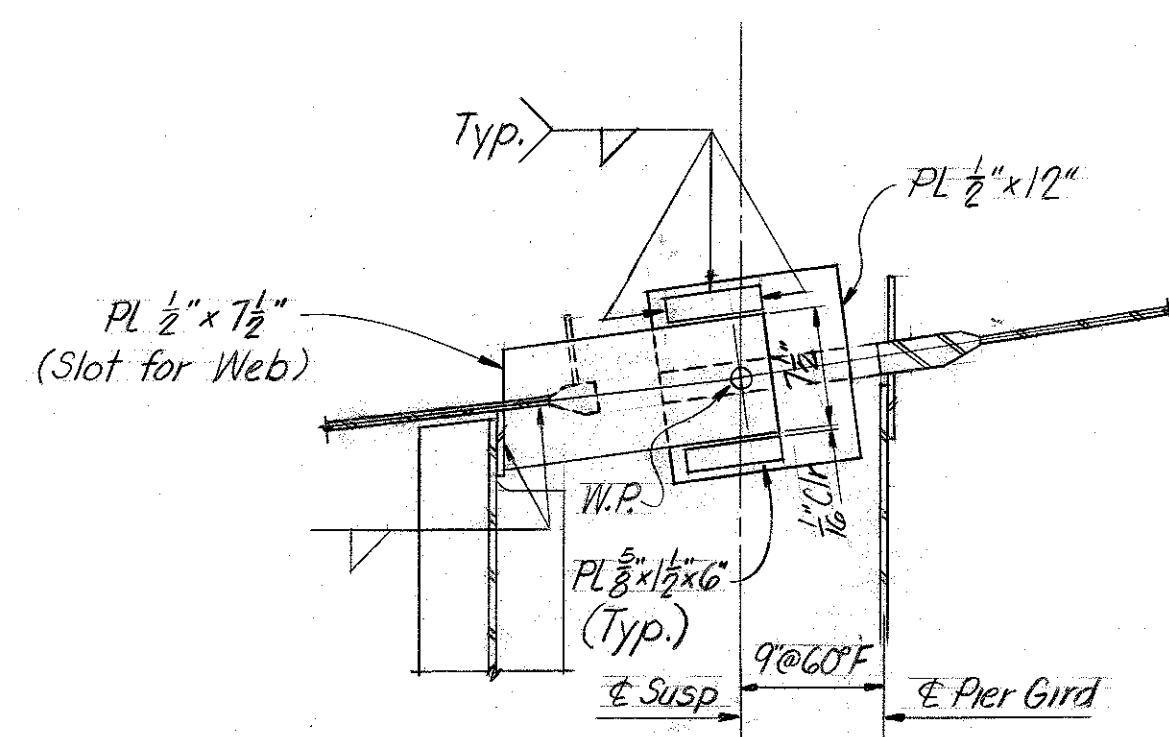
COMMONWEALTH OF KENTUCKY	
BUREAU OF HIGHWAYS	
FRANKFORT COUNTY OF	
JEFFERSON	
LOUISVILLE-LEXINGTON	
ROAD	
STATION 267+00	P. E. PROJECT NO. I64-2(92)
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.
	DRAWING NO.
	19238

UPDATE DATE
LETTING DATE

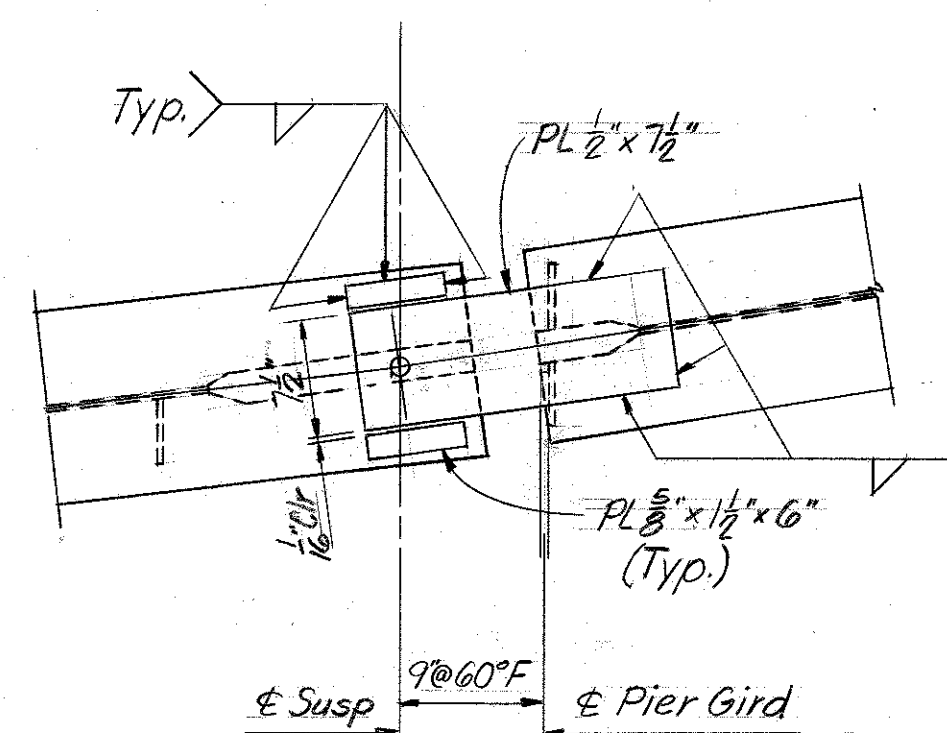
DESIGNED BY	DATE	REVISION	DATE	REVISION	DATE
RBS	2-75	DMB	4-75	DMB	
CHECKED BY	DATE	CHECKED BY	DATE	CHECKED BY	DATE
DMB		DMB		DMB	
TRACED BY	DATE	TRACED BY	DATE	TRACED BY	DATE



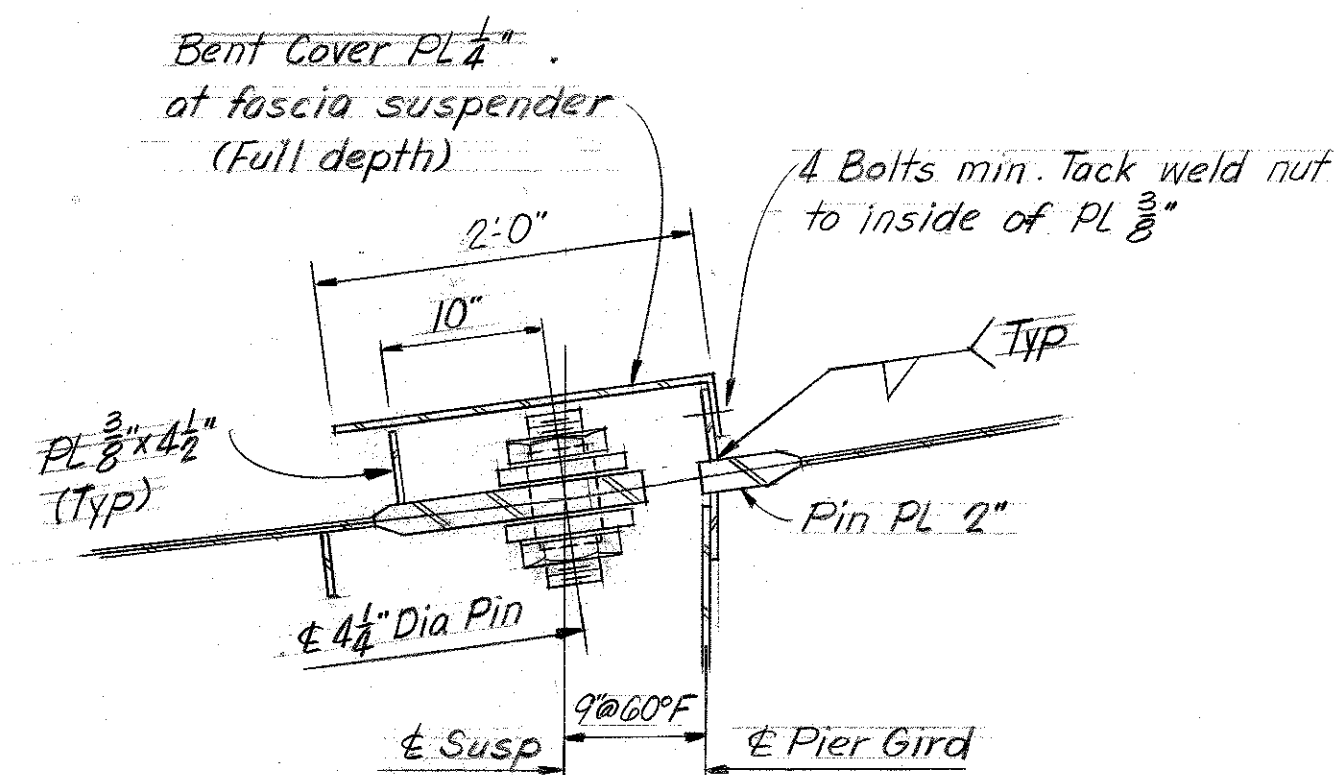
TYPICAL SUSPENDER DETAIL AT
FASCIA GIRDER
(Drawn at Pier Gird 131E; Pier Gird 128E similar)
Section D-D similar to Section C-C, Sht. 19



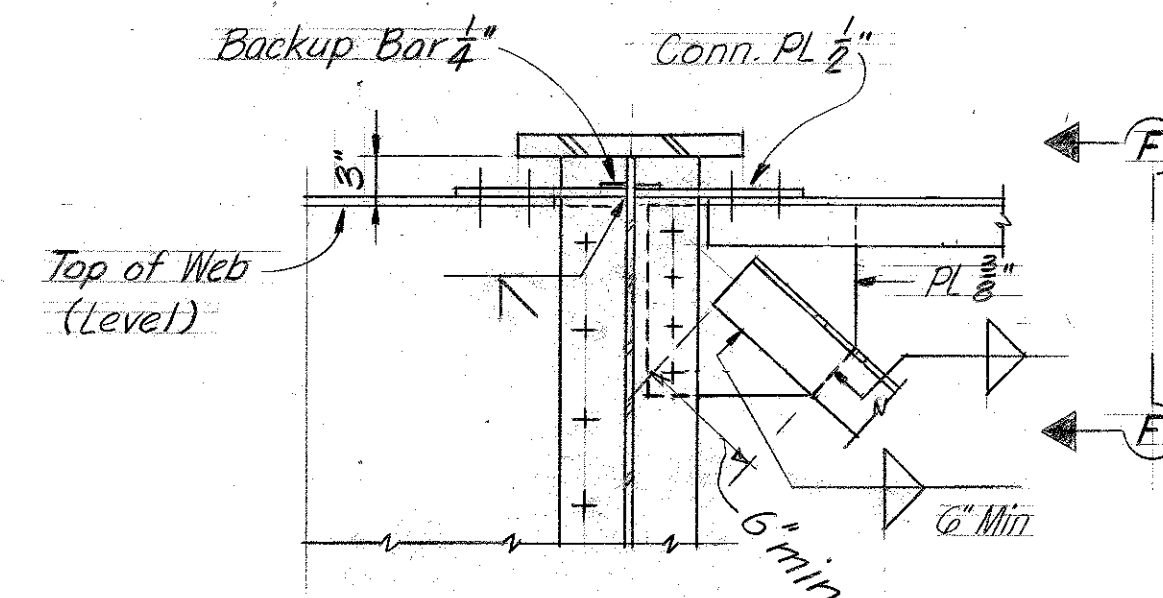
SECTION A-A



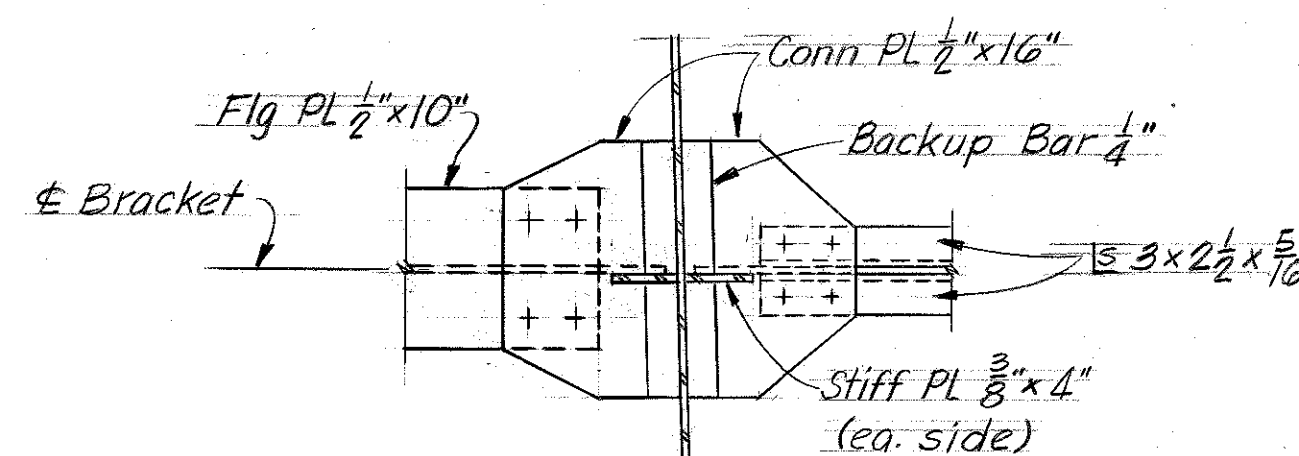
SECTION B-B



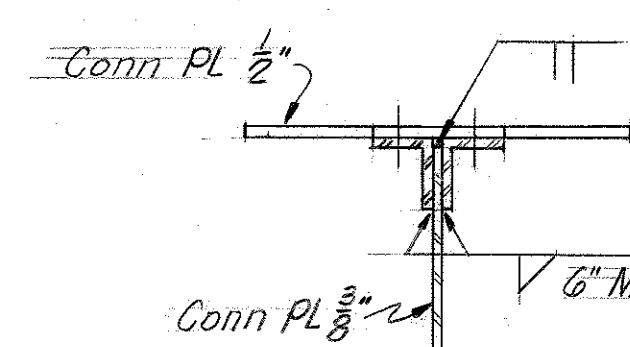
SECTION C-C



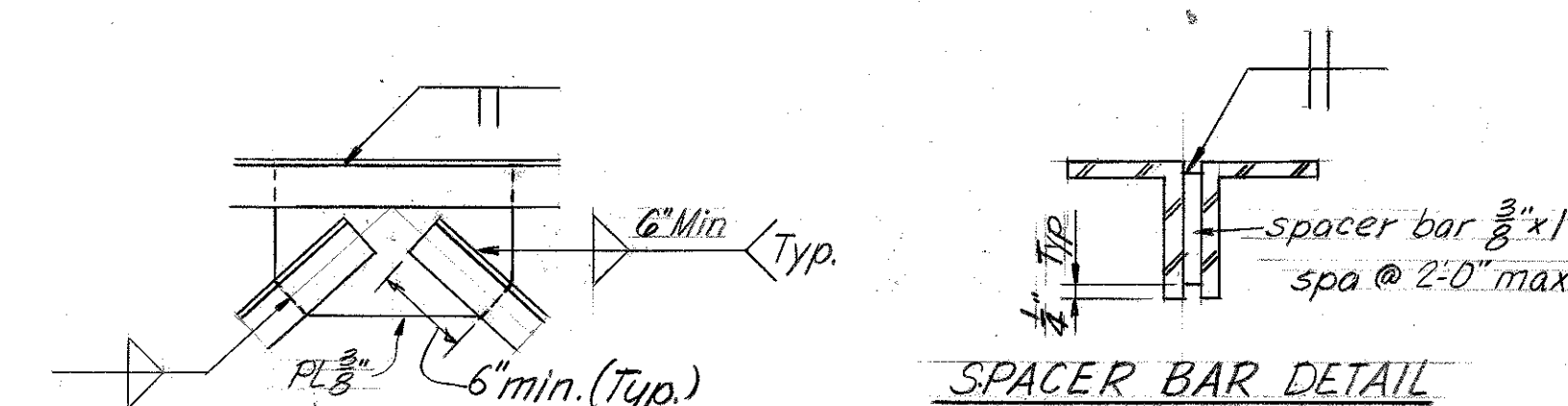
DETAIL A
(Detail B similar)
(Bott. Flg similar)



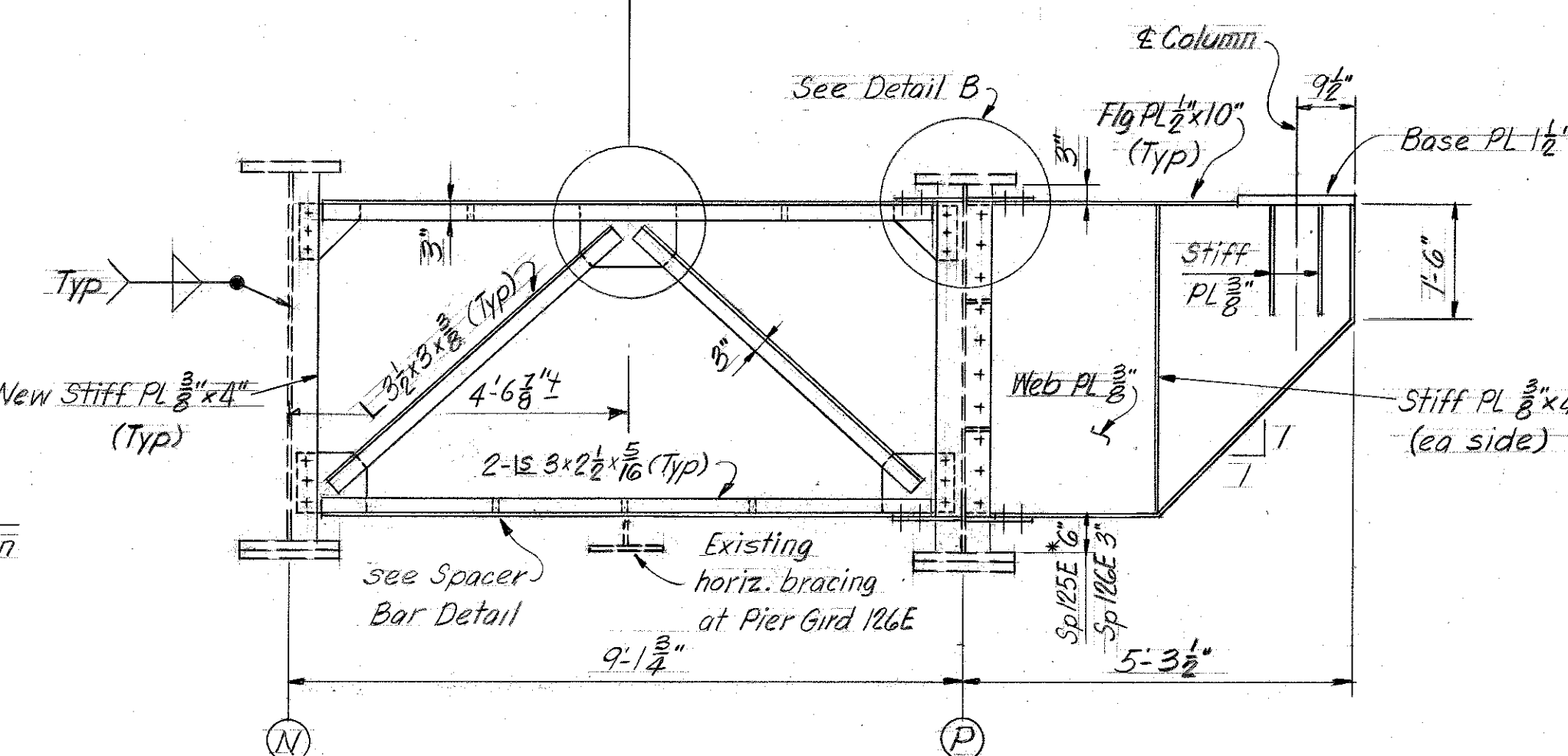
PLAN-BRACKET FLANGE CONN.
(Top shown; Bott. similar)



SECTION F-F

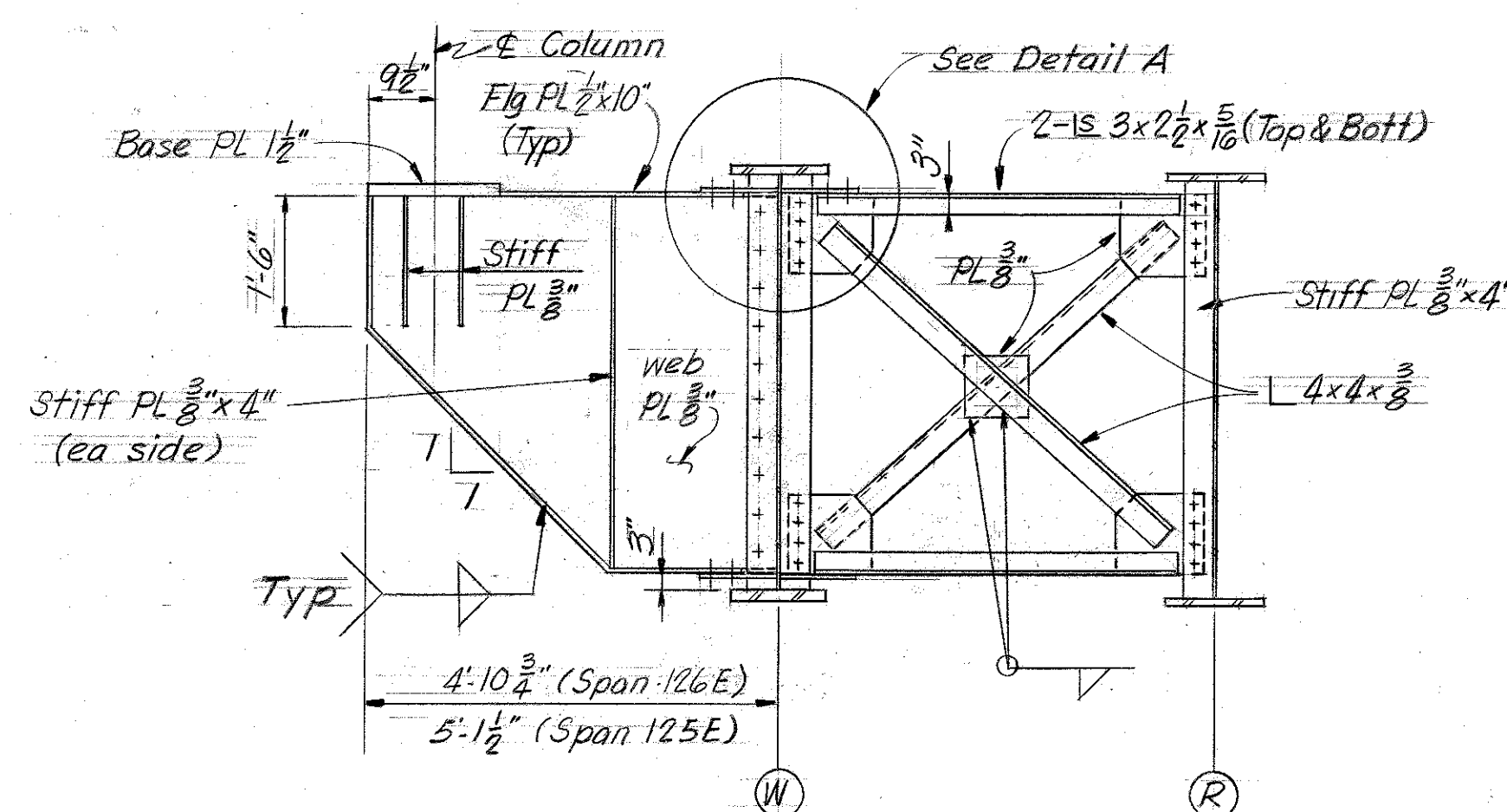


SPACER BAR DETAIL

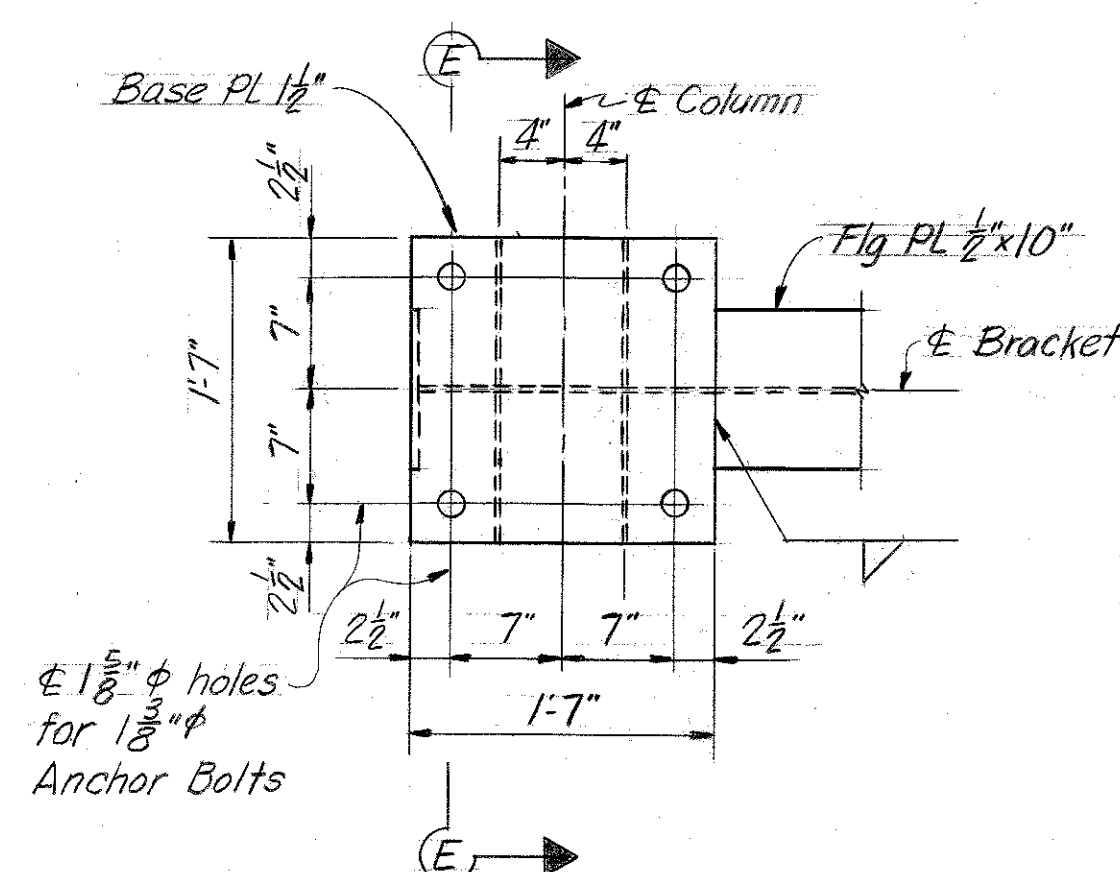


*Verify by field measurement to clear horiz. bracing at Pier Gird 126E.

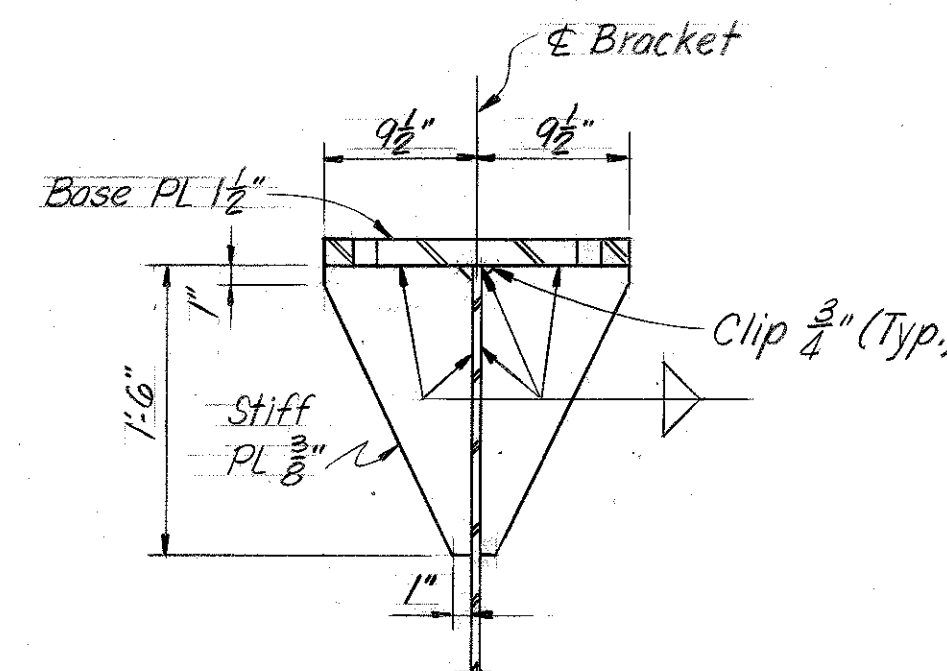
SIGN SUPPORT BRACKET & CROSSFRAME CF-4
SOUTH FASCIA



SIGN SUPPORT BRACKET & CROSSFRAME CF-5
NORTH FASCIA



PLAN-COLUMN BASE PLATE



SECTION E-E

SUSPENDER & SIGN
SUPPORT BRACKET DETAILS
STRUCTURAL STEEL

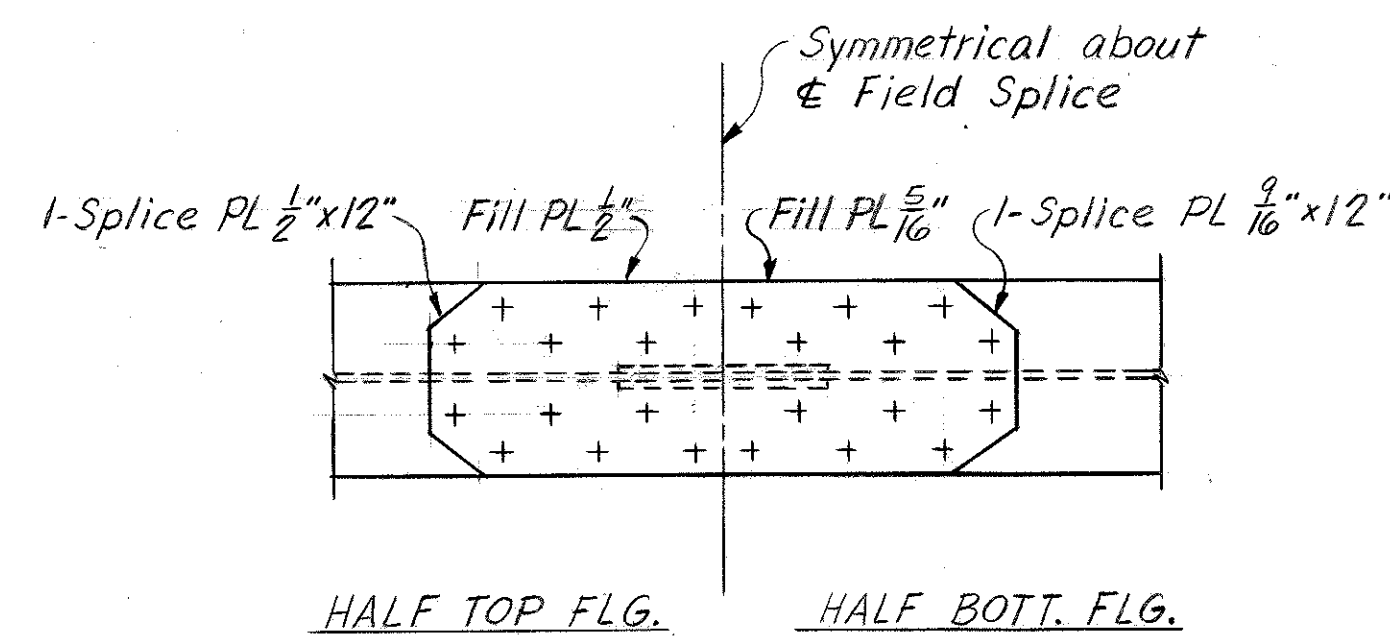
For Notes see Sht 11.

COMMONWEALTH OF KENTUCKY		
BUREAU OF HIGHWAYS		
FRANKFORT		
COUNTY OF		
JEFFERSON		
LOUISVILLE-LEXINGTON		
ROAD		
STATION 267+00	P. E. PROJECT NO. 164-2(92)	DRAWING NO.
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.	19238

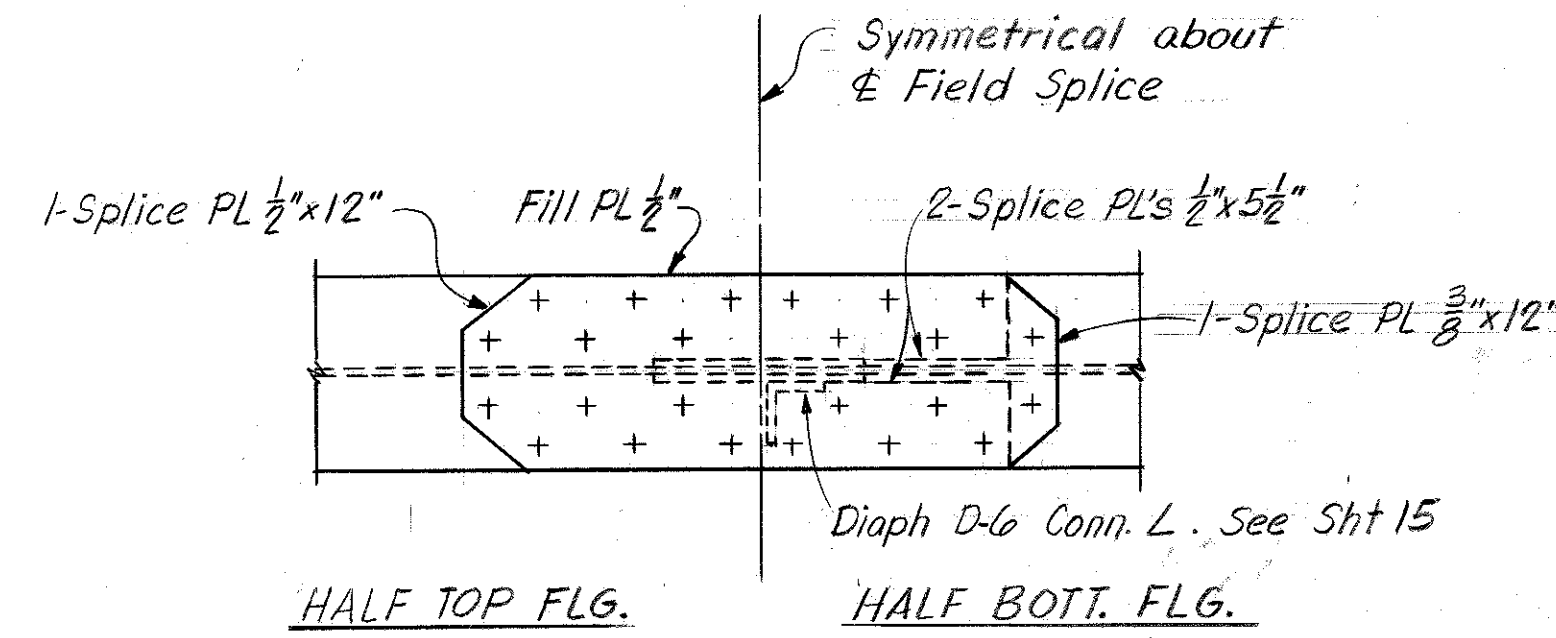
SHEET 20

UPDATE DATE
LETTING DATE

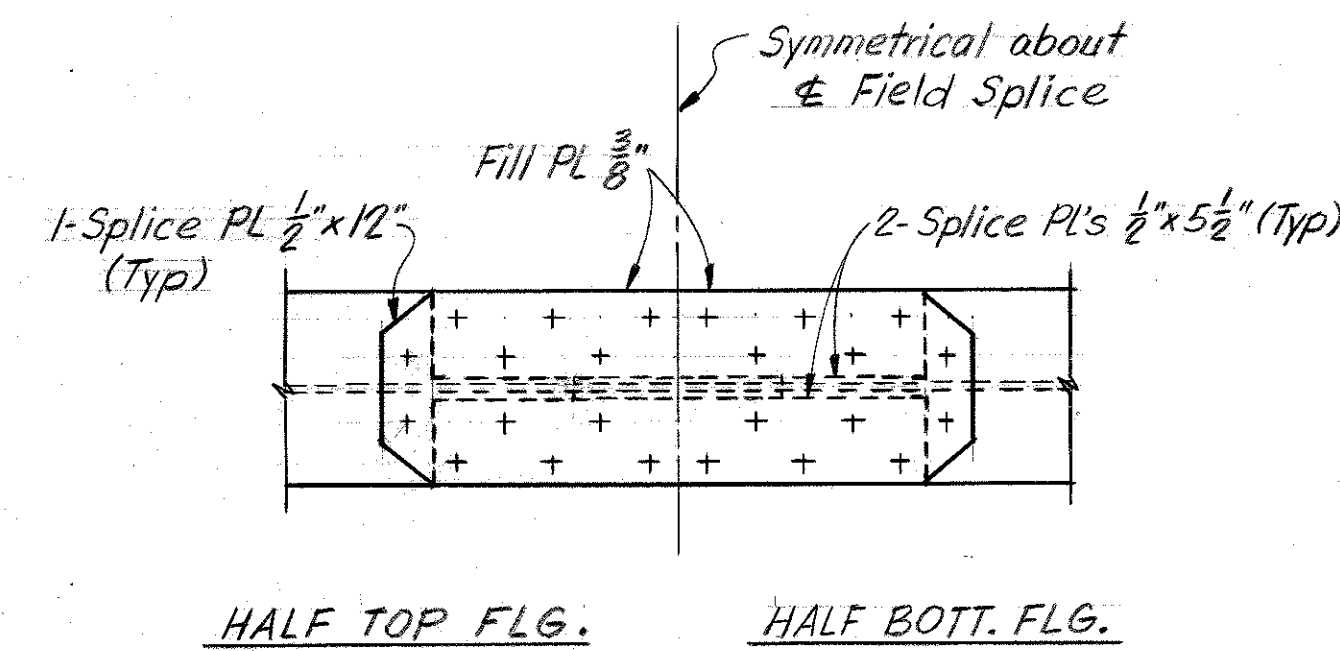
DESIGNED BY DMB CHECKED BY RBS DATE 11/74 REVISED BY ELW DATE 1/75 TRACED BY DMB



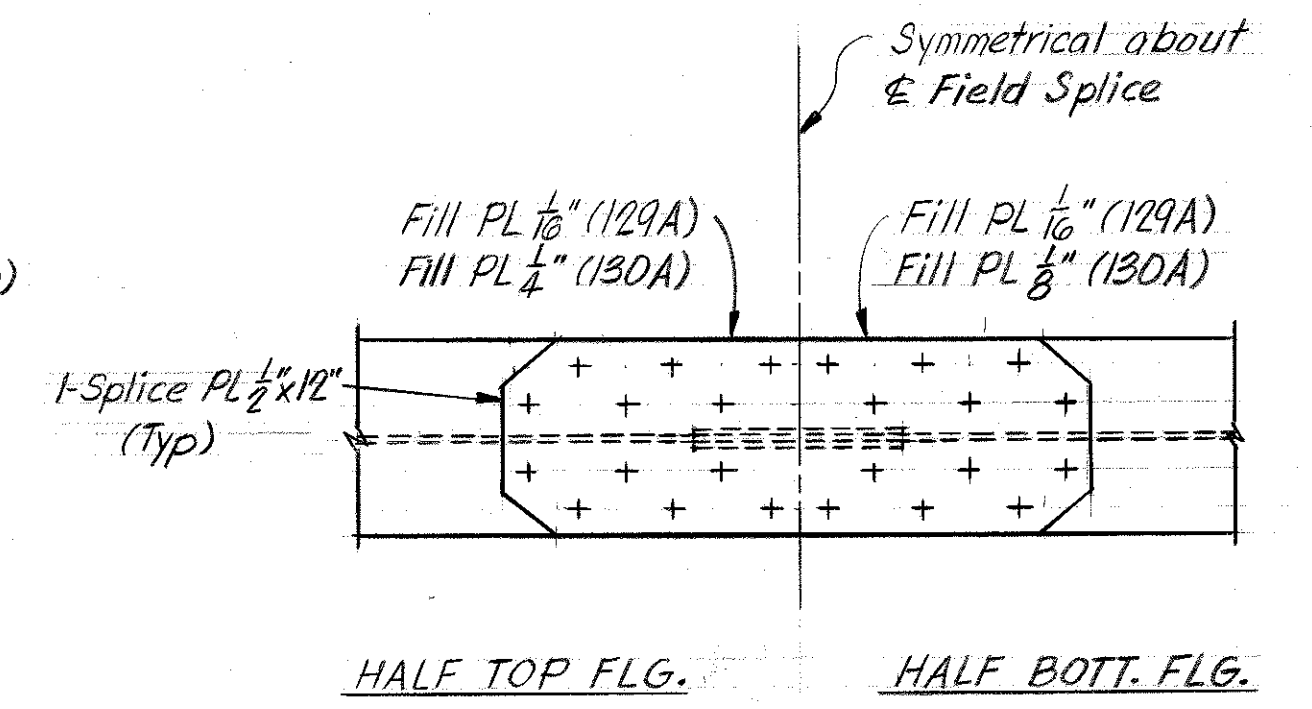
FIELD SPLICE ①



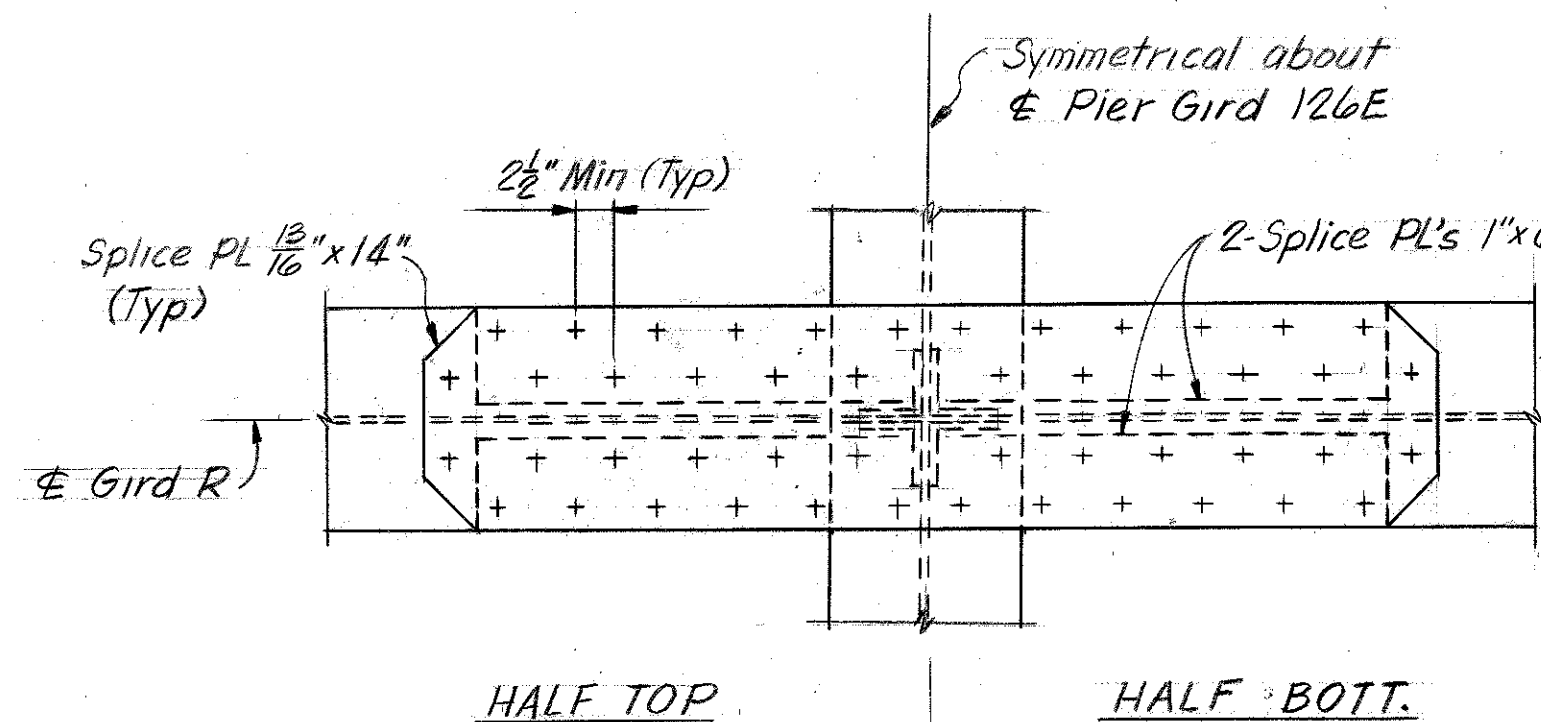
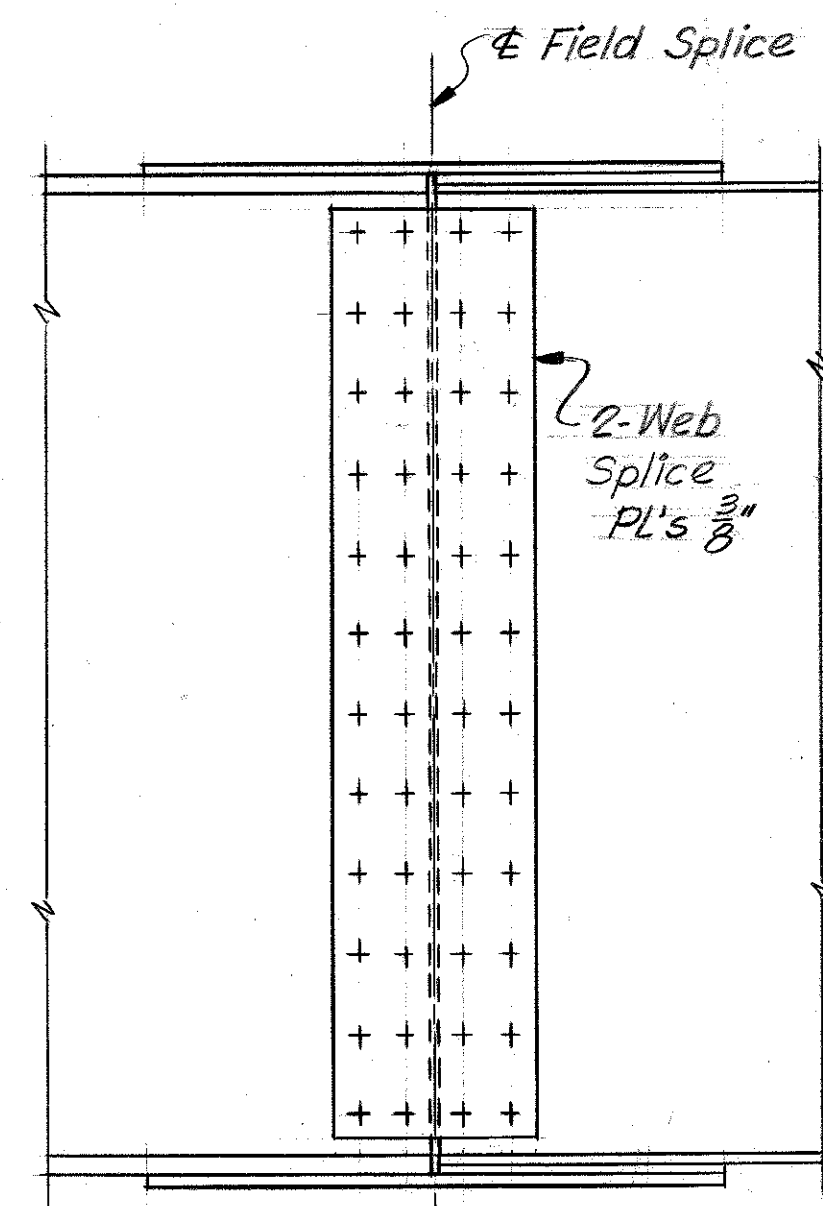
FIELD SPLICE ②



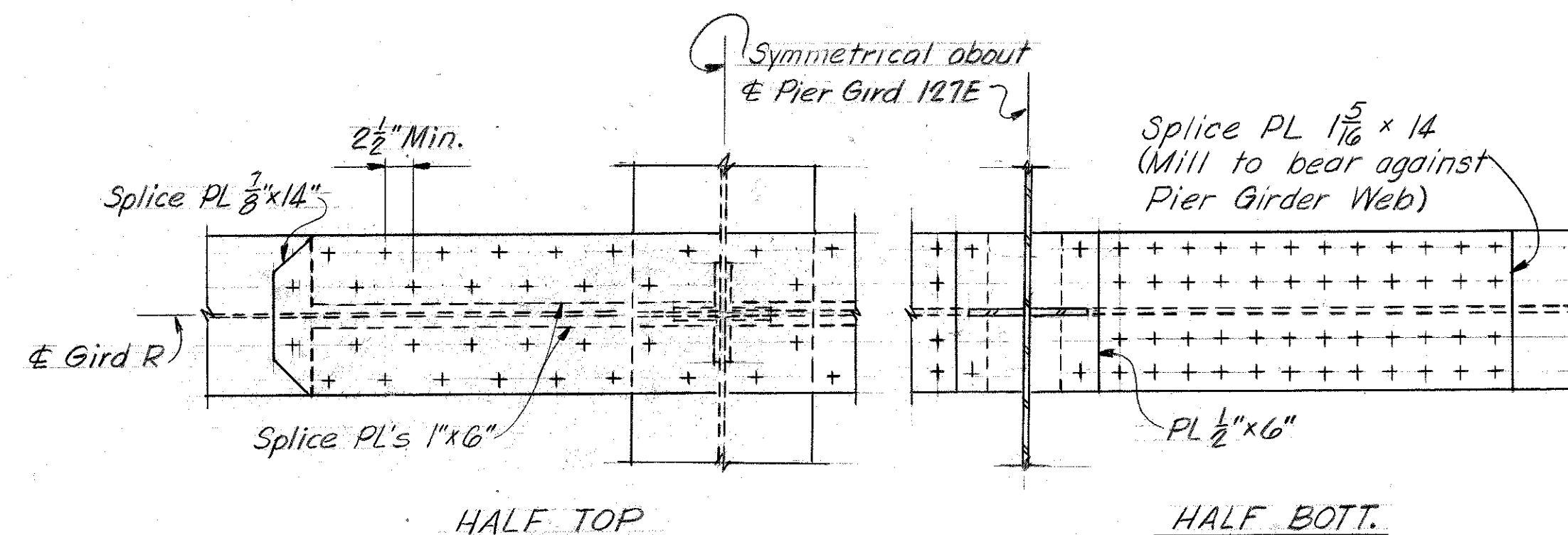
FIELD SPLICE ③



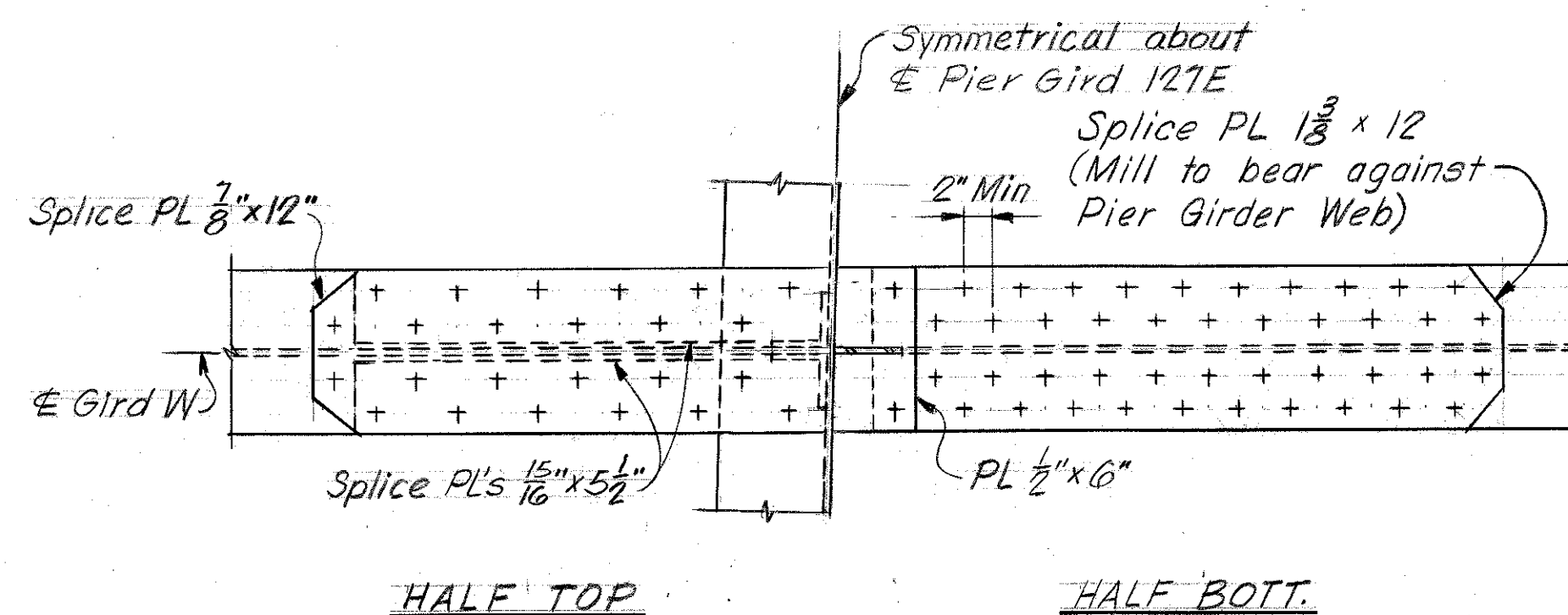
FIELD SPLICE ④



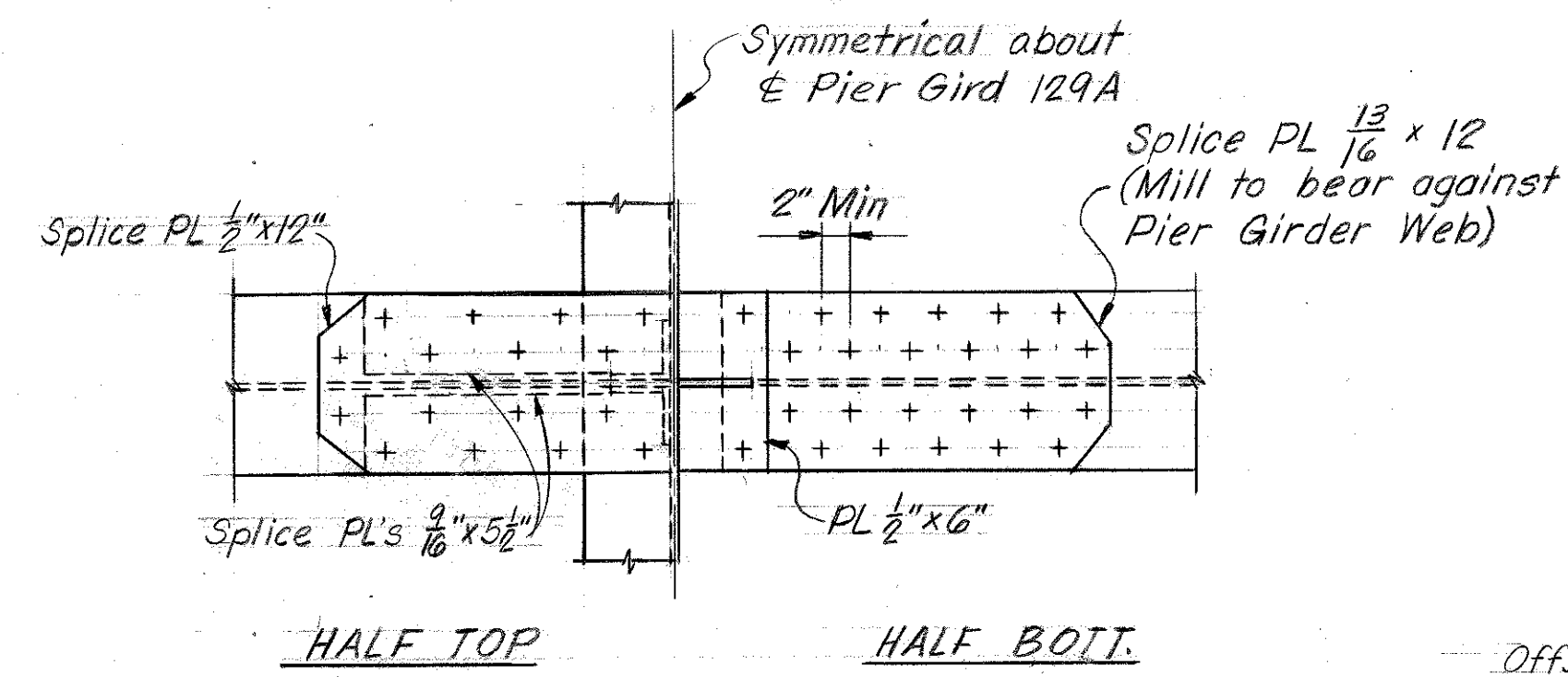
GIRD. R FLANGE SPLICE AT PIER GIRD. 126E



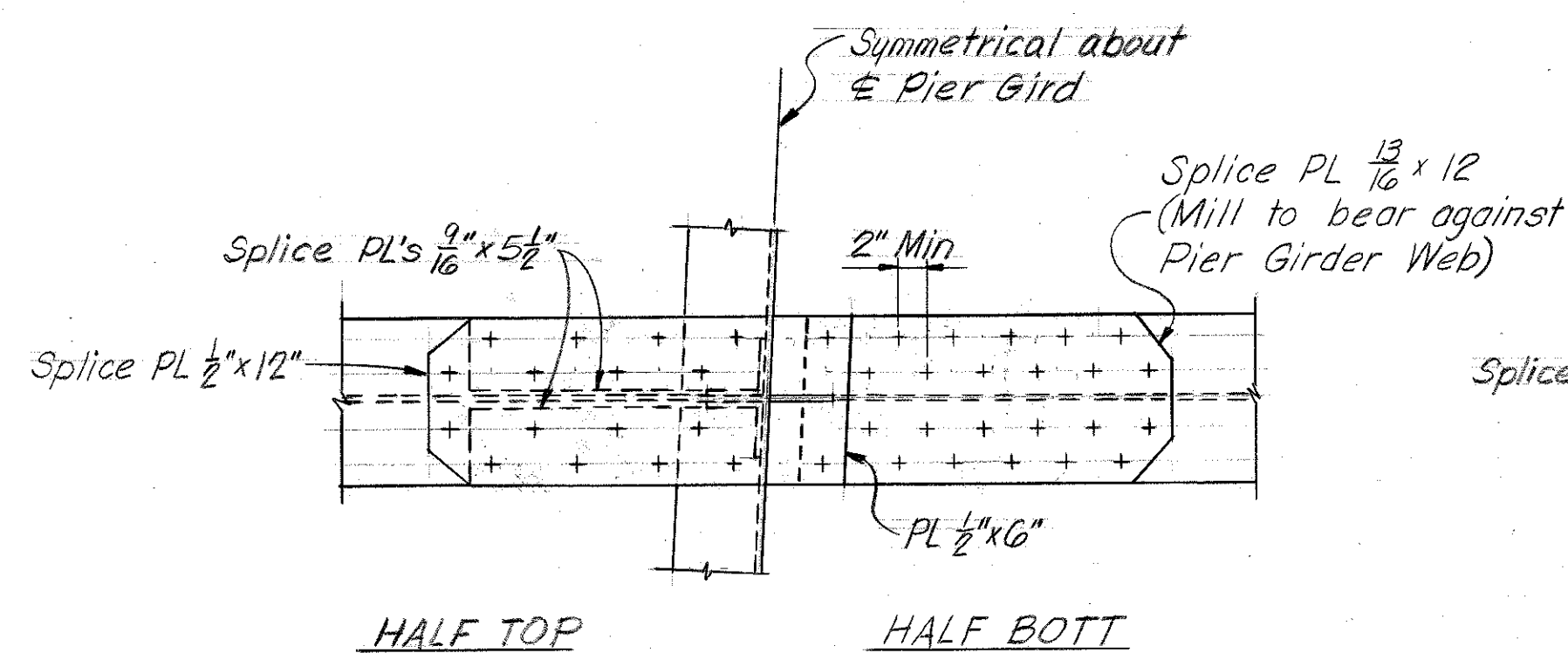
GIRD. R FLANGE SPLICE AT PIER GIRD. 127E



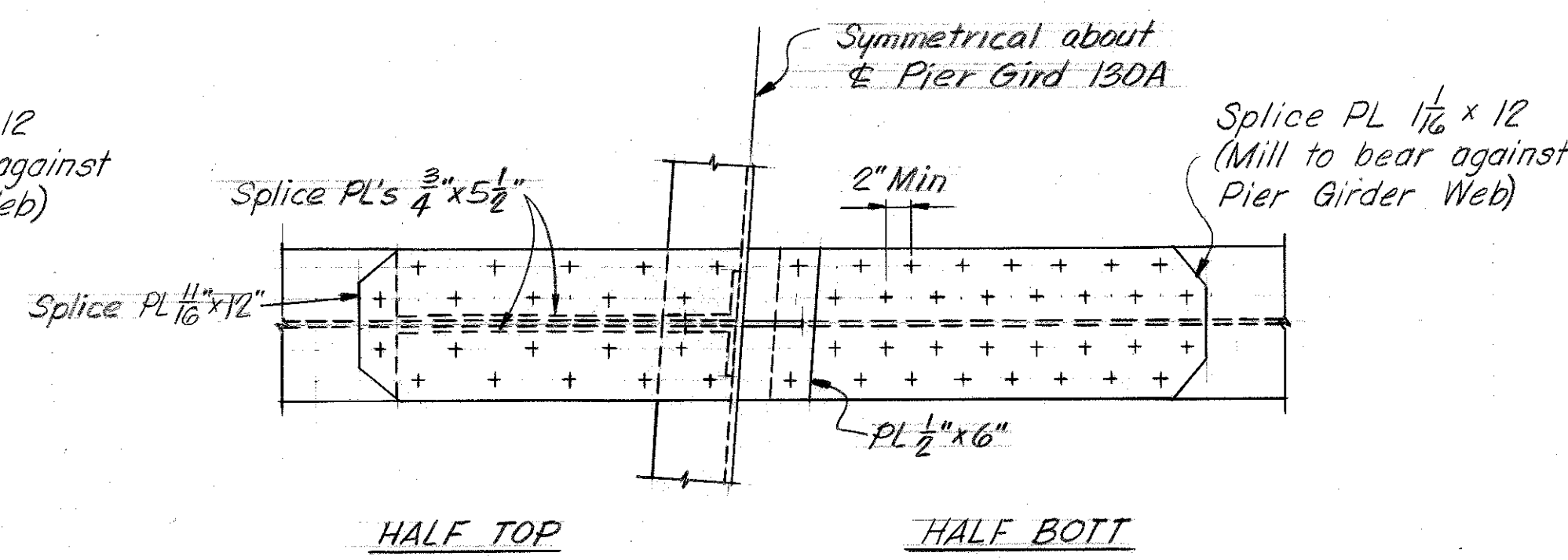
GIRD. W FLANGE SPLICE AT PIER GIRD. 127E



GIRD. R FLANGE SPLICE AT PIER GIRD. 129A

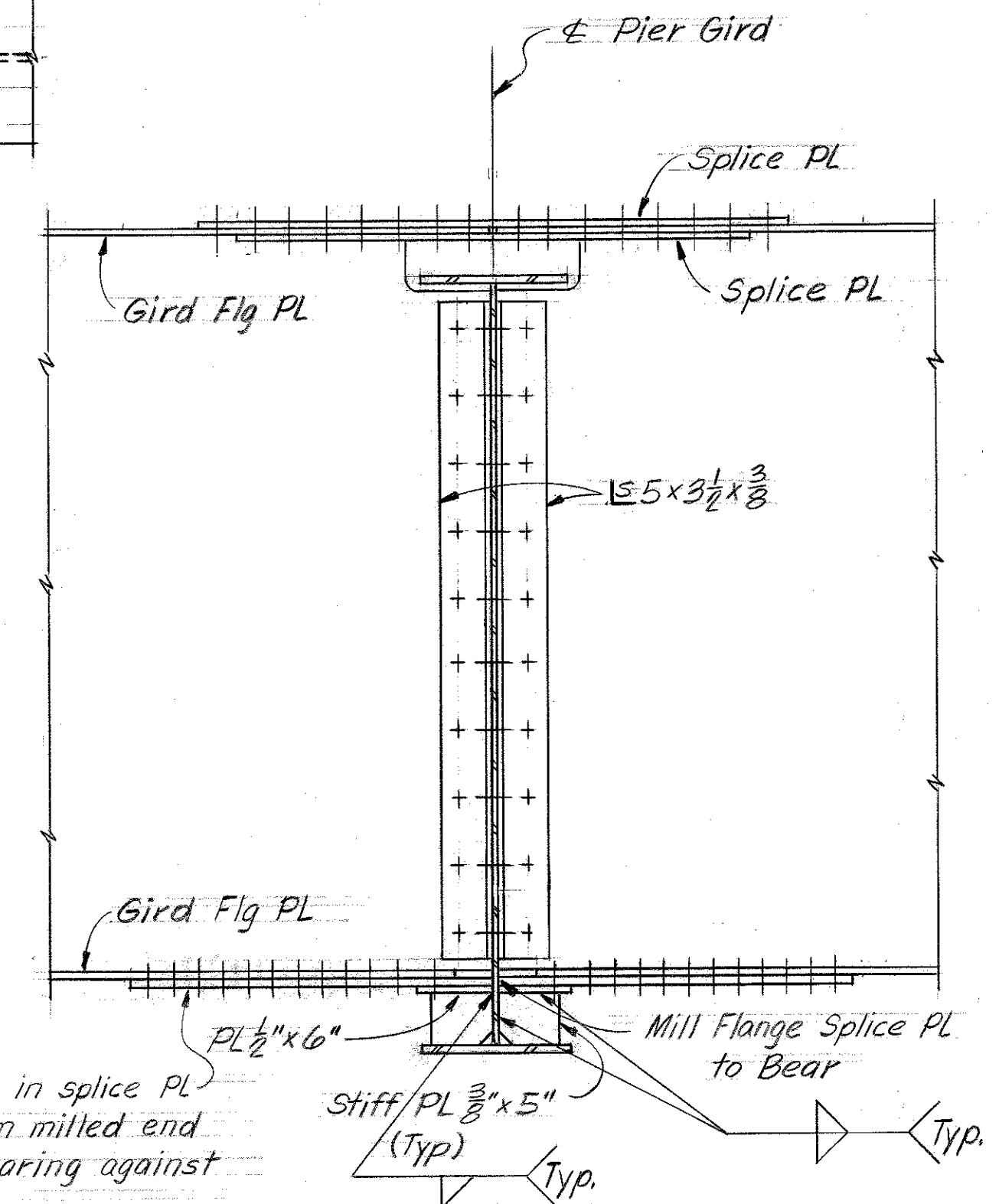


GIRD. T FLANGE SPLICE AT PIER GIRD. 129A & 129B



GIRDS. S, T, & U FLANGE SPLICE AT PIER GIRD. 130A

Offset holes in splice PL
1/16" away from milled end
to ensure bearing against
web PL.

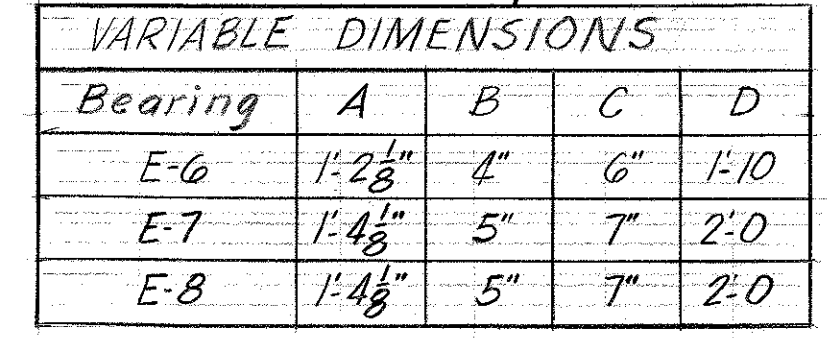
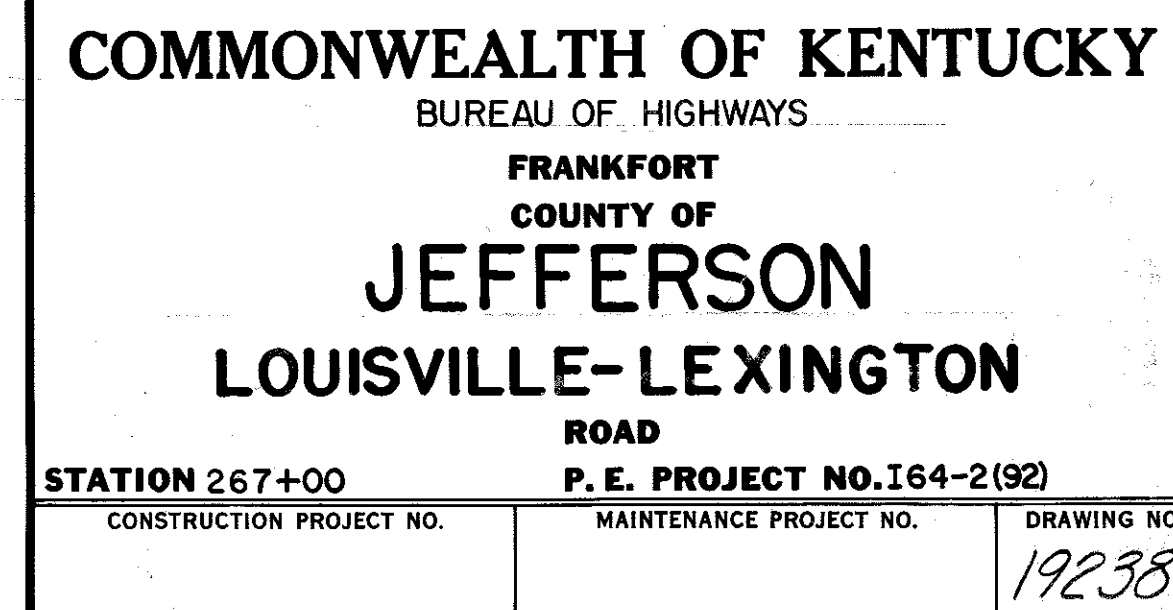


TYPICAL PIER GIRDER SECTION
AT GIRDER FLANGE SPLICE
(DRAWN FOR 129A
OTHERS SIMILAR)

For Notes see Sht 11.

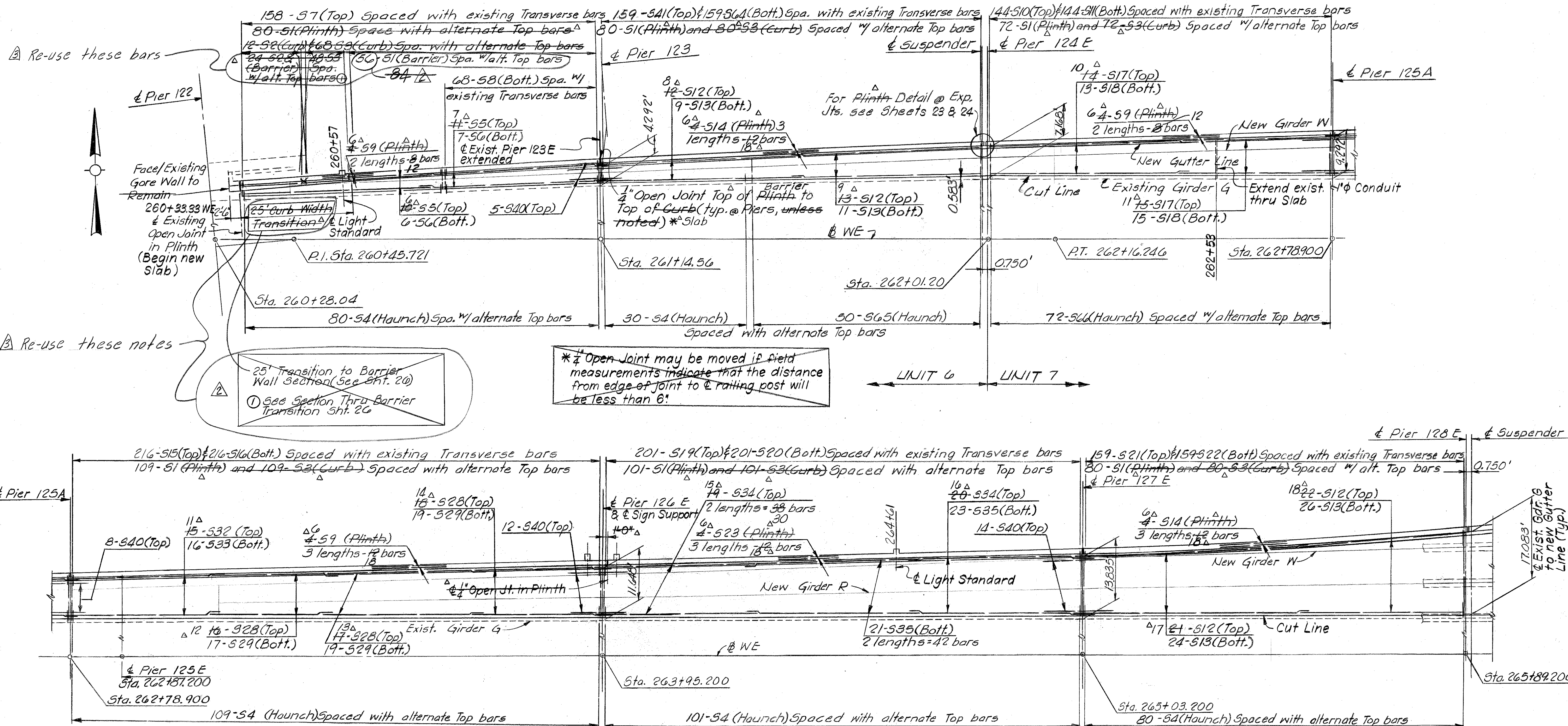
GIRDER SPLICES
STRUCTURAL STEEL

DESIGNED BY	<u>DMB</u>	CHECKED BY	<u>RBS</u>	DATE	<u>1-75</u>	REVISED	DATE
DETAILS BY	<u>LEW</u>	CHECKED BY	<u>ELW</u>	DATE	<u>4/75</u>	REVISED	DATE
TRACED BY		CHECKED BY		DATE		REVISED	DATE



UPDATE DATE
LETTING DATE

DESIGNED BY: DMB
CHECKED BY: RHM
DATE: 4-75
REVISIONS:
1. INDICATES REVISION FOR G.M. BARRIER
2. CHANGES BACK TO A
DATE: 7-75
DATE: 3-76
DATE: 5-76



SUPERSTRUCTURE QUANTITIES		
Concrete, Class 'AA'	(576)	cu. yds.
Steel Reinforcement	(163,120)	lbs.
Removal of Exist. Reinf. Conc.	1	lump sum *
Bridge Railing, New	25	lin. ft.
Bridge Railing, Relocated	890	lin. ft.
Deck Drainage	1	lump sum

* Includes approx. 890 lin. ft. of deck slab, curb & plinth

NOTES

- For reinforcement details see Sht. 30
- For concrete removal details see Sht. 27
- For sign support details see Shts. 28 & 37-40
- For deck drain & light standard details see Sht. 28
- For handrail post spacing see Sht. 29
- For deck cross sections see Sht. 27
- For details of expansion dams see Shts. 23 & 24, & 52
- For bridge lighting details see Shts. 42 & 43
- For construction elevations see Shts. 31-34

DECK PLAN

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON

STATION 267+00
CONSTRUCTION PROJECT NO.
P. E. PROJECT NO. 164-2(92)
MAINTENANCE PROJECT NO.
DRAWING NO.
19238

Sheet No. 25

25

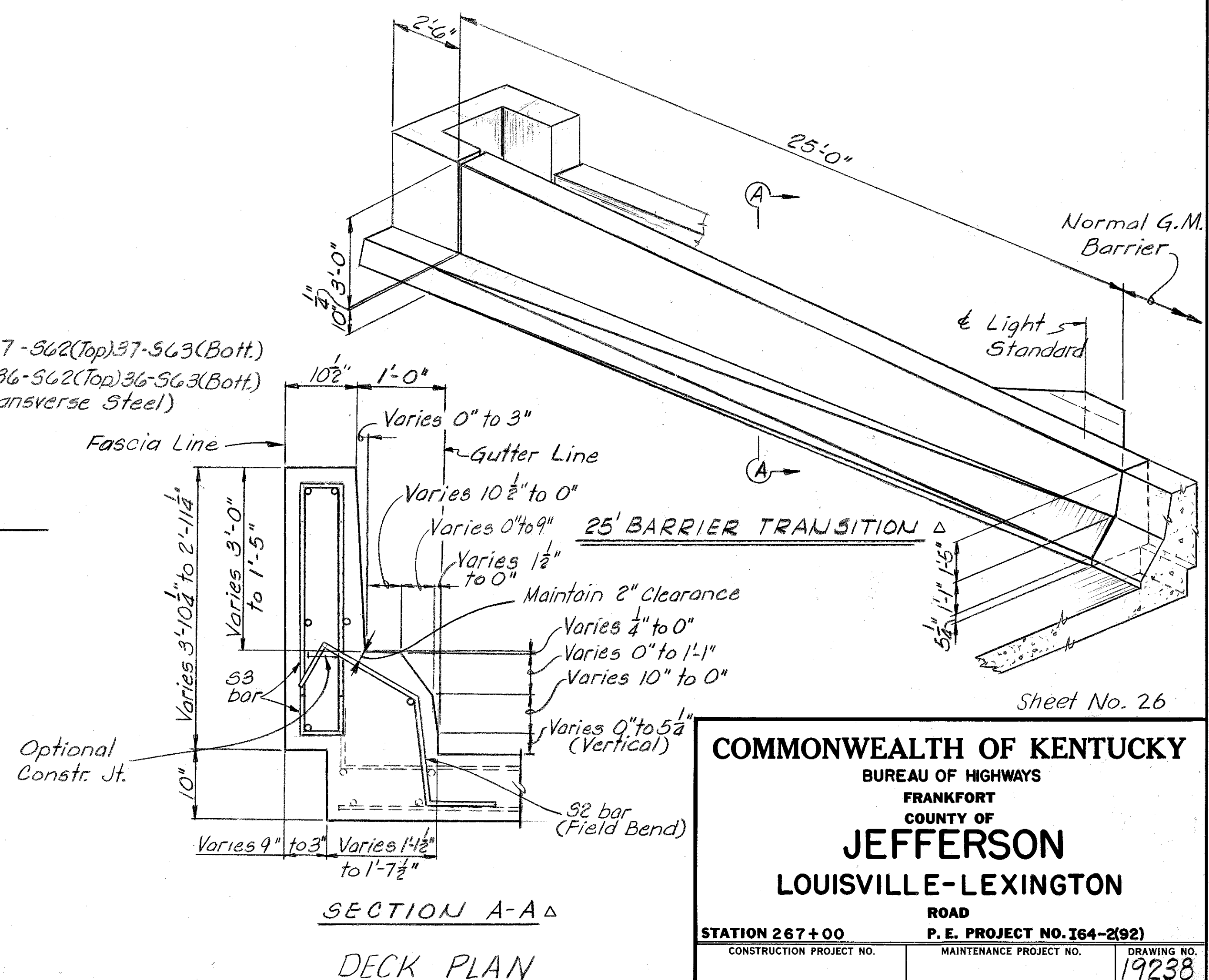
DESIGNED BY	DMB	CHECKED BY	RBS	DATE	4-75	Indicates Revision for G.M. Barrier	DATE	7-75
DRAWN BY	RHMC	CHECKED BY	PME	DATE			DATE	
DESIGNED BY		CHECKED BY		DATE			DATE	
DRAWN BY		CHECKED BY		DATE			DATE	



- ① 56-524(Top) 56-526(Bott.)
 - ② 55-525(Top) 55-527(Bott.)
 - ③ 43-542(Top) 43-543(Bott.)
 - ④ 43-544(Top) 43-545(Bott.)
 - ⑤ 43-546(Top) 43-547(Bott.)
 - ⑥ 44-548(Top) 44-549(Bott.)
 - ⑦ 43-550(Top) 43-551(Bott.)
 - ⑧ 43-552(Top) 43-553(Bott.)
 - ⑨ 37-554(Top) 37-555(Bott.)
 - ⑩ 37-556(Top) 37-557(Bott.)
 - ⑪ 37-558(Top) 37-559(Bott.) and 37-562(Top) 37-563(Bott.)
 - ⑫ 36-560(Top) 36-561(Bott.) and 36-562(Top) 36-563(Bott.)
- (Space above bars with existing Transverse Steel)

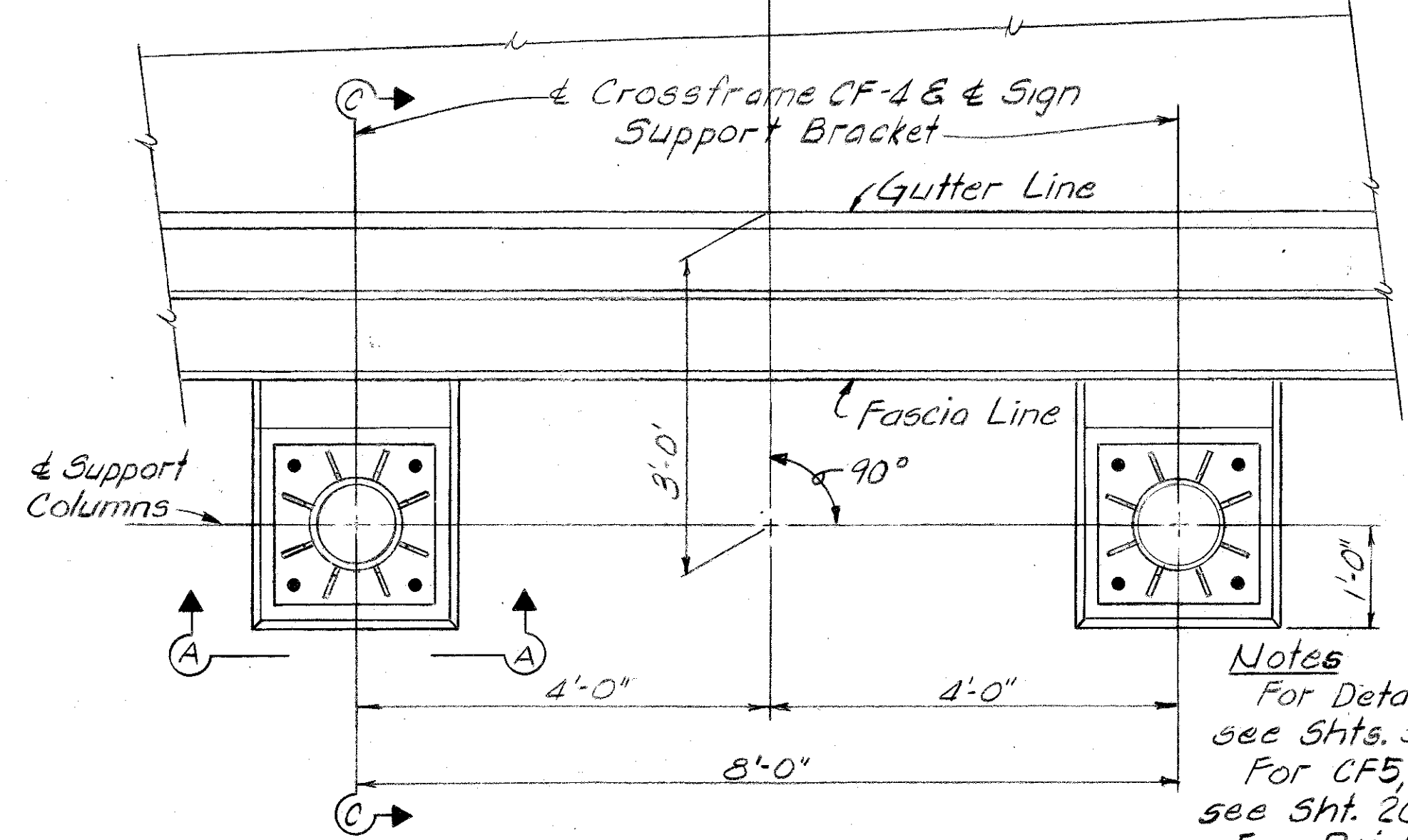
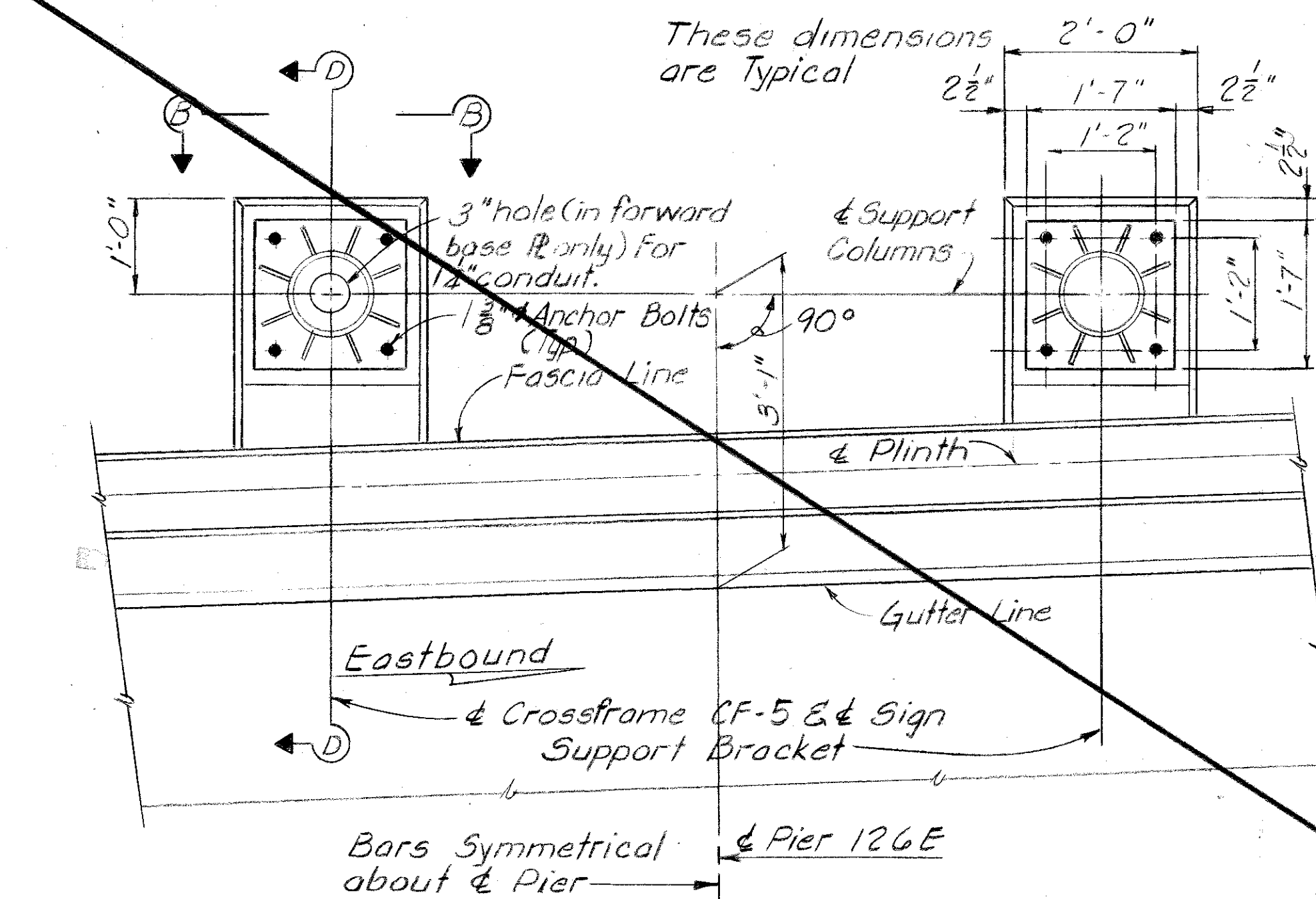
1'-8"	#4 Bars
2'-1"	#5 Bars
2'-6"	#6 Bars

MINIMUM BAR LAPS

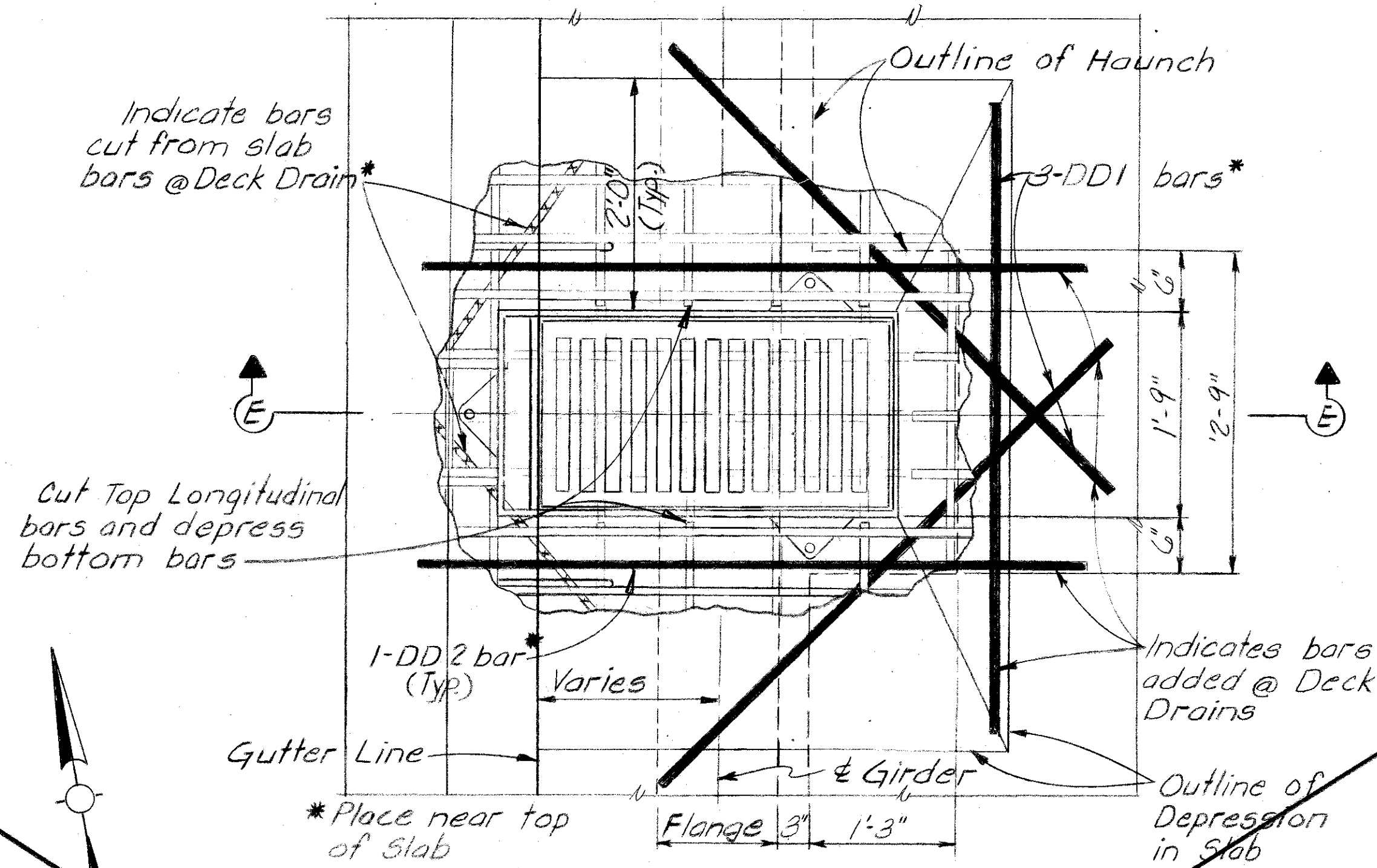
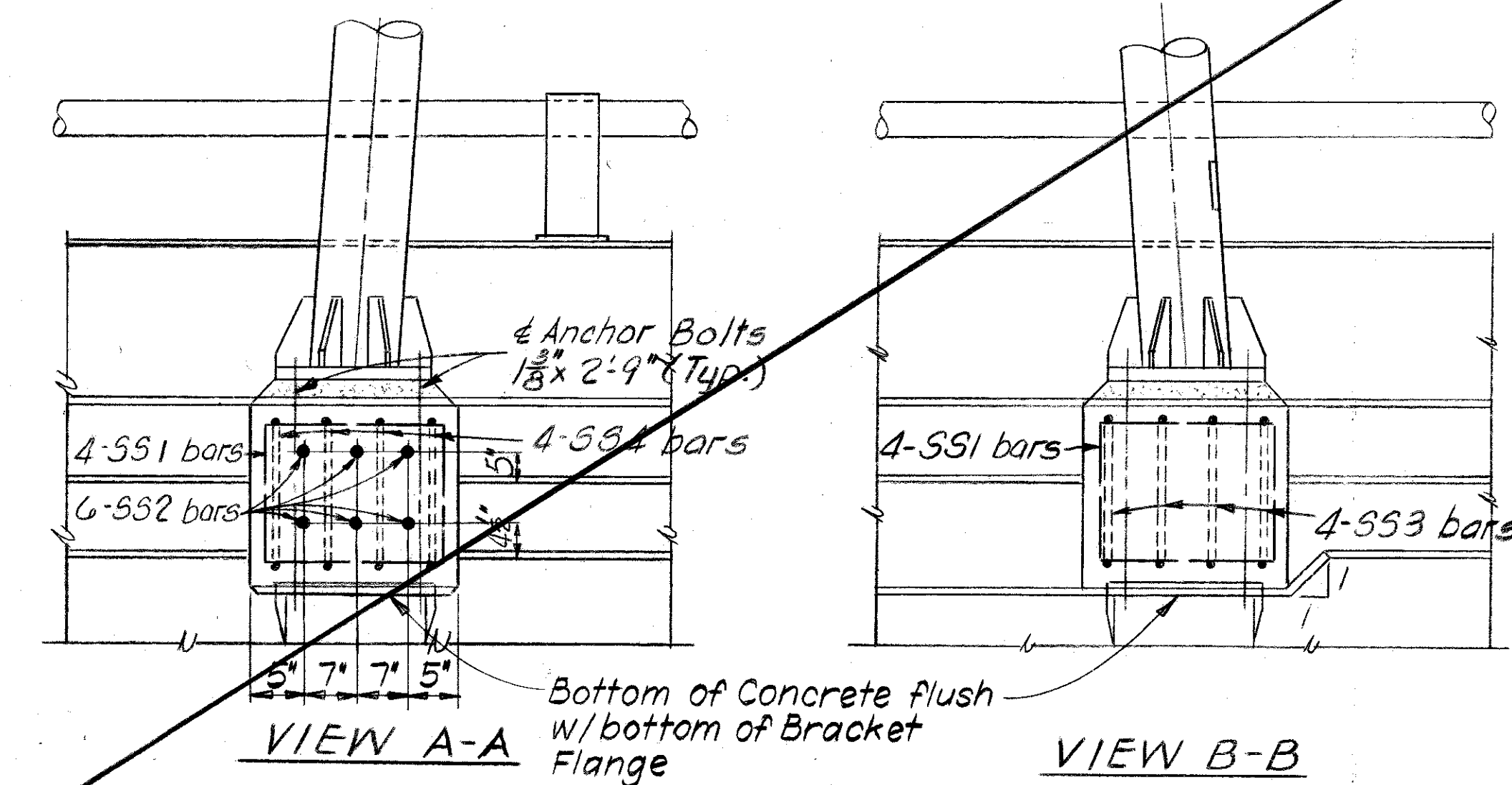


LETTING DATE

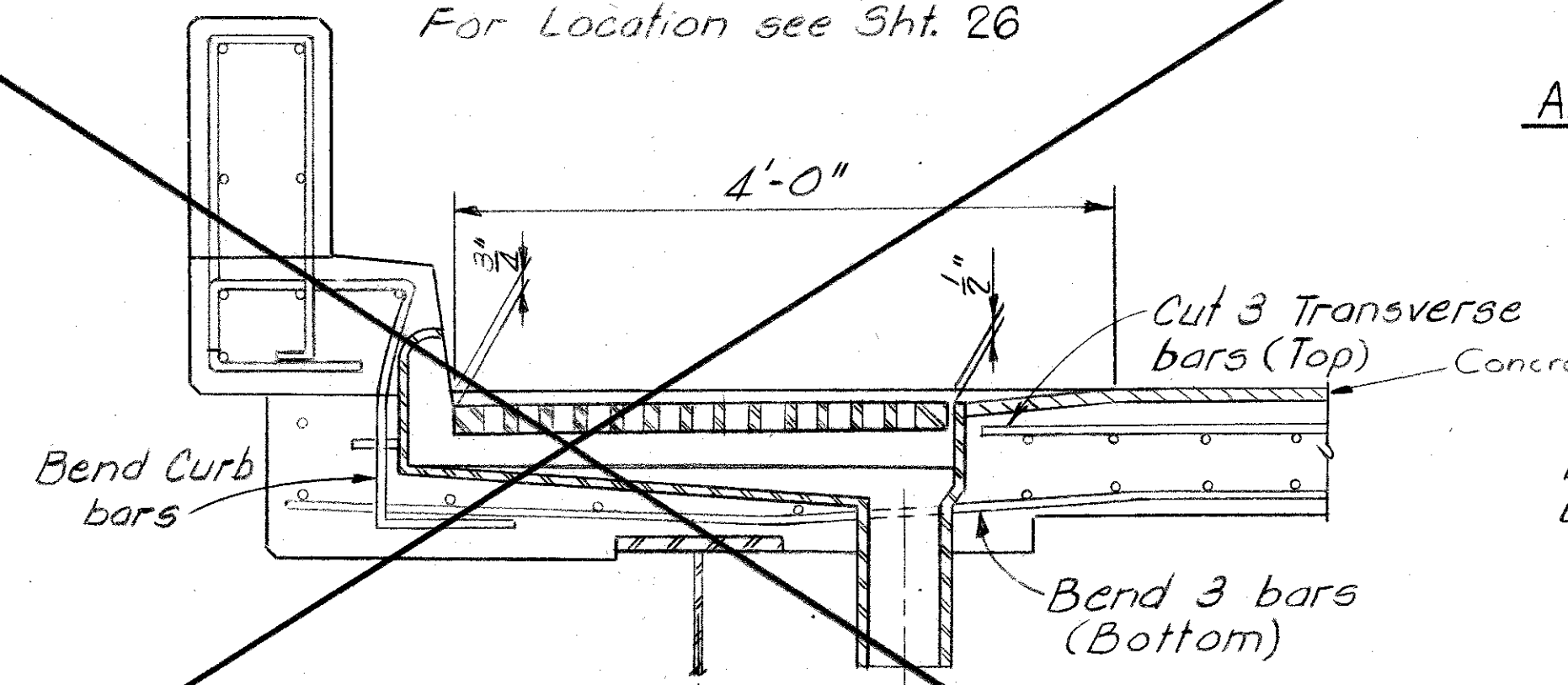
DESIGNED BY: **RHM** CHECKED BY: **PME** DATE: **4-1-75**
 REVISION: **7/75** DATE: **7/75**
 REVISION: **7/75** DATE: **7/75**
 REVISION: **7/75** DATE: **7/75**
 REVISION: **7/75** DATE: **7/75**



PLAN OF SIGN SUPPORT BASE

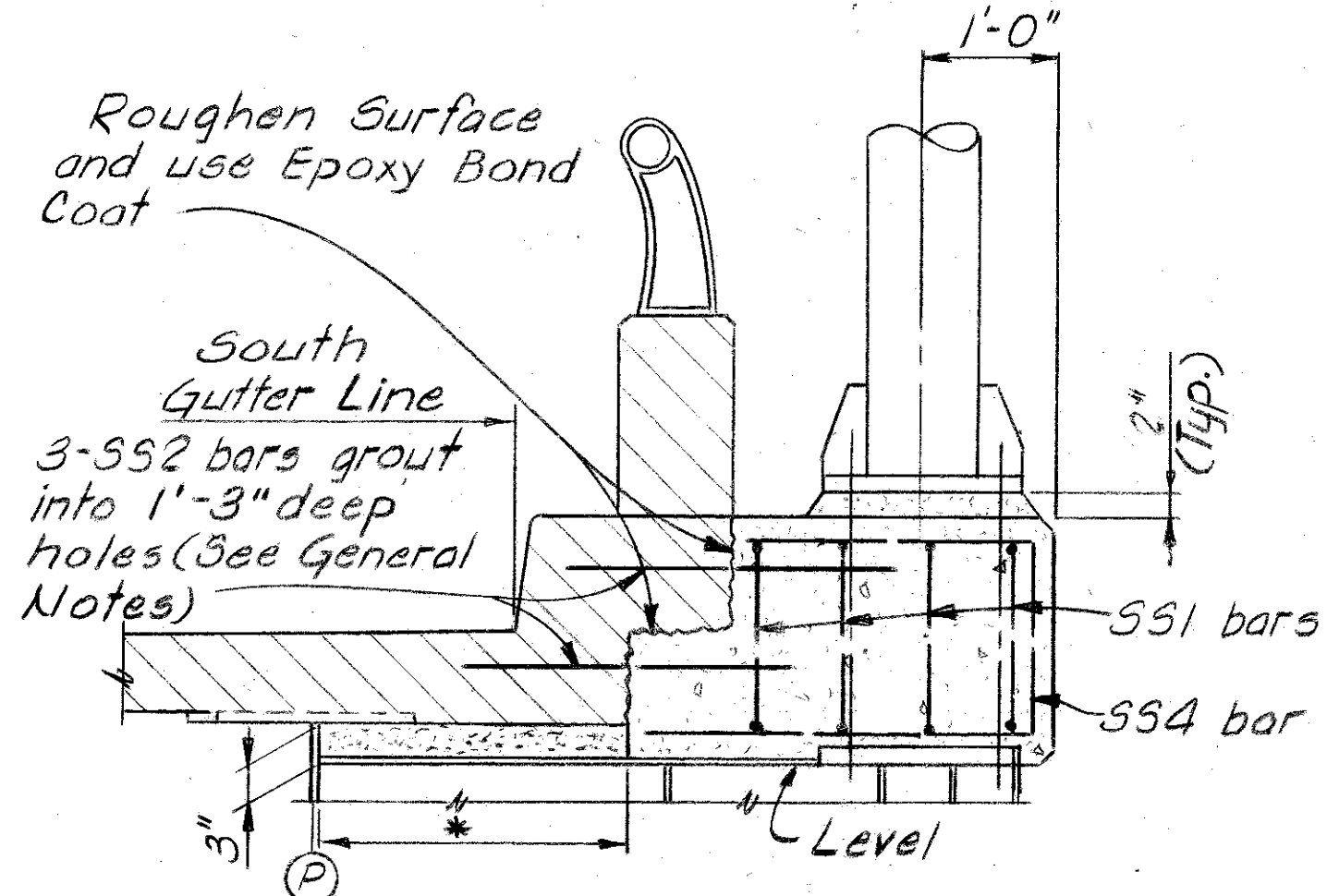


PLAN OF DECK DRAIN
For Location see Sht. 26



SECTION E-E

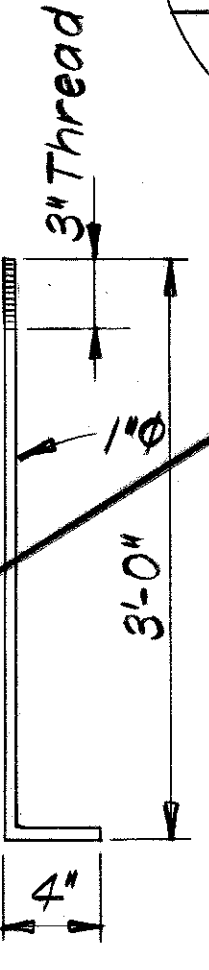
VOID
 SEE SHEET 53
 FOR REVISION



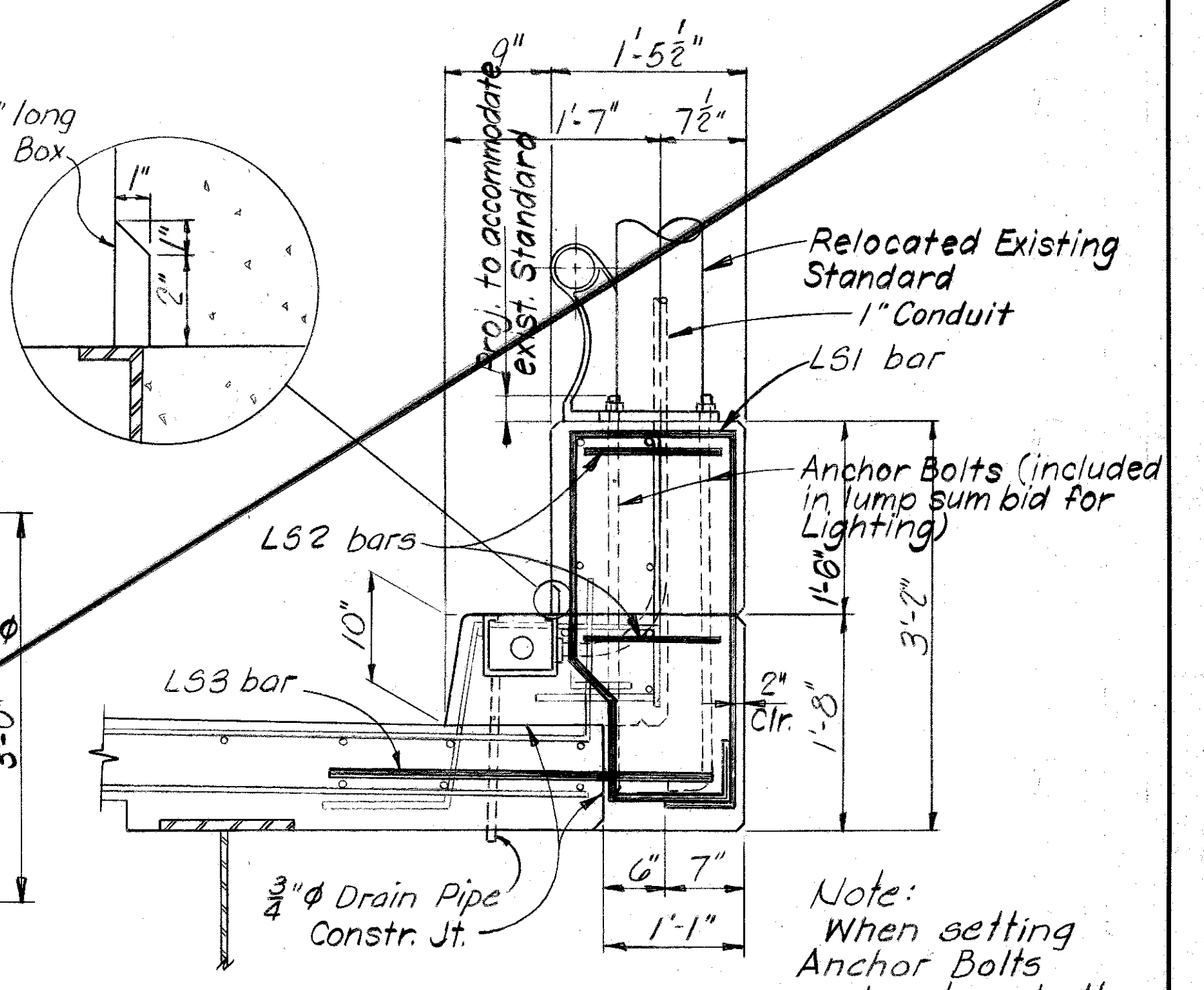
SECTION C-C

* Fill area between top flange of bracket and bottom of slab w/ expansive grout.

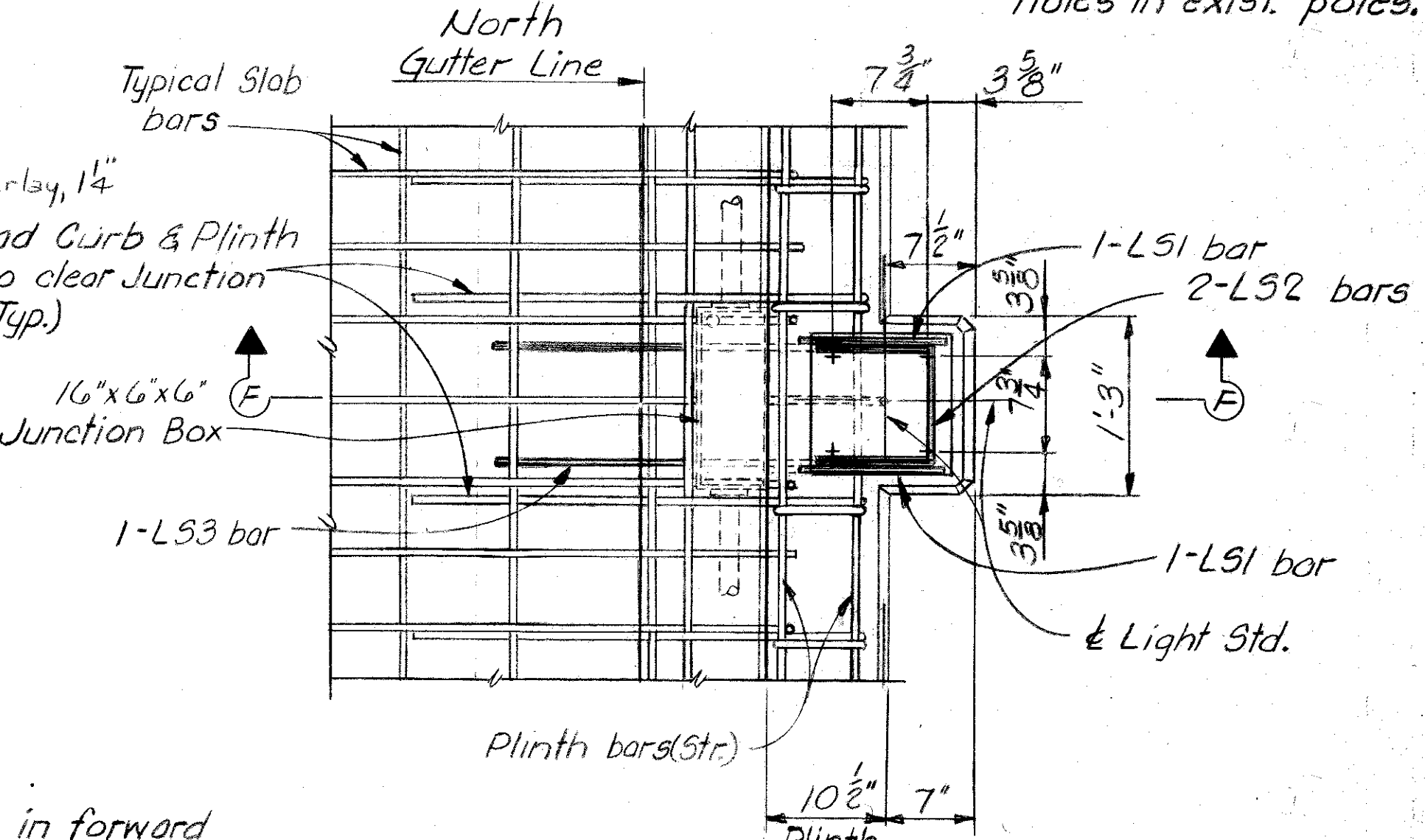
Inset 1'-8" long @ Junction Box



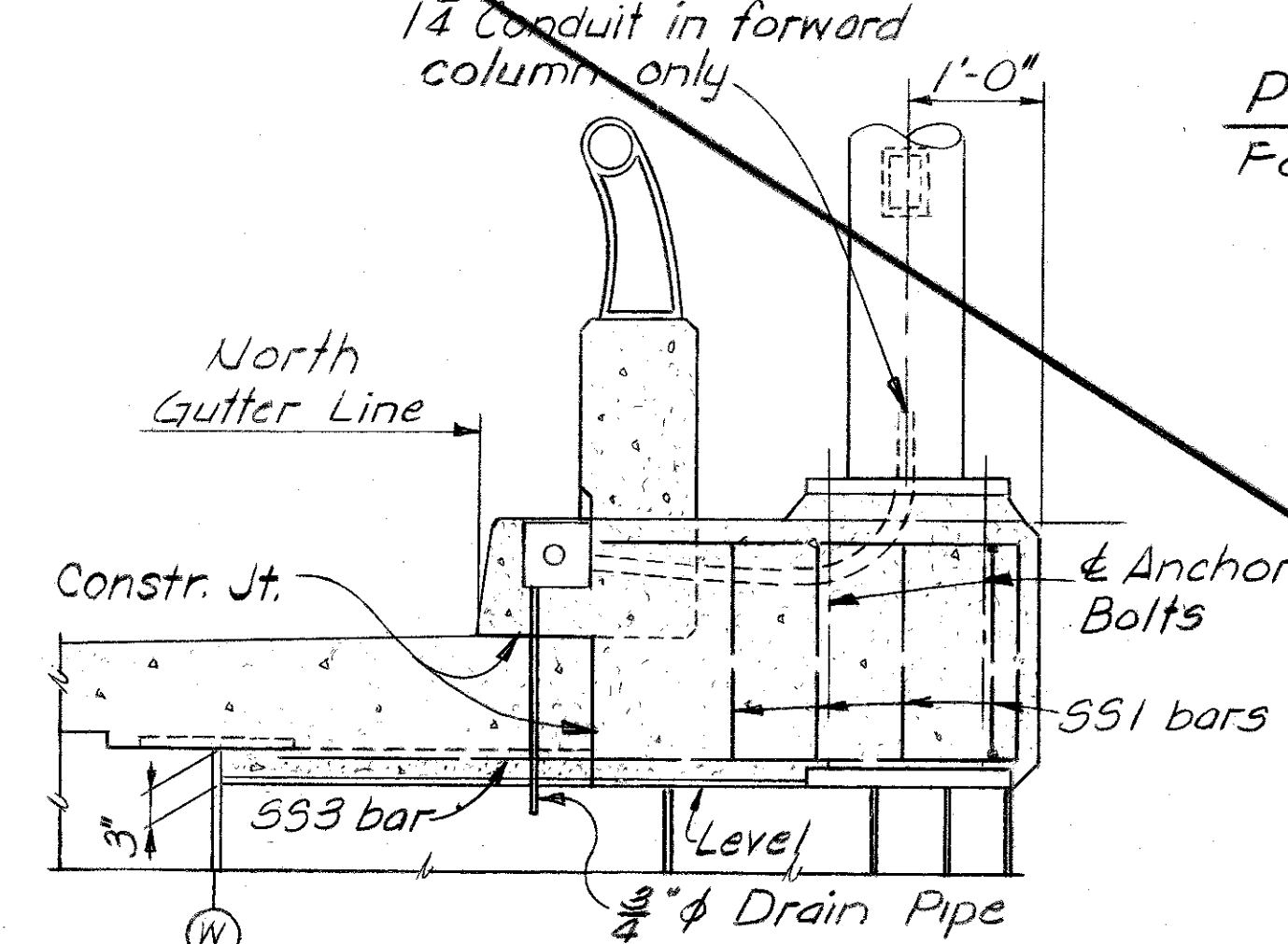
ANCHOR BOLT DETAIL



SECTION F-F



PLAN OF LIGHT STANDARD BASE
For Reinforcing Bar Details see Sht. 30
For Location see Shts. 25 & 26.



SECTION D-D

DECK-MISC. DETAILS

SHEET 28

COMMONWEALTH OF KENTUCKY
 BUREAU OF HIGHWAYS
 FRANKFORT
 COUNTY OF
JEFFERSON
 LOUISVILLE-LEXINGTON
 ROAD
 STATION 267+00
 CONSTRUCTION PROJECT NO. P. E. PROJECT NO. 164-2(92)
 MAINTENANCE PROJECT NO. DRAWING NO. 19238



UPDATE DATE
LETTING DATE

DESIGNED BY: **DMB** CHECKED BY: **RBS** DATE: **7-75**
TRACED BY: **BME** CHECKED BY: **RMC** DATE: **Apr 1, 75**
Δ Indicates revision for G.M. Barrier

SLAB																			
MARK	TYPE	SIZE	NO. OF BARS IN SPAN EAST OF PIER											LENGTH		a		LOCATION	
			122	123	124	125A	126	127	128	129A	129B	130A	131	ft.	in.	ft.	in.		
S1	②	4	80	80	72	109	101	80	56	65	65	74	47	6	0	-	-	Plinth Tie	
S2	⑤	4	12	-	-	-	-	-	-	-	-	-	-	4	1	1	4 1/2	Curb Tie	
S3	⑤	4	68	80	72	109	101	80	56	65	65	74	47	4	0	1	3	"	"
S4	⑥	6	80	30	-	109	101	80	56	65	65	74	47	4	8	1	7	Haunch	
S5	Str.	4	21	-	-	-	-	-	-	-	-	99	-	42	8	-	-	T/SL	
S6	"	5	13	-	-	-	-	-	-	-	-	127	-	43	1	-	-	B/SL	
S7	⑦	6	158	-	-	-	-	-	-	-	-	-	-	6	0	4	9	T/ST	
S8	Str.	6	68	-	-	-	-	-	-	-	-	-	-	4	9	-	-	B/ST	
S9	"	4	8	-	8	12	-	-	-	-	-	-	-	41	0	-	-	LP	
S10	⑦	6	-	-	144	-	-	-	-	-	-	-	-	10	11	9	8	T/ST	
S11	Str.	6	-	-	144	-	-	-	-	-	-	-	-	9	8	-	-	B/ST	
S12	"	4	-	25	-	-	-	43	-	-	-	-	-	45	0	-	-	T/SL	
S13	"	5	-	20	-	-	-	50	-	-	-	-	-	45	5	-	-	B/SL	
S14	"	4	-	12	-	-	-	12	8	-	-	-	-	31	0	-	-	LP	
S15	⑦	6	-	-	-	216	-	-	-	-	-	-	-	13	4	12	1	T/ST	
S16	Str.	6	-	-	-	216	-	-	-	-	-	-	-	12	1	-	-	B/ST	
S17	"	4	-	-	29	-	-	-	-	-	-	-	-	40	7	-	-	T/SL	
S18	"	5	-	-	28	-	-	-	-	-	-	-	-	41	0	-	-	B/SL	
S19	⑦	6	-	-	-	-	201	-	-	-	-	-	-	15	6	14	3	T/ST	
S20	Str.	6	-	-	-	-	201	-	-	-	-	-	-	14	3	-	-	B/ST	
S21	⑦	6	-	-	-	-	-	159	-	-	-	-	-	18	9	17	6	T/ST	
S22	Str.	6	-	-	-	-	-	159	-	-	-	-	-	17	6	-	-	B/ST	
S23	"	4	-	-	-	-	12	-	-	8	8	-	-	37	0	-	-	LP	
S24	⑦	6	-	-	-	-	-	56	-	-	-	-	-	20	10	19	7	T/ST	
S25	⑦	6	-	-	-	-	-	55	-	-	-	-	-	22	11	21	8	"	
S26	Str.	6	-	-	-	-	-	56	-	-	-	-	-	19	7	-	-	B/ST	
S27	"	6	-	-	-	-	-	55	-	-	-	-	-	21	8	-	-	"	
S28	"	4	-	-	-	51	-	50	-	-	-	-	-	31	8	-	-	T/SL	
S29	"	5	-	-	-	55	-	64	-	-	-	-	-	32	1	-	-	B/SL	
S30	"	4	-	-	-	-	-	-	-	-	-	8	-	42	0	-	-	LP	
S31	"	4	-	-	-	-	-	-	-	-	-	-	8	26	0	-	-	LP	
S32	"	4	-	-	-	15	-	-	59	28	-	-	-	28	0	-	-	T/SL	
S33	"	5	-	-	-	16	-	-	73	24	-	-	-	28	5	-	-	B/SL	
S34	"	4	-	-	-	58	-	-	50	-	-	-	-	37	8	-	-	T/SL	
S35	"	5	-	-	-	65	-	-	80	-	-	-	-	38	1	-	-	B/SL	
S36	"	4	-	-	-	-	-	-	34	16	-	-	-	19	2	-	-	T/SL	
S37	"	5	-	-	-	-	-	-	47	17	-	-	-	19	7	-	-	B/SL	
S38	"	4	-	-	-	-	-	-	-	-	-	104	26	8	-	-	-	T/SL	
S39	"	5	-	-	-	-	-	-	-	-	-	130	27	1	-	-	-	B/SL	
S40	"	5	-	5	-	8	12	14	-	21	31	40	-	15	0	-	-	T/S@Piers	
S41	⑦	6	-	159	-	-	-	-	-	-	-	-	-	8	9	7	6	T/ST	
S42	⑦	6	-	-	-	-	-	-	-	43	-	-	-	25	2	23	11	"	
S43	Str.	6	-	-	-	-	-	-	-	43	-	-	-	23	11	-	-	B/ST	
S44	⑦	6	-	-	-	-	-	-	-	43	-	-	-	27	6	26	3	T/ST	
S45	Str.	6	-	-	-	-	-	-	-	43	-	-	-	26	3	-	-	B/ST	
S46	⑦	6	-	-	-	-	-	-	-	43	-	-	-	29	10	28	7	T/ST	
S47	Str.	6	-	-	-	-	-	-	-	43	-	-	-	28	7	-	-	B/ST	
S48	⑦	6	-	-	-	-	-	-	-	44	-	-	-	32	8	31	5	T/ST	
S49	Str.	8	-	-	-	-	-	-	-	44	-	-	-	31	5	-	-	B/ST	
S50	⑦	6	-	-	-	-	-	-	-	43	-	-	-	35	6	34	3	T/ST	
S51	Str.	6	-	-	-	-	-	-	-	43	-	-	-	34	3	-	-	B/ST	

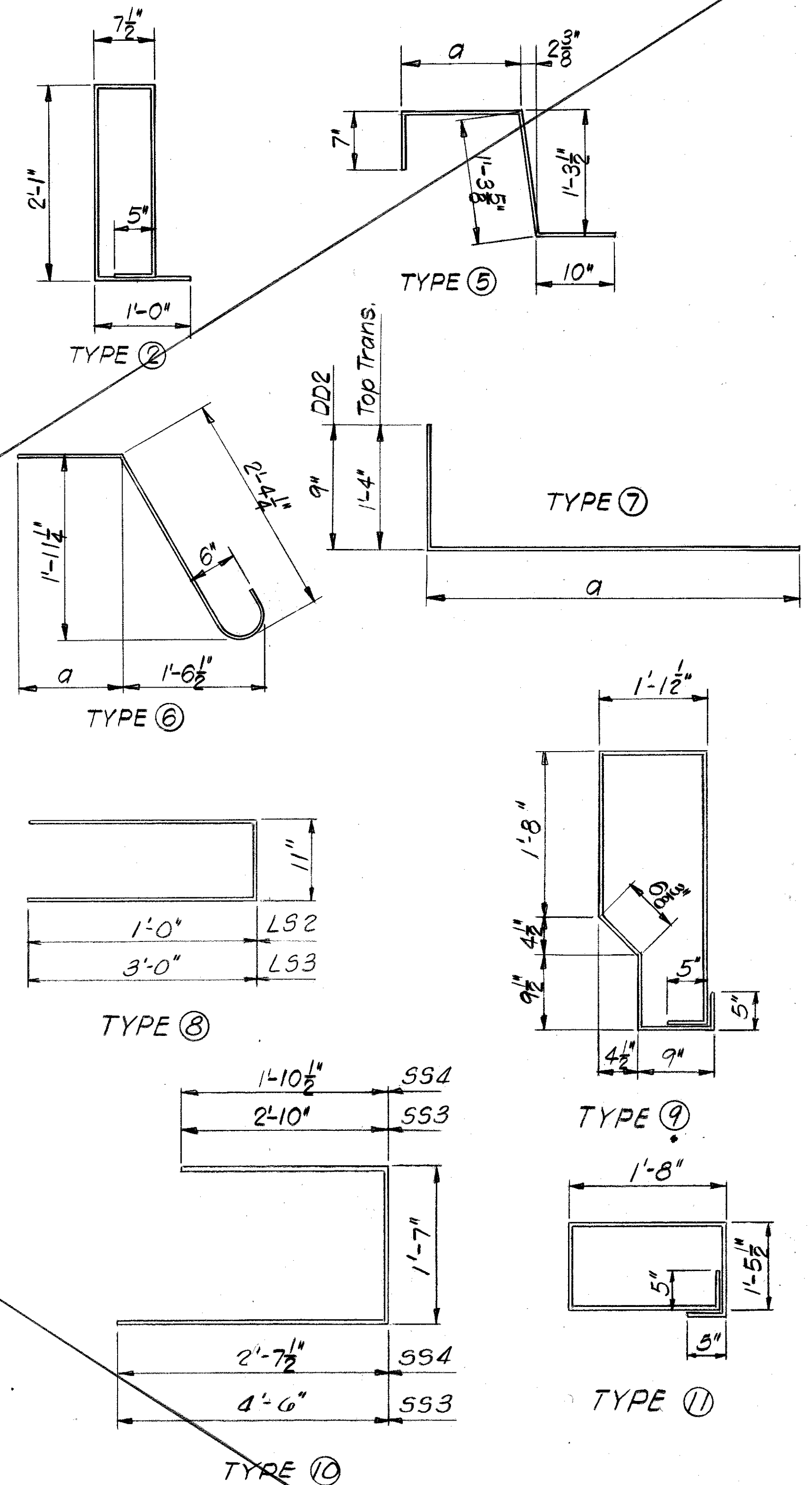
SLAB																			
MARK	TYPE	SIZE	NO. OF BARS IN SPAN EAST OF PIER											LENGTH		a		LOCATION	
			122	123	124	125A	126	127	128	129A	129B	130A	131	ft.	in.	ft.	in.		
S52	⑦	6	-	-	-	-	-	-	-	-	-	43	-	38	4	37	1	T/ST	
S53	Str.	6	-	-	-	-	-	-	-	-	-	43	-	37	1	-	-	B/ST	
S54	⑦	6	-	-	-	-	-	-	-	-	-	37	-	40	11	39	8	T/ST	
S55	Str.	6	-	-	-	-	-	-	-	-	-	37	-	39	8	-	-	B/ST	
S56	⑦	6	-	-	-	-	-	-	-	-	-	37	-	43	6	42	3	T/ST	
S57	Str.	6	-	-	-	-	-	-	-	-	-	37	-	42	3	-	-	B/ST	
S58	⑦	6	-	-	-	-	-	-	-	-	-	37	-	23	1	21	9	T/ST	
S59	Str.	6	-	-	-	-	-	-	-	-	-	37	-	33	6	-	-	B/ST	
S60	⑦	6	-	-	-	-	-	-	-	-	-	36	93	25	0	23	9	T/ST	
S61	Str.	6	-	-	-	-	-	-	-	-	-	36	93	36	0	-	-	B/ST	
S62	Str.	6	-	-	-	-	-	-	-	-	-	73	93	26	2	-	-	T/ST	
S63	Str.	6	-	-	-	-	-	-	-	-	-	73	93	13	10	-	-	B/ST	
S64	Str.	6	-	159	-	-	-	-	-	-	-	-	-	7	6	-	-	"	
S65	⑥	6	-	50	-	-	-	-	-	-	-	-	-	9	7	6	6	Haunch	
S66	⑥	6	-	-	72	-	-	-	-	-	-	-	-	10	5	7	4	"	
LS1	⑨	4	2	-	-	-	2	-	-	-	-	2	-	8	4	-	-	Light Std.	
LS2	⑧	4	2	-	-	-	2	-	-	-	-	2	-	2	10	-	-	"	
LS3	⑧	4	1	-	-	-	1	-	-	-	-	1	-	6	10	-	-	"	
SS1	⑪	4	-	-	-	4	4	-	-	-	-	-	-	6	10	-	-	SS/NF	
SS1	⑪	4	-	-	-	4	4	-	-	-	-	-	-	6	10	-	-	SS/SF	
SS2	Str.	6	-	-	-	6	6	-	-	-	-	-	-	2	6	-	-	"	
SS3	⑩	6	-	-	-	4	4	-	-	-	-	-	-	8	10	-	-	SS/NF	
SS4	⑩	6	-	-	-	4	4	-	-	-	-	-	-	6	4	-	-	SS/SF	
DD1	Str.	6	-	-	-	-	-	-	-	3	3	-	-	5	6	-	-	Slab@Drain	
DD2	⑦	6	-	-	-	-	-	-	-	2	2	-	-	6	3	5	6	"	

T/ST = Top of Slab Transverse
B/ST = Bott. " " "
T/SL = Top " " Longitudinal
B/SL = Bott. " " "
LP = Longitudinal Plinth
SS/NF = Sign Support, North Fascia
SS/SF = " " " South "

Long. Curb Bars are included with T/SL Bars.
" Haunch " " " " B/SL " "

VOID
See Sheet 54 for
Revision

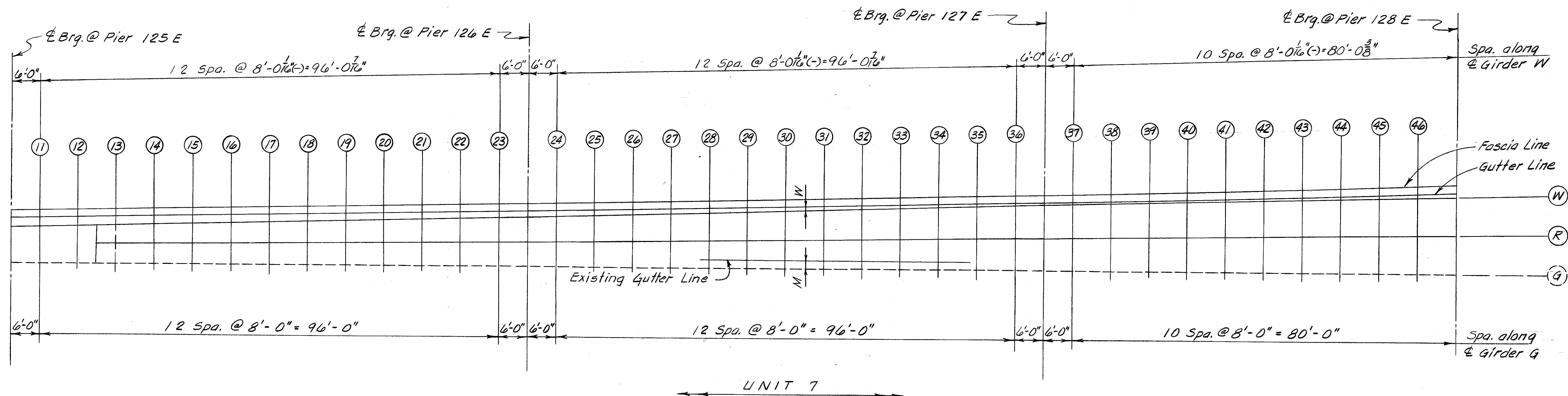
DECK BAR LIST



Sheet No. 30

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
ROAD
LOUISVILLE-LEXINGTON
STATION 267+00
CONSTRUCTION PROJECT NO. P. E. PROJECT NO. 164-2(92)
MAINTENANCE PROJECT NO. DRAWING NO. 19238

DESIGNED BY <u>RBS</u>	CHECKED BY <u>Black</u>	DATE <u>3-75</u>	REVISED _____	DATE _____
DETAILED BY <u>E.L.U./RHM</u>	CHECKED BY <u>Black</u>	DATE <u>3-75</u>	REVISED _____	DATE _____
TRACED BY _____	CHECKED BY _____	DATE _____	REVISED _____	DATE _____



COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON
ROAD
STATION 267+00 P. E. PROJECT NO. 164-2(92)

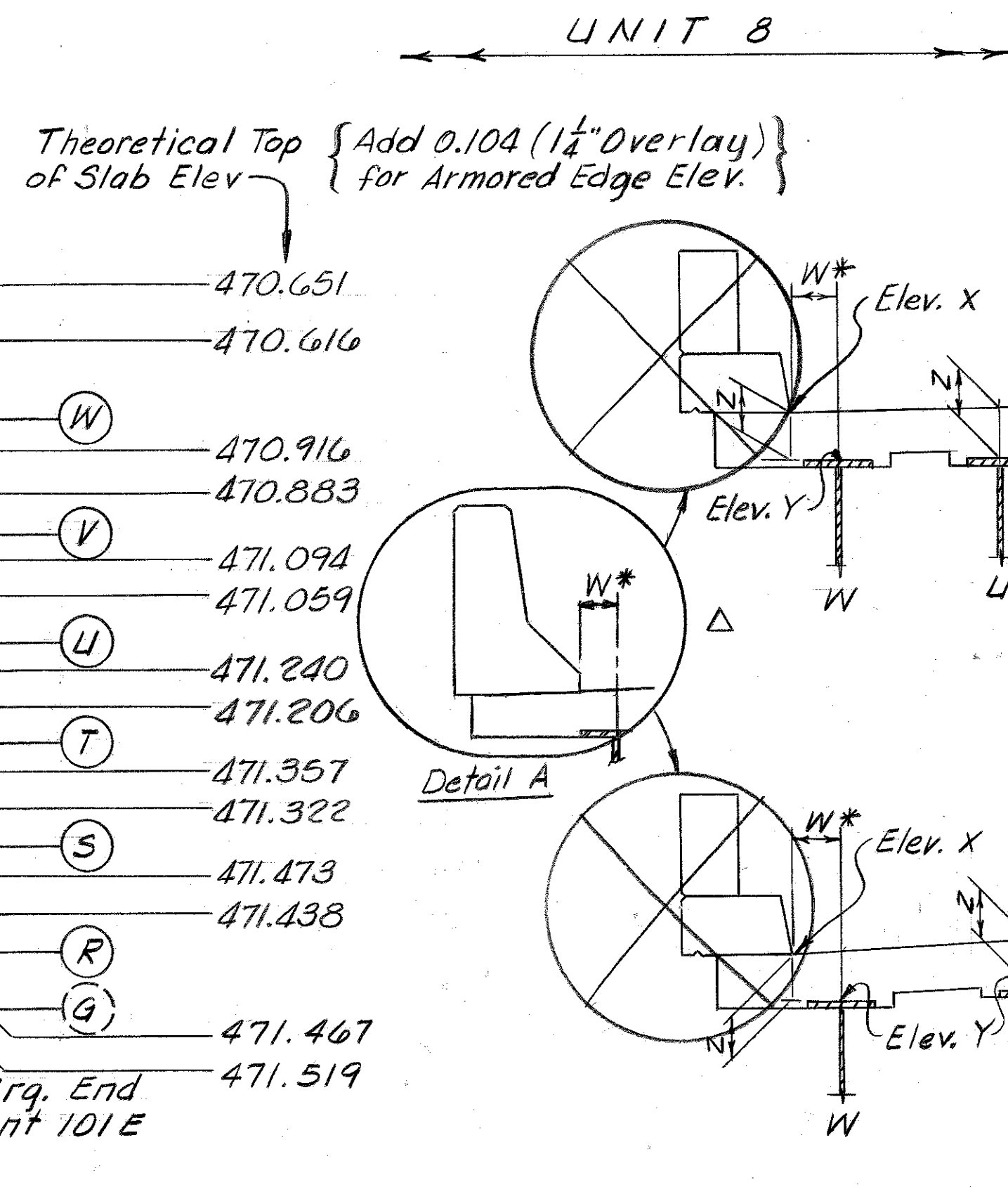
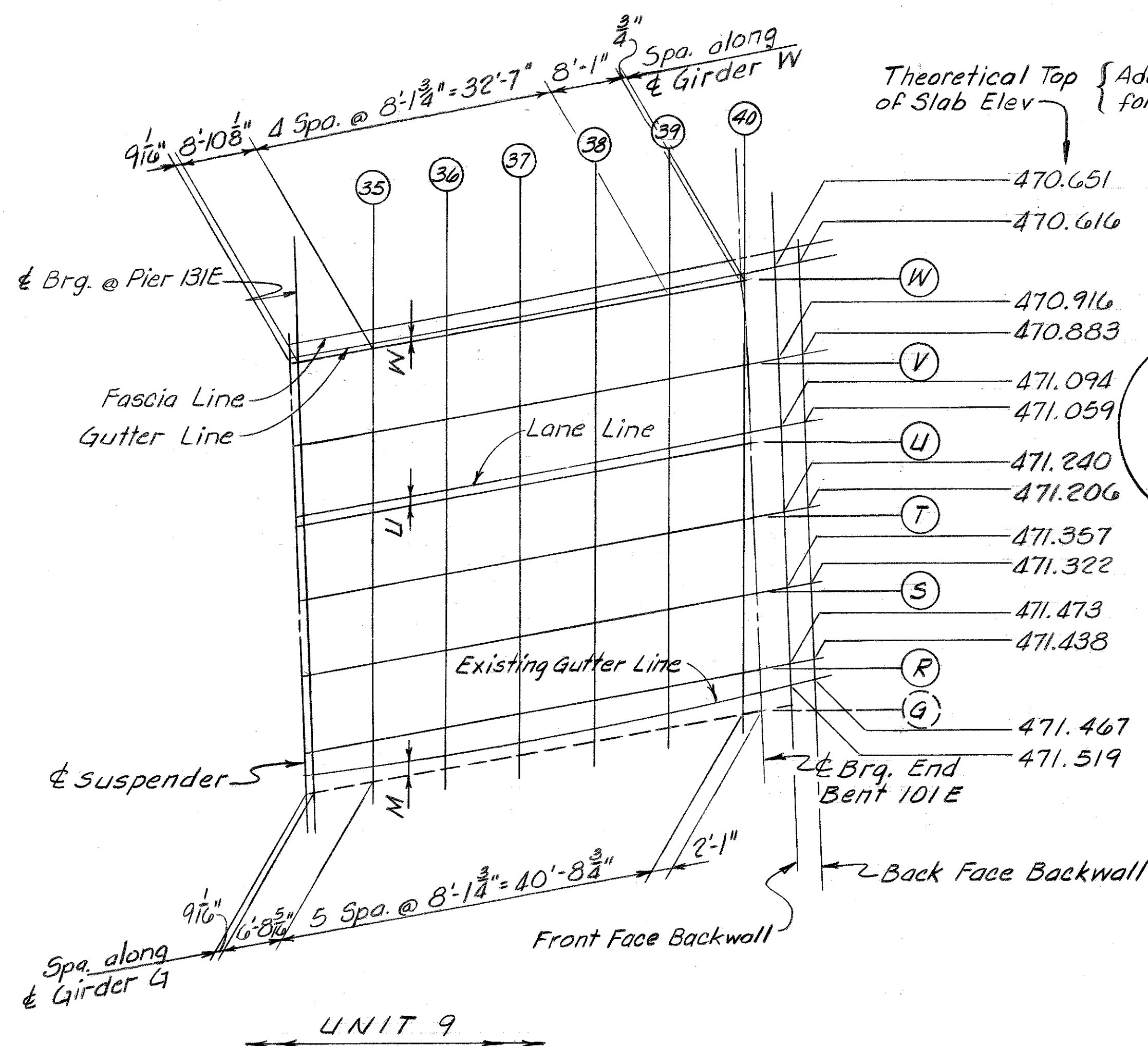
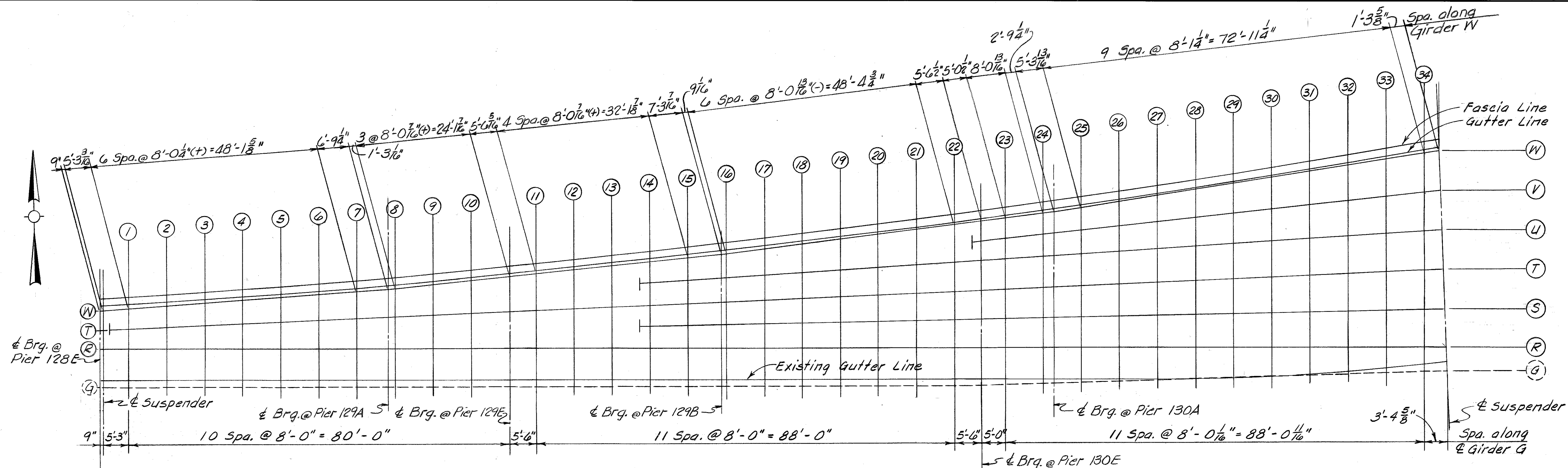
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.	DRAWING NO.

CONSTRUCTION ELEVATIONS

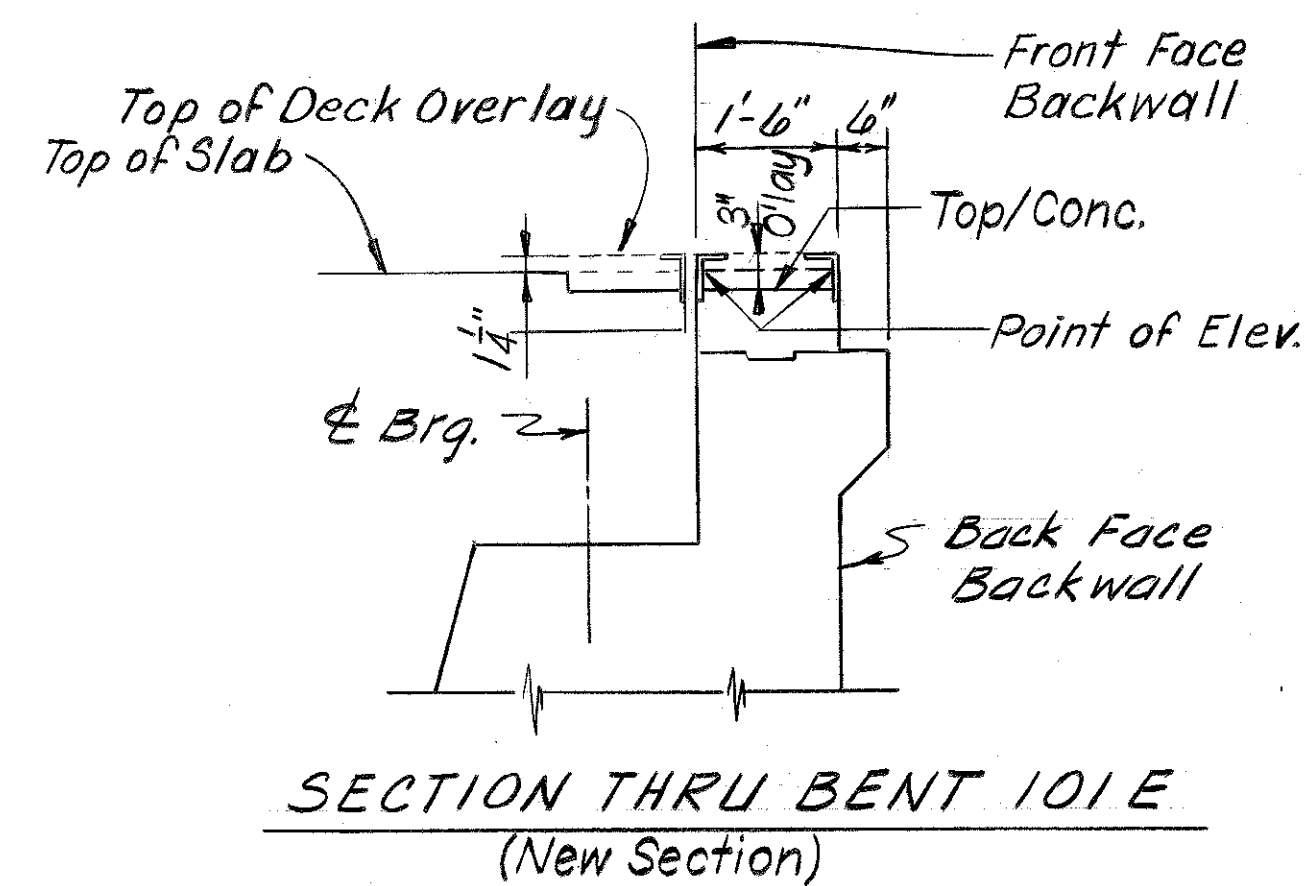
DRAWING NO.	19238
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UPDATE DATE
LETTING DATE

DESIGNED BY: *ELU/PRM* CHECKED BY: *RB* DATE: 3.75
 REVISION: *1* DATE: 3.15
 TRACED BY: *ELU/PRM* CHECKED BY: *RB* DATE: 3.15



TYPICAL SECTIONS
 * - Negative sign indicates girder on opposite side of reference line than shown.



SHEET 32

COMMONWEALTH OF KENTUCKY
 BUREAU OF HIGHWAYS
 FRANKFORT
 COUNTY OF
JEFFERSON
 LOUISVILLE-LEXINGTON
 ROAD
 STATION 267+00
 P. E. PROJECT NO. 164-2(92)
 CONSTRUCTION PROJECT NO. MAINTENANCE PROJECT NO. DRAWING NO. 19238

CONSTRUCTION ELEVATIONS

UPDATE DATE
LETTING DATE

DESIGNED BY: R. B. B. CHECKED BY: R. B. B. DATE: 3-15
DATE: 3-15
REVISIONS: 1 BY: E. L. U. / R. H. M. CHECKED BY: R. B. B. DATE: 3-15
DATE: 3-15
TRACED BY: R. B. B. CHECKED BY: R. B. B. DATE: 3-15
DATE: 3-15

Section	TABLE OF ELEVATIONS FOR UNIT 6			
	GIRDER W			
	W	X@Gutter	Y	Z
Brq. 122				
34				
35				
36	0'-7 1/2"	470.256		
37	0'-3 1/2"	470.328		
38	0'-0 3/4"	470.396		
39	10'-2 1/2"	470.465		
40	0'-5 1/2"	470.531		
41	0'-8 1/2"	470.590		
42	0'-11 1/2"	470.659		
43	1'-1 3/8"	470.731		
Brq. 123	1'-3 1/8"	470.799		
44	1'-5"	470.887		
45	1'-6 3/4"	470.984		
46	1'-8 3/4"	471.082		
47	1'-9 3/8"	471.177		
48	1'-10 5/16"	471.265		
49	1'-11 3/4"	471.348		
50	2'-0 7/16"	471.423		
51	2'-1"	471.486		
52	2'-1 5/16"	471.543		
53	2'-1 7/16"	471.592		
E. Susp. Δ	2'-1 3/8"	471.621		
Brq. 124	2'-1 3/8"	471.629		

Δ Theoretical Top of Slab Elev. shown.
Add 0.104 (1 1/4" Overlay) for Exp. Dam Elev.
See Section A-A Sheet 23.

Section	TABLE OF ELEVATIONS FOR UNIT 7							
	GIRDER W				GIRDER R			
	W	X@Gutter	Y	Z	*M	X	Y	Z
Brq. 124	2'-1 3/8"	471.629						
1	2'-2"	471.700						
2	2'-2 3/16"	471.791						
3	2'-3"	471.869						
4	2'-3 1/4"	471.940						
5	2'-3 1/2"	472.005						
6	2'-3 5/8"	472.065						
7	2'-2 1/2"	472.121						
8	2'-2 1/8"	472.177						
9	2'-1 3/8"	472.237						
10	2'-0 3/8"	472.308						
Brq. 125	1'-11 3/8"	472.393						
11	1'-11 1/4"	472.464						
12	1'-10 1/2"	472.565						
13	1'-9 3/4"	472.669			1'-5 3/4"	472.548		
14	1'-9"	472.767				472.639		
15	1'-8 1/2"	472.857				472.727		
16	1'-7 1/2"	472.943				472.810		
17	1'-6 3/4"	473.017				472.886		
18	1'-6"	473.084				472.953		
19	1'-5 3/8"	473.149				473.018		
20	1'-4 1/4"	473.203				473.078		
21	1'-3 1/4"	473.253				473.136		
22	1'-2 5/16"	473.305				473.194		
23	1'-2 1/16"	473.363				473.256		
Brq. 126	1'-1 3/8"	473.400				473.306		
24	1'-1 1/4"	473.444				473.361		
25	1'-0 1/2"	473.507				473.443		
26	0'-11 1/2"	473.575				473.528		
27	0'-10 3/4"	473.641				473.614		
28	0'-10"	473.713				473.697		
29	0'-9 1/4"	473.783				473.776		
30	0'-8 1/2"	473.848				473.849		
31	0'-7 3/4"	473.912				473.916		
32	0'-6 13/16"	473.970				473.976		
33	0'-6 3/16"	474.027				474.034		
34	0'-5 7/16"	474.078				474.087		
35	0'-4 1/16"	474.131				474.146		
36	0'-3 13/16"	474.182				474.203		
Brq. 127	0'-3 5/16"	474.225				474.248		
37	0'-2 13/16"	474.266				474.292		
38	0'-2 1/4"	474.321				474.357		
39	0'-2 1/8"	474.377				474.422		
40	0'-2 1/4"	474.430				474.486		
41	0'-2 3/4"	474.473				474.543		
42	0'-3 3/16"	474.507				474.590		
43	0'-4 3/4"	474.530				474.625		
44	0'-6 1/4"	474.541				474.647		
45	0'-8 1/8"	474.540				474.660		
46	0'-10 1/4"	474.531				474.664		
Brq. 128	1'-0 3/4"	474.518			1'-5 3/4"	474.665		

*M Dimension from Existing Girder G to Existing Gutter Line.
Elevation at Existing Gutter Line does not change

CONSTRUCTION NOTES

1. Take elevations on top of beam at points indicated (1-1, 2-2, etc.) after diaphragms are in place and all falsework has been removed but before forms for concrete slabs have been put in place. Read elevations to three decimals using a target rod and enter readings in "Table of Elevations", column Y.
2. Compute dimension Z as follows (See Typ. Section Sht. No. 31):
Elevation X minus Elevation Y equals dimension Z. Elevation X includes camber due to weight of concrete slab, curb, plinth, handrail and future surfacing. Measuring of dimension Z gives the final check on beam tolerances for camber, beam damage and errors in erection that produce reverse cambers, sags, and unsightly fascia beams.
3. For setting templates measure dimension Z above top of beam for top of templates. Do not set templates by elevations.
4. The elevations given on this sheet are theoretical elevations based on the "As Built" plans of the original bridge construction. The profile grade and cross slope shall be adjusted in the field as directed by the Engineer to insure the new slab meets the existing slab properly and adequate slab thickness is maintained.

SHEET 33

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON
ROAD
STATION 267+00
CONSTRUCTION PROJECT NO.
P. E. PROJECT NO. 164-2(92)
MAINTENANCE PROJECT NO.
DRAWING NO.
19238

CONSTRUCTION ELEVATIONS

LETTING DATE

DESIGNED BY: ELU/R.H.M. CHECKED BY: ELU/R.H.M. DATE: 3-75
 REVISION: 1 DATE: 3-75
 REVISION: 2 DATE: 3-75
 REVISION: 3 DATE: 3-75

Section	TABLE OF ELEVATIONS FOR UNIT 8																				Section
	GIRDER W				GIRDER V			GIRDER U			GIRDER T			GIRDER S			GIRDER R				
	W	X@Gutter	4	Z	X	4	Z	X	4	Z	X	4	Z	X	4	Z	*M	X	4	Z	
Brq. 128	1'-0 3/4"	474.518															1'-5 3/4"	474.665			Brq. 128
Susp. Δ	1'-1"	474.519									474.602							474.667			Susp. Δ
1	0'-11 3/8"	474.533									474.616							474.689			1
2	0'-10 3/8"	474.548									474.636							474.721			2
3	0'-9 3/8"	474.556									474.652							474.747			3
4	0'-8 1/2"	474.558									474.658							474.764			4
5	0'-8 1/2"	474.551									474.660							474.776			5
6	0'-8"	474.539									474.656							474.783			6
7	0'-8 3/8"	474.519									474.649							474.785			7
Brq. 129 A	0'-8 3/8"	474.503									474.644							474.789			Brq. 129 A
8	0'-8 3/8"	474.500									474.642							474.790			8
9	0'-7"	474.481									474.638							474.797			9
10	0'-6"	474.461									474.638							474.807			10
Brq. 129 E	0'-5 1/2"	474.434									474.630							474.809			Brq. 129 E
11	0'-5"	474.411									474.621							474.811			11
12	0'-4 3/8"	474.372									474.599							474.801			12
13	0'-5 3/8"	474.323									474.571							474.785			13
14	0'-5 1/2"	474.264						474.410			474.532			474.655				474.763			14
15	0'-6 1/2"	474.199						474.355			474.486			474.616				474.733			15
Brq. 129 B	0'-7 1/2"	474.139						474.301			474.443			474.572				474.706			Brq. 129 B
16	0'-7 1/2"	474.133						474.296			474.440			474.572				474.704			16
17	0'-6 3/8"	474.066						474.256			474.393			474.549				474.678			17
18	0'-5 1/2"	473.946						474.210			474.346			474.521				474.650			18
19	0'-4 1/2"	473.895						474.125			474.263			474.449				474.582			19
20	0'-4 3/8"	473.770						474.015			474.157			474.350				474.487			20
21	0'-4 1/2"	473.639						473.897			474.047			474.244				474.389			21
22	0'-5 1/2"	473.504						473.772			473.934			474.129				474.286			22
Brq. 130 E	0'-6"	473.409			473.555			473.686			473.854			474.050			1'-5 3/4"	474.213			Brq. 130 E
23	0'-6 3/8"	473.321			473.478			473.605			473.783			473.975			1'-3 3/4"	474.146			23
24	0'-7 3/8"	473.180			473.340			473.479			473.672			473.861			1'-0 3/4"	474.042			24
Brq. 130 A	0'-8 1/2"	473.131			473.288			473.437			473.633			473.822				474.007			Brq. 130 A
25	0'-7 1/2"	473.040			473.217			473.357			473.562			473.749			0'-9 3/8"	473.941			25
26	0'-6 3/8"	472.903			473.107			473.235			473.453			473.637			0'-6 1/2"	473.842			26
27	0'-5 1/2"	472.763			472.984			473.117			473.346			473.530			0'-3 3/8"	473.743			27
28	0'-4 1/2"	472.617			472.857			472.994			473.230			473.419			10'-0 1/2"	473.638			28
29	0'-4 1/2"	472.410			472.663			472.813			473.053			473.247			0'-0 3/8"	473.470			29
30	0'-5"	472.220			472.482			472.647			472.892			473.091			0'-0 3/8"	473.320			30
31	0'-5 3/8"	472.020			472.284			472.472			472.720			472.925			10'-2 1/4"	473.157			31
32	0'-6 1/2"	471.810			472.079			472.287			472.537			472.753			0'-6 1/2"	472.988			32
33	0'-7 1/2"	471.605			471.880			472.111			472.360			472.586			1'-0 1/2"	472.825			33
34	0'-9 1/2"	471.387			471.666			471.918			472.165			472.401			1'-8 3/8"	472.640			34
Susp. Δ	0'-9 3/8"	471.362			471.637			471.883			472.120			472.348			2'-0 1/8"	472.571			Susp. Δ
Brq. 131 E	0'-9 1/2"	471.350			471.625			471.870			472.106			472.332			1'-11 1/2"	472.553			Brq. 131 E

Section	TABLE OF ELEVATIONS FOR UNIT 9																				Section		
	GIRDER W				GIRDER V			GIRDER U				GIRDER T			GIRDER S			GIRDER R					
	W	X@Gutter	4	Z	X	4	Z	U	X@U	4	Z	X	4	Z	X	4	Z	*M	X	4		Z	
±Brq. 131 E	0'-9 1/2"	471.350			471.625			2'-2"	471.807			472.106			472.332			1'-11 1/2"	472.553			±Brq. 131 E	
35	0'-8 1/2"	471.225			471.498			2'-1 1/2"	471.688			471.966			472.183			1'-7 3/8"	472.396			35	
36	0'-8 1/2"	471.099			471.368			2'-0 3/8"	471.559			471.813			472.011			1'-4 1/2"	472.204			36	
37	0'-8 1/2"	470.994			471.261			2'-0 1/2"	471.451			471.680			471.860			1'-3 3/8"	472.034			37	
38	0'-8 1/2"	470.901			471.170			2'-0 3/8"	471.359			471.563			471.725			1'-4"	471.882			38	
39	0'-9 3/8"	470.794			471.066			2'-1 3/8"	471.252			471.434			471.578			1'-6 3/8"	471.718			39	
40	0'-10 1/2"	470.676			470.950			2'-2 3/8"	471.134			471.293			471.418			1'-11 3/8"	471.543			40	
±Brq. 101 E	0'-10 1/2"	470.676			470.943			2'-2 3/8"	471.118			471.267			471.383			2'-0 1/2"	471.499			±Brq. 101 E	

△ Theoretical Top of Slab Elevation shown.
 Add 0.104 (1/2 Overlay) for Exp. Dam Elevation.
 See Section A-A, Sheet 23.

*M Dimension from Existing Girder G
 to Existing Gutter Line. Elevation at Existing
 Gutter Line does not change.

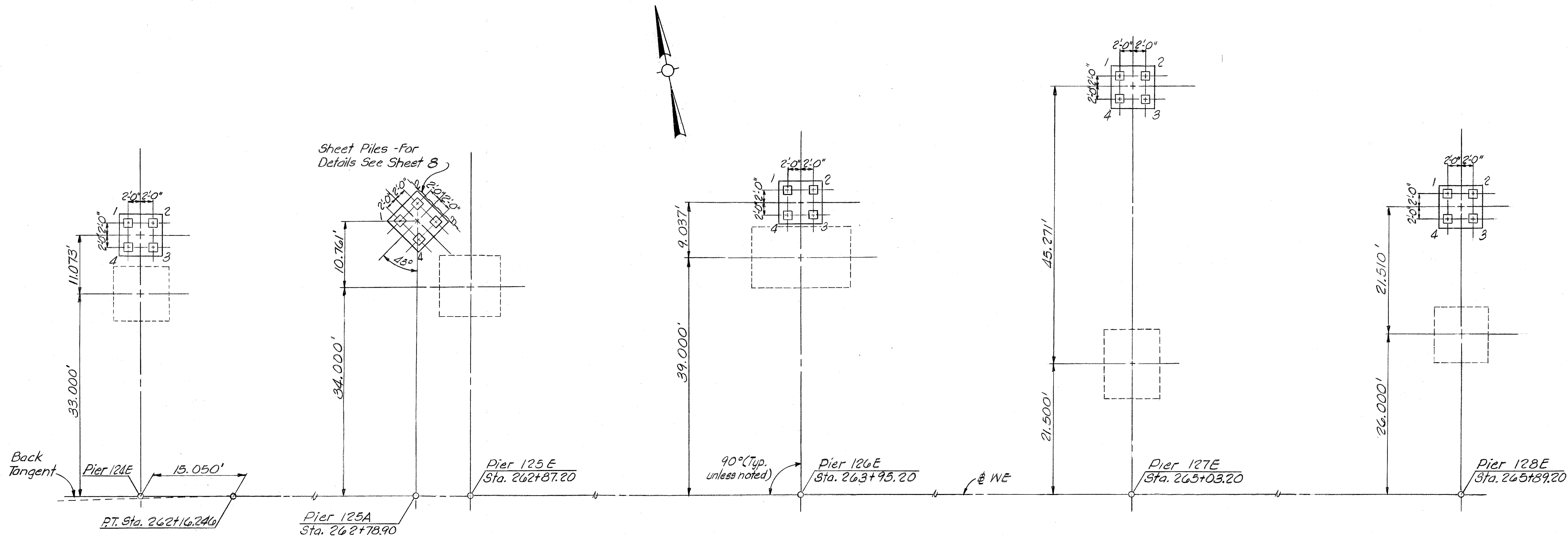
CONSTRUCTION ELEVATIONS

SHEET 34

COMMONWEALTH OF KENTUCKY
 BUREAU OF HIGHWAYS
 FRANKFORT
 COUNTY OF
JEFFERSON
 LOUISVILLE-LEXINGTON
 ROAD
 STATION 267+00
 CONSTRUCTION PROJECT NO.
 P.E. PROJECT NO. 164-2(92)
 MAINTENANCE PROJECT NO.
 DRAWING NO.
19238

UPDATE DATE
LETTING DATE

DESIGNED BY	DATE	REVISION	DATE	REVISION	DATE
BY	3-73				
CHECKED BY					
TRACED BY					



PIER 124E				
Pile No	Cutoff Elevation	Tip of Pile As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	433.50			
2				
3				
4				

Max. Pile Tip Elev. = 405'

PIER 125A				
Pile No	Cutoff Elevation	Tip of Pile As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	432.23			
2				
3				
4				

Max. Pile Tip Elev. = 406'

PIER 126E				
Pile No	Cutoff Elevation	Tip of Pile As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	433.97			
2				
3				
4				

Max. Pile Tip Elev. = 406'

PIER 127E				
Pile No	Cutoff Elevation	Tip of Pile As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	435.91			
2				
3				
4				

Max. Pile Tip Elev. = 406'

PIER 128E				
Pile No	Cutoff Elevation	Tip of Pile As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	436.30			
2				
3				
4				

Max. Pile Tip Elev. = 405'

PILE RECORD - After all piles have been driven, the Resident Engineer shall record for each pile, the tip-of-pile elevation as driven, the length of pile in place and the calculated bearing capacity and shall return one blue print copy of this sheet with this data to the Director of the Division of Bridges so that the data may be recorded on the original plans. Lengths of piles in place shown hereon are the actual lengths of piles in the finished structure below cutoff elevation and are not necessarily pay items. This pile record does not replace other records of piles required to be kept and submitted by the Resident Engineer.

PILE RECORD

SHEET 35

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

FRANKFORT

COUNTY OF

JEFFERSON

LOUISVILLE-LEXINGTON

ROAD

STATION 267+00

P. E. PROJECT NO. 164-2(92)

CONSTRUCTION PROJECT NO.

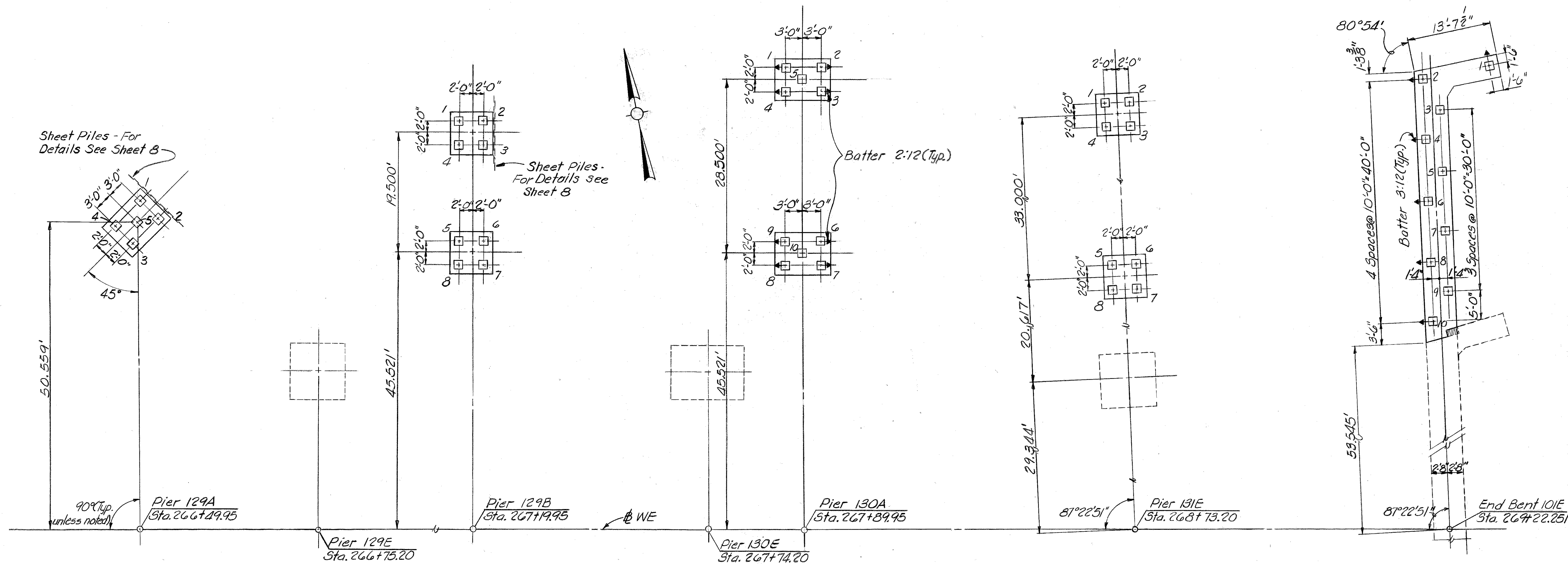
MAINTENANCE PROJECT NO.

DRAWING NO.

19238

UPDATE DATE
LETTING DATE

DESIGNED BY: RMH CHECKED BY: RB DATE: 1-75
TRACED BY: RMH CHECKED BY: RB DATE: 1-75



PIER 129A				
Pile No	Cutoff Elevation	Tip of Pile Elevation As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	435.72			
2				
3				
4				
5				

Max. Pile Tip Elev. = 404'

PIER 129B				
Pile No	Cutoff Elevation	Tip of Pile Elevation As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	435.36			
2				
3				
4				
5	435.40			
6				
7				
8				

Max. Pile Tip Elev. = 404'

PIER 130A				
Pile No	Cutoff Elevation	Tip of Pile Elevation As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	435.87			
2				
3				
4				
5				
6	435.73			
7				
8				
9				
10				

Max. Pile Tip Elev. = 404'

PIER 131E				
Pile No	Cutoff Elevation	Tip of Pile Elevation As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	436.40			
2				
3				
4				
5	436.28			
6				
7				
8				

Max. Pile Tip Elev. = 403'

END BENT 101E				
Pile No	Cutoff Elevation	Tip of Pile Elevation As Driven	Length of Pile in Place	Calculated Bearing Capacity-Tons
1	460.49			
2				
3				
4				
5				
6				
7				
8				
9				
10	460.49			

Max. Pile Tip Elev. = 401'

SHEET 36

COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON
ROAD
STATION 267+00 P. E. PROJECT NO. 164-2(92)
CONSTRUCTION PROJECT NO. MAINTENANCE PROJECT NO. DRAWING NO. 19238

PILE RECORD

DESIGN: DESIGNED FOR A SIGN AREA OF 600 SQ FT IN ACCORDANCE WITH "SPECIFICATIONS FOR DESIGN AND CONSTRUCTION OF STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS" PUBLISHED BY AASHO, 1968 WITH WIND REDUCED TO 80 MPH IN ACCORDANCE WITH IM 40-1-69 OF THE FEDERAL HIGHWAY ADMINISTRATION.

SUPERELEVATION OF ROADWAY: THE CONTRACTOR SHALL ALLOW FOR DIFFERENCES IN ELEVATION ACROSS THE FULL SHOULDER WIDTH AS SHOWN ON THE ROADWAY PLANS IN MAINTAINING THE REQUIRED 18 FOOT MINIMUM VERTICAL CLEARANCE TO THE BOTTOM OF THE LOWEST PART OF THE SIGNS OR SUPPORTS. SIGNS ARE TO BE CENTERED OVER THE LANE OR LANES TO WHICH THEY APPLY UNLESS SHOWN OTHERWISE.

~~CONCRETE: CLASS "A" CONCRETE IS TO BE USED THROUGHOUT.~~

~~BEVELED EDGES: ALL EXPOSED CONCRETE EDGES ARE TO BE BEVELED 7/8" UNLESS OTHERWISE SHOWN.~~

~~REINFORCEMENT: DIMENSIONS FROM FACE OF CONCRETE TO BARS ARE CLEAR DISTANCES EXCEPT AS OTHERWISE SHOWN. DIMENSIONS FOR BAR SPACINGS ARE DISTANCES CENTER TO CENTER OF BARS.~~

SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS TO THE DIVISION OF CONSTRUCTION FOR APPROVAL PRIOR TO FABRICATION IN ACCORDANCE WITH THE SPECIFICATIONS.

FABRICATION OF WELDED ALUMINUM STRUCTURES: THE ALUMINUM SIGN SUPPORT SHALL BE FABRICATED IN ACCORDANCE WITH AASHTO'S SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS.

MATERIAL SPECIFICATIONS: THE FOLLOWING ASTM DESIGNATIONS SHALL GOVERN ALL MATERIAL USED:

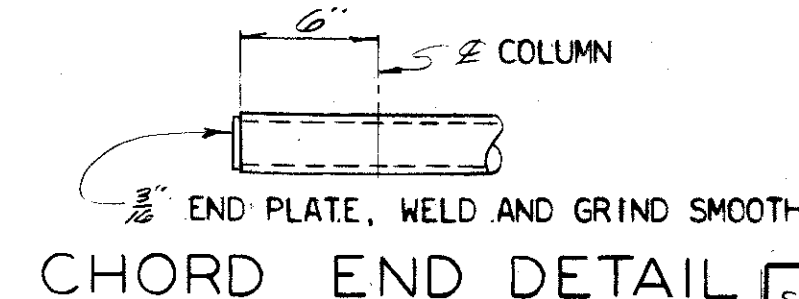
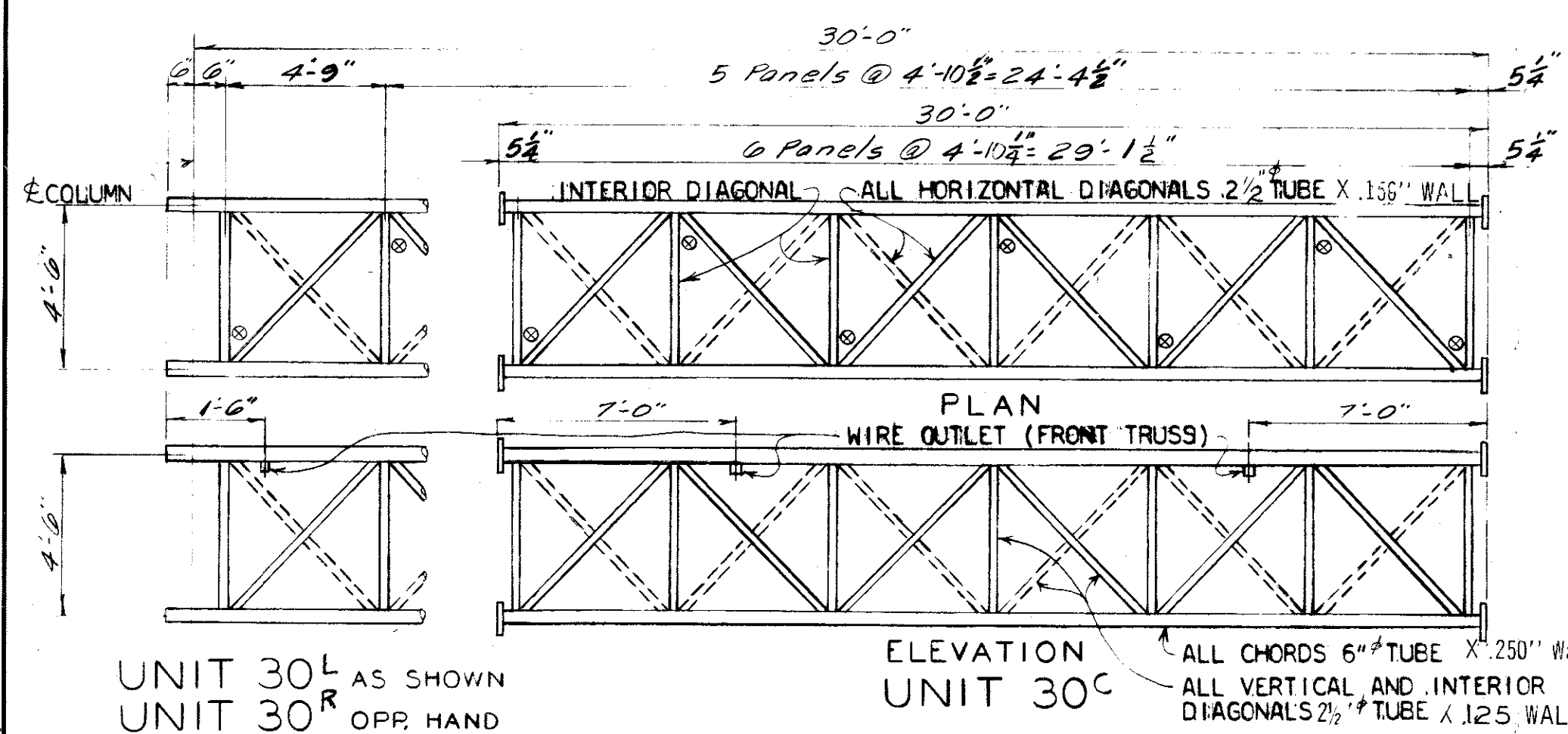
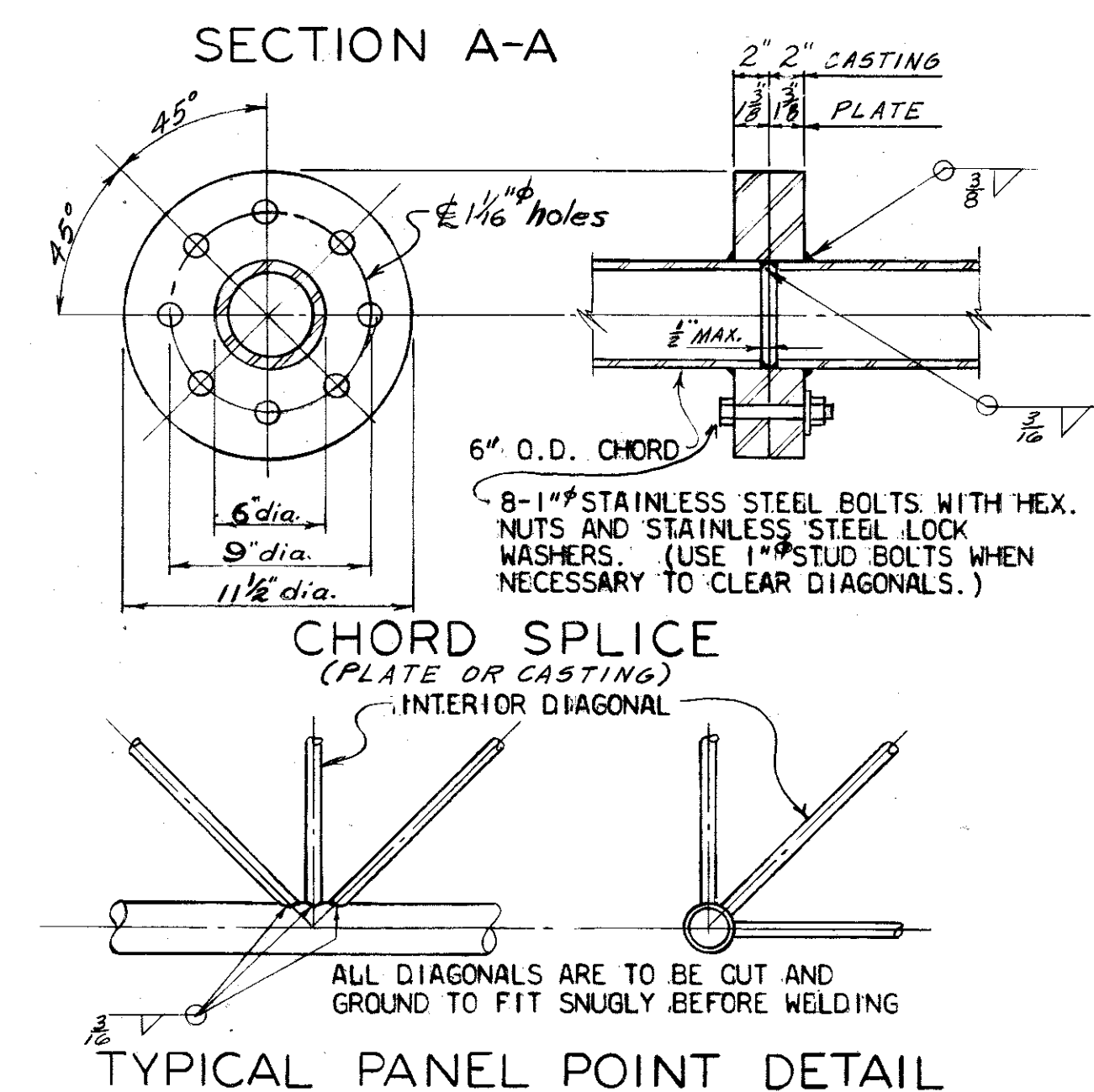
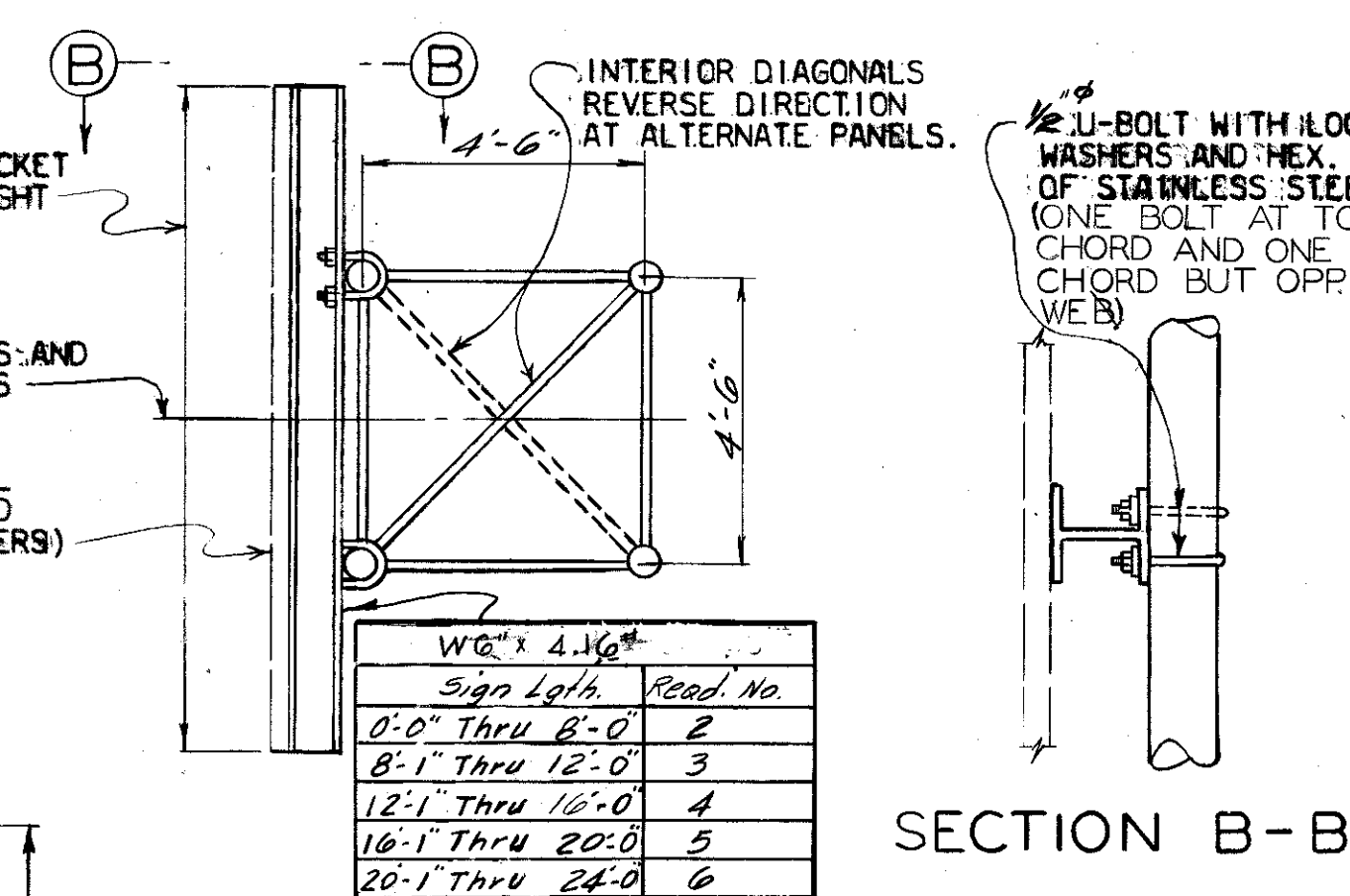
ASTM	MATERIAL
B221-72	72 EXTRUDED TUBE, ALUMINUM ALLOY 6061-T6511
B241-72	72 PIPE, ALUMINUM ALLOY 6061-T6
B308-72	72 STRUCTURAL SHAPES, ALUMINUM ALLOY 6061-T6
B221-72	72 EXTRUDED BAR, ROD AND SHAPES, ALUMINUM ALLOY 6061-T6511
B209-72a	72 SHEET AND PLATE, ALUMINUM ALLOY 6061-T6
A320-70	70 STAINLESS STEEL HARDWARE , NUTS, BOLTS AND WASHERS
A165-71	71 TYPE NS, CADMIUM COATING FOR ASTM A36 ANCHOR BOLTS, NUTS AND WASHERS
B26-72	72 SAND MOLD CASTINGS, ALUMINUM ALLOY SG70A - T6
B108-72	72 PERMANENT MOLD CASTINGS, ALUMINUM ALLOY SG70B HEAT TREATED TO A TEMPER T6 INSTEAD OF TEMPER T61

~~FOOTINGS: ALL FOOTINGS SHALL BE POURED AGAINST UNDISTURBED EARTH AND ARE DESIGNED TO TRANSFER NO MORE THAN 1½ TONS PER SQUARE FOOT BEARING PRESSURE TO THE SOIL UNDER ANY DESIGN LOADING CONDITION.~~

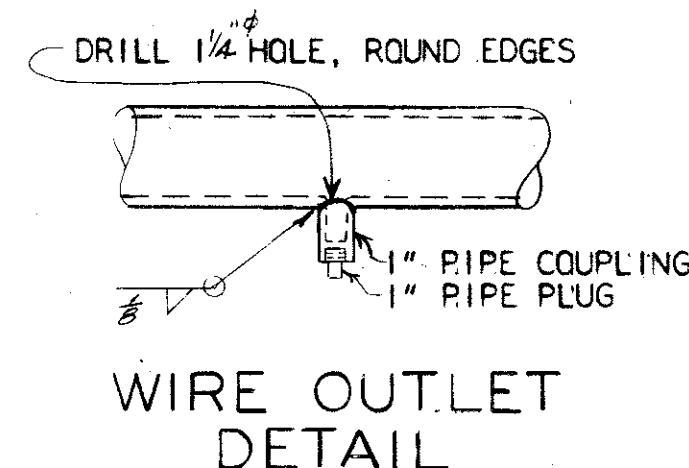
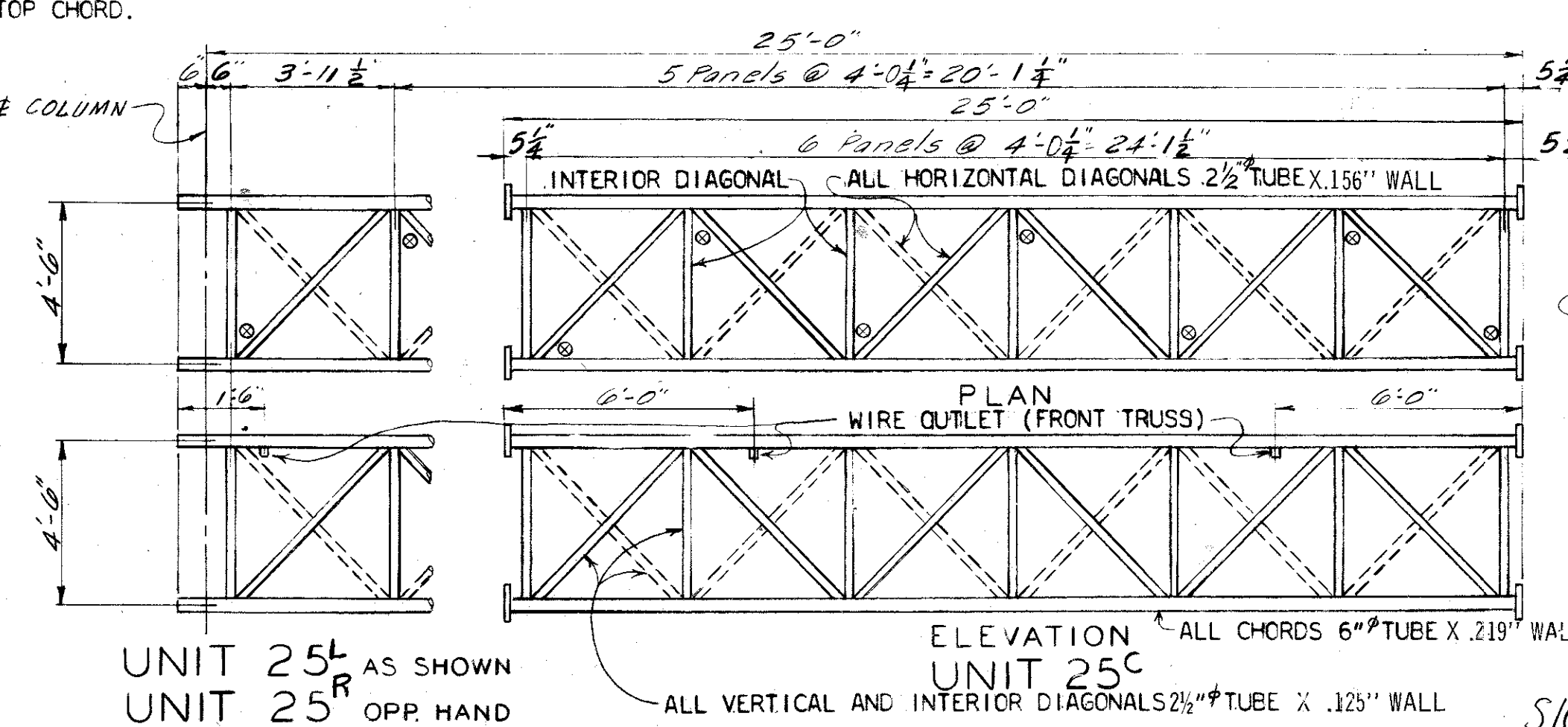
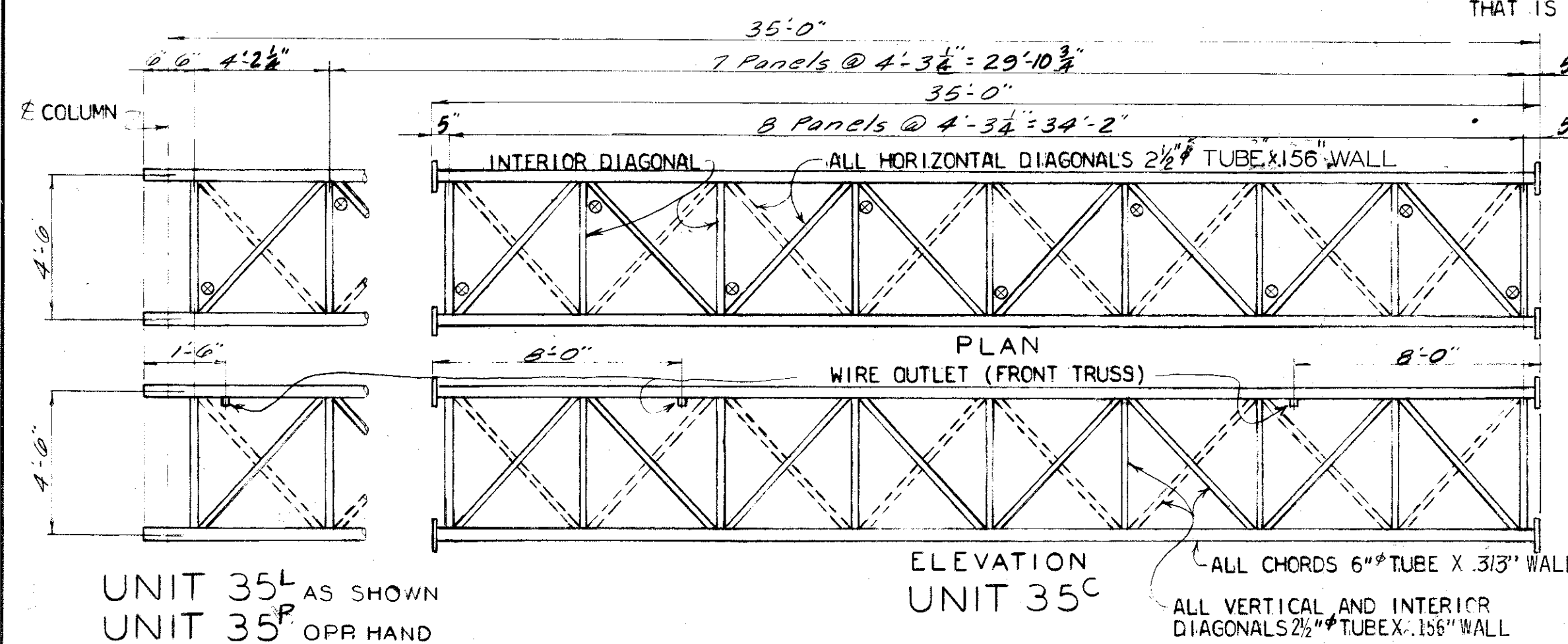
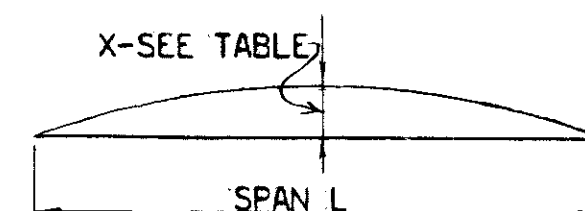
VERTICAL DIMENSIONS: VERTICAL DIMENSIONS H_R AND H_L SHALL NOT EXCEED 27' AND THE COMBINED DIMENSIONS $(H_R + F_R)$ OR $(H_L + F_L)$ SHALL NOT EXCEED 36'.

MILL TEST REPORTS: NOTARIZED TEST REPORTS IN TRIPPLICATE SHALL BE FURNISHED THE DEPARTMENT OF HIGHWAYS STATING THAT THE MATERIALS USED CONFORM TO THE SPECIFICATIONS.

Note: Fabricator shall modify length of a Standard Unit to make the required "Span L" of Sign Support. Sign information does not include "Exit No." signs.



L	X
75'	24'
80'	25'
85'	26'
90'	3''
95'	34'
100'	35'
105'	36'



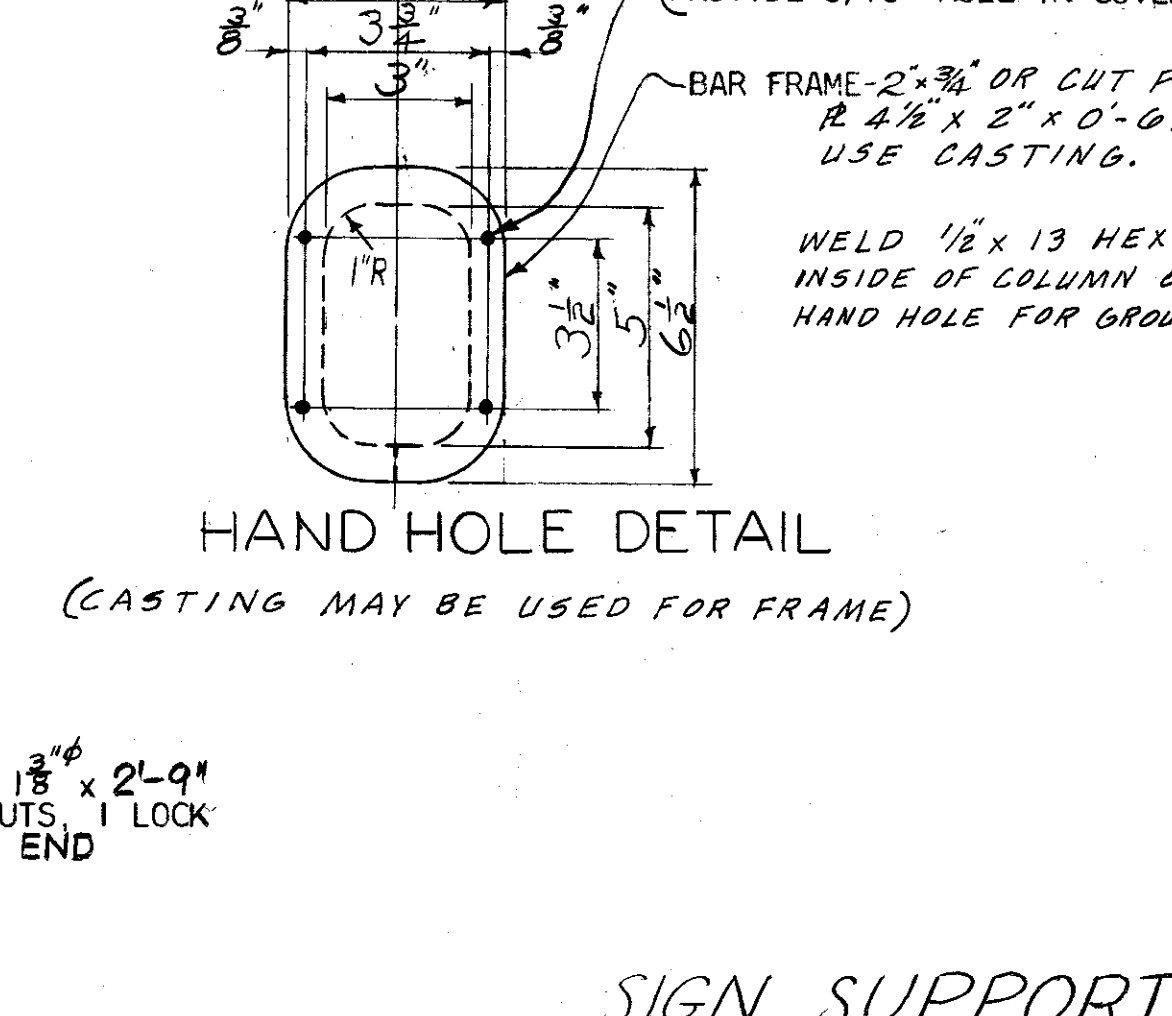
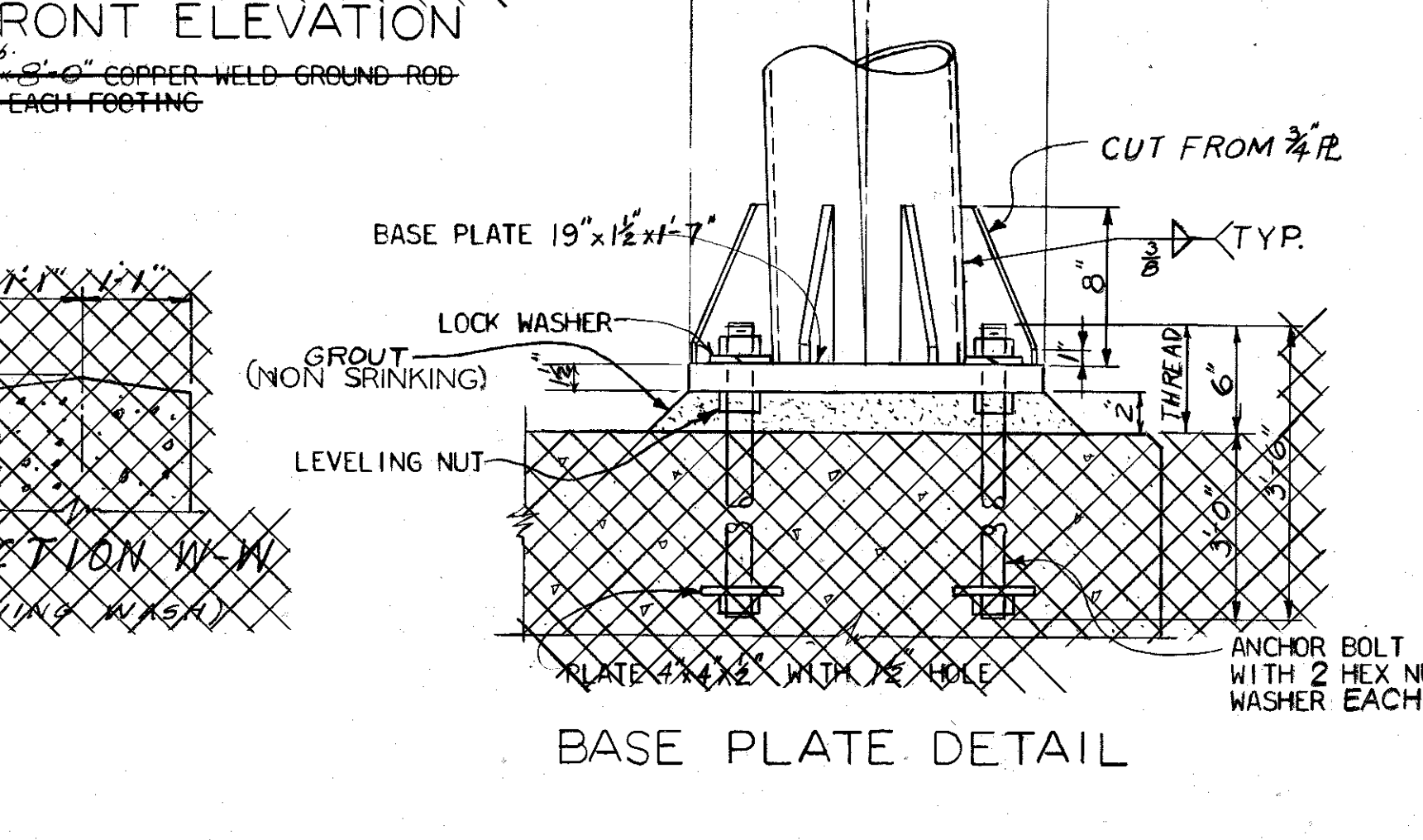
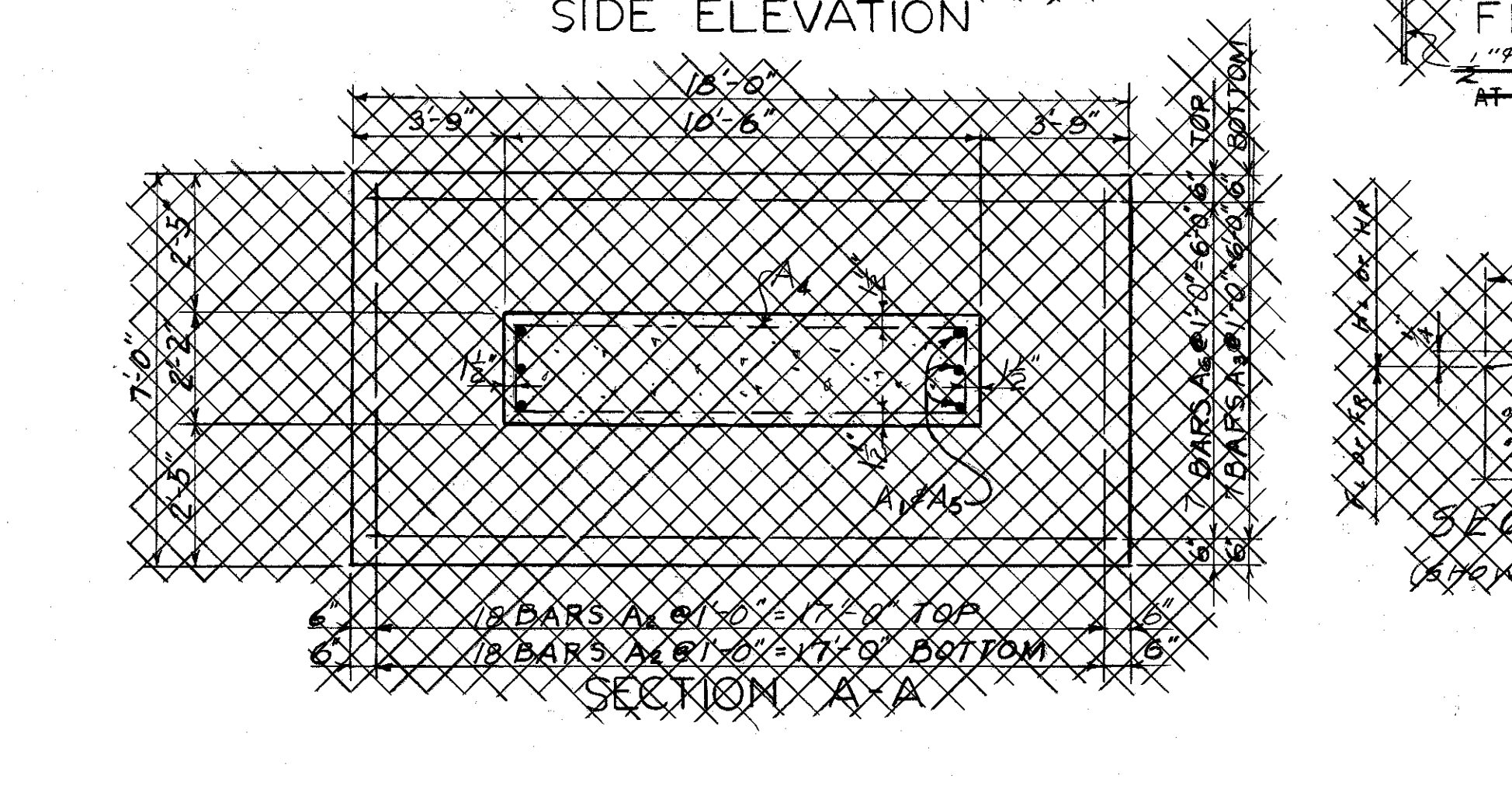
COMMONWEALTH OF KENTUCKY
DEPARTMENT OF HIGHWAYS
FRANKFORT

STANDARD OVERHEAD
SIGN SUPPORTS
75' TO 105' SPANS
ALUMINUM

STATION PROJECT NO.

APPROVED *J. H. Brown* DRAWING 19238 IND

SIGN SUPPORT



Technical drawing of Shim S3 (Plate or Gating). The drawing shows a side view and a cross-section. The side view dimensions are: total width 6 1/2", two segments of 3 1/4" each, and a height of 6 3/8". The cross-section shows a thickness of 13/16" Dia. and a total height of 6 1/8". The cross-section also shows a 1/4" dimension and a 1/16" dimension.

SHIM S3
(PLATE OR GATING)

STATION		PROJECT NO.	
APPROVED	<i>[Signature]</i> STATE HIGHWAY ENGINEER	DRAWING 19238 NO.	INDEX

GENERAL NOTE

SPECIFICATIONS: THE KENTUCKY DEPARTMENT OF HIGHWAYS STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, CURRENT EDITION WITH REVISIONS, SHALL APPLY TO THIS PROJECT.
DESIGN: DESIGNED FOR A SIGN AREA OF 630 SQ FT IN ACCORDANCE WITH "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS" PUBLISHED BY AASHO, 1968 WITH THE WIND REDUCED TO 80 MPH IN ACCORDANCE WITH IM 40-1-69 OF THE FEDERAL HIGHWAY ADMINISTRATION.

SUPERELEVATION OF ROADWAY: THE CONTRACTOR SHALL ALLOW FOR DIFFERENCES IN ELEVATION ACROSS THE FULL SHOULDER WIDTH AS SHOWN ON THE ROADWAY PLANS IN MAINTAINING THE REQUIRED 18 FOOT MINIMUM VERTICAL CLEARANCE TO THE BOTTOM OF THE LOWEST PART OF THE SIGNS OR SUPPORT. SIGNS ARE TO BE CENTERED OVER THE LANE OR LANES TO WHICH THEY APPLY UNLESS SHOWN OTHERWISE.

CONCRETE: CLASS "A" CONCRETE IS TO BE USED THROUGHOUT.

BEVELED EDGES: ALL EXPOSED CONCRETE EDGES ARE TO BE BEVELED 7/8" UNLESS OTHERWISE SHOWN.

REINFORCEMENT: DIMENSIONS FROM FACE OF CONCRETE TO BARS ARE CLEAR DISTANCES EXCEPT AS OTHERWISE SHOWN. DIMENSIONS FOR BAR SPACINGS ARE DISTANCES CENTER TO CENTER OF BARS.

SHOP DRAWINGS: THE CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS TO THE DIVISION OF CONSTRUCTION FOR APPROVAL PRIOR TO FABRICATION IN ACCORDANCE WITH THE SPECIFICATIONS.

FABRICATION: THE SIGN SUPPORT SHALL BE FABRICATED IN ACCORDANCE WITH THE SPECIFICATIONS. ALL METAL COMPONENTS OF THE SUPPORT EXCEPT THE STAINLESS STEEL HARDWARE SHALL BE HOT-DIP GALVANIZED AFTER ALL FABRICATION HAS BEEN COMPLETED. THE GALVANIZED MATERIAL SHALL BE LOADED, HAULED, AND HANDLED IN SUCH A MANNER THAT THE GALVANIZING WILL NOT BE DAMAGED. ALL ABRADED AND DAMAGED SURFACES SHALL BE REGALVANIZED OR REPAIRED BY PAINTING WITH TWO COATS OF ZINC OXIDE, ZINC DUST PAINT CONFORMING TO THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-15145. THE PAINT IS TO BE PROPERLY COMPOUNDED IN A SUITABLE VEHICLE IN THE RATIO OF ONE PART ZINC OXIDE TO FOUR PARTS ZINC DUST, BY WEIGHT. ALL REPAIRS ARE TO BE AS DIRECTED BY THE ENGINEER.

MILL TEST REPORTS: NOTARIZED TEST REPORTS IN TRIPLICATE SHALL BE FURNISHED THE DEPARTMENT OF HIGHWAYS STATING THAT THE MATERIALS USED CONFORM TO THE SPECIFICATIONS.

WELDERS AND WELDING PROCEDURE: WELDED FABRICATION SHALL BE LIMITED TO THE INERT-GAS SHIELDED, TUNGSTEN OR CONSUMABLE ELECTRODE METHODS OF ARC WELDING. WELDING PROCEDURE, WELDERS AND WELDING OPERATORS SHALL BE QUALIFIED ACCORDING TO SECTION IX OF THE ASME BOILER AND PRESSURE VESSEL CODE, 1971 EDITION; PART A, REQUIREMENTS FOR FERROUS METALS. CERTIFIED COPIES IN TRIPLICATE OF ASME BOILER CODE, APPENDIX II, QUALIFICATION TEST FORMS Q-1, Q-1G AND Q-1F SHALL BE SUBMITTED TO THE DEPARTMENT OF HIGHWAYS. THE PERTINENT FORMS SHALL INDICATE ACCEPTABLE QUALIFICATION OF WELDING PROCEDURES AND WELDER PERFORMANCE ON GROOVE AND FILLET WELDS.

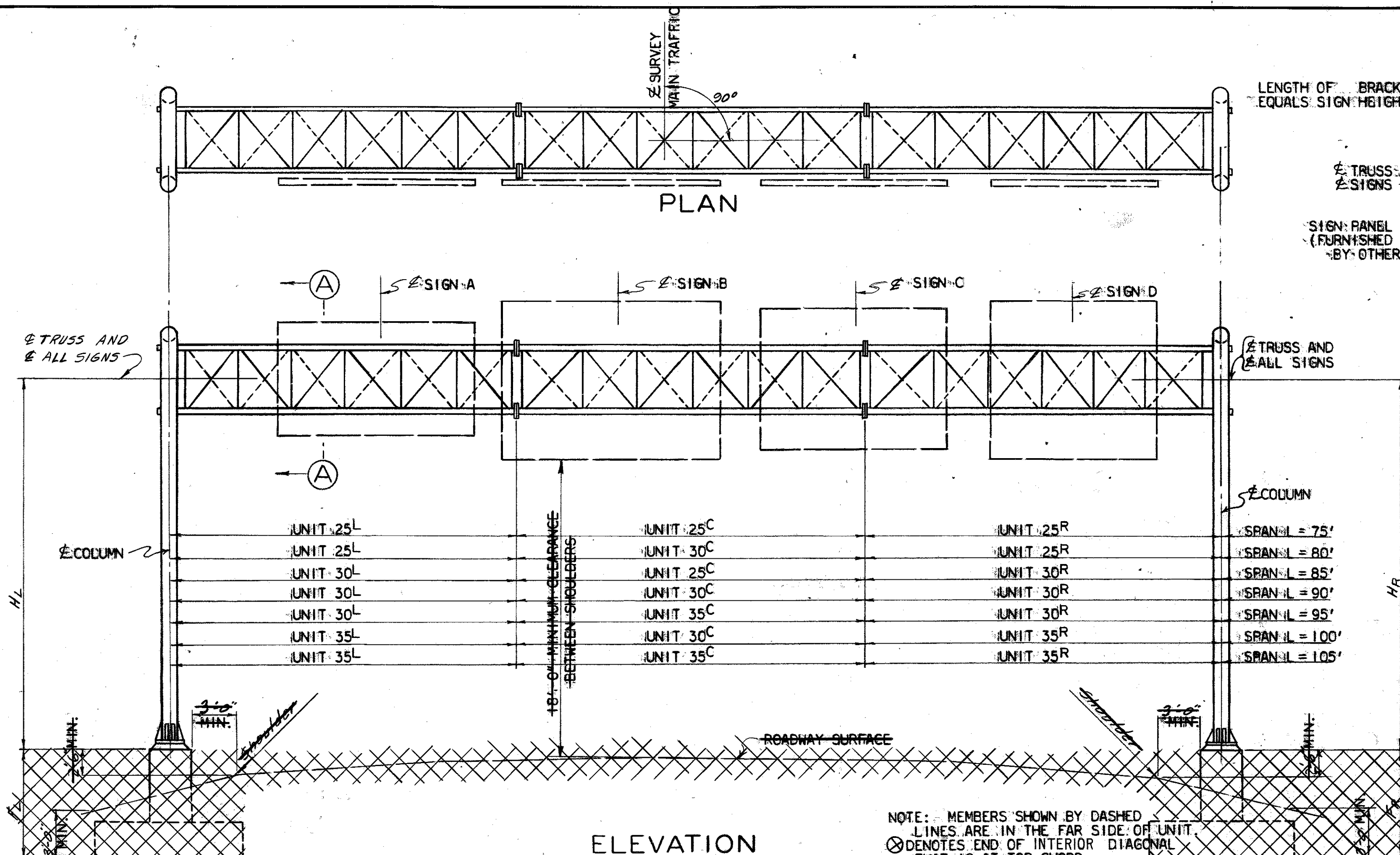
MATERIAL SPECIFICATIONS: THE FOLLOWING ASTM DESIGNATIONS SHALL GOVERN ALL MATERIALS USED:

ASTM	MATERIAL
A53-71a	GRADE B SEAMLESS STEEL PIPE GALVANIZED IN ACCORDANCE WITH ASTM A-123
A36-70a	STRUCTURAL SHAPES, PLATES, BARS AND ANCHOR BOLTS, GALVANIZED IN ACCORDANCE WITH ASTM A-123
A320-70	STAINLESS STEEL HARDWARE - NUTS, BOLTS AND WASHERS
A106-71	GRADE B SEAMLESS CARBON STEEL PIPE GALVANIZED IN ACCORDANCE WITH ASTM A-123 MAY BE USED FOR PIPE LESS THAN 2"
A27-71	CARBON STEEL CASTING - GRADE 70-36

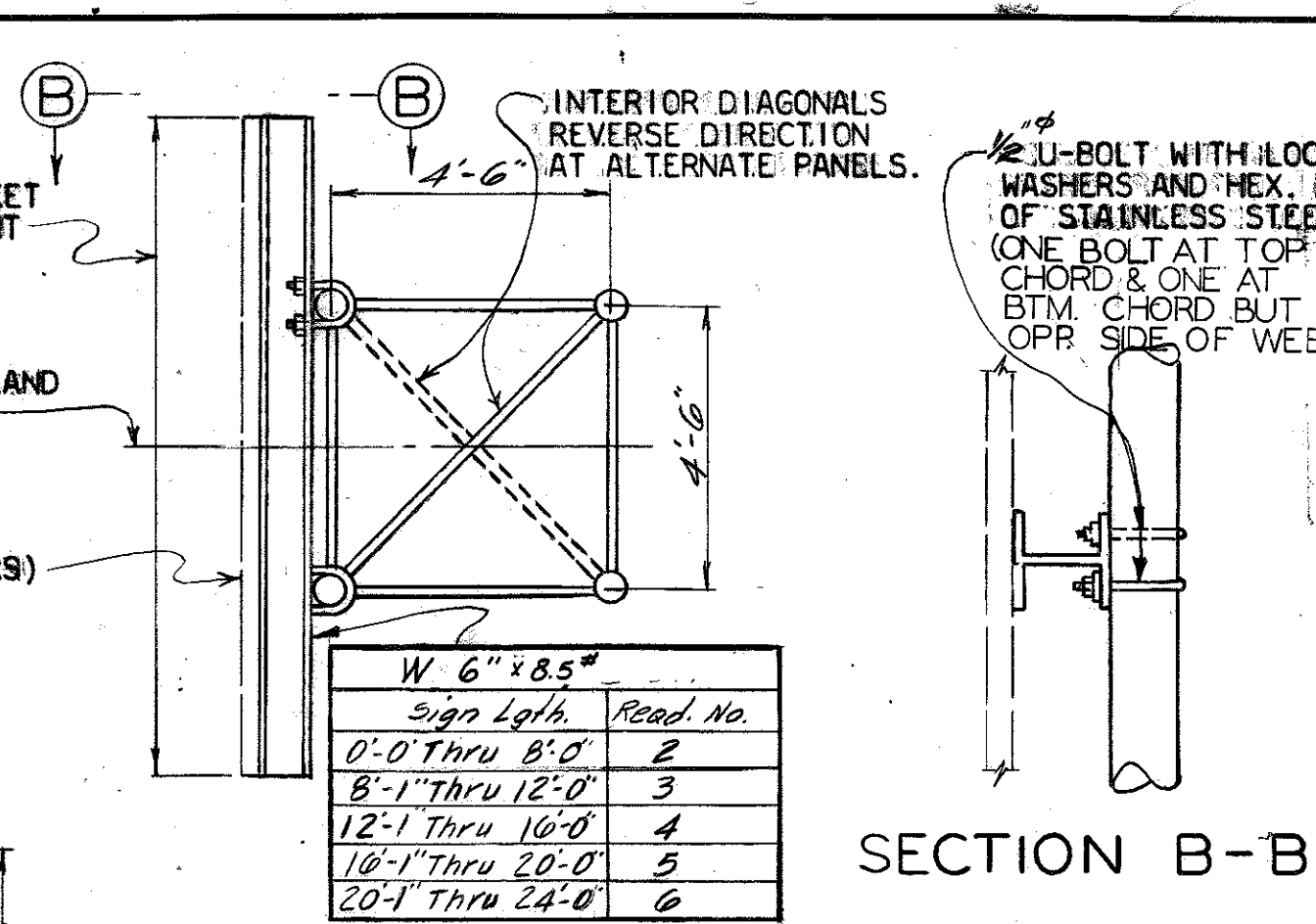
FOOTINGS: ALL FOOTINGS SHALL BE POURED AGAINST UNDISTURBED EARTH AND ARE DESIGNED TO TRANSFER NO MORE THAN 1 1/2 TONS PER SQUARE FOOT BEARING PRESSURE TO THE SOIL UNDER ANY DESIGN LOADING CONDITION.

VERTICAL DIMENSIONS: VERTICAL DIMENSIONS H_R AND H_L SHALL NOT EXCEED 27' AND THE COMBINED DIMENSIONS (H_R + F_R) OR (H_L + F_L) SHALL NOT EXCEED 36'.

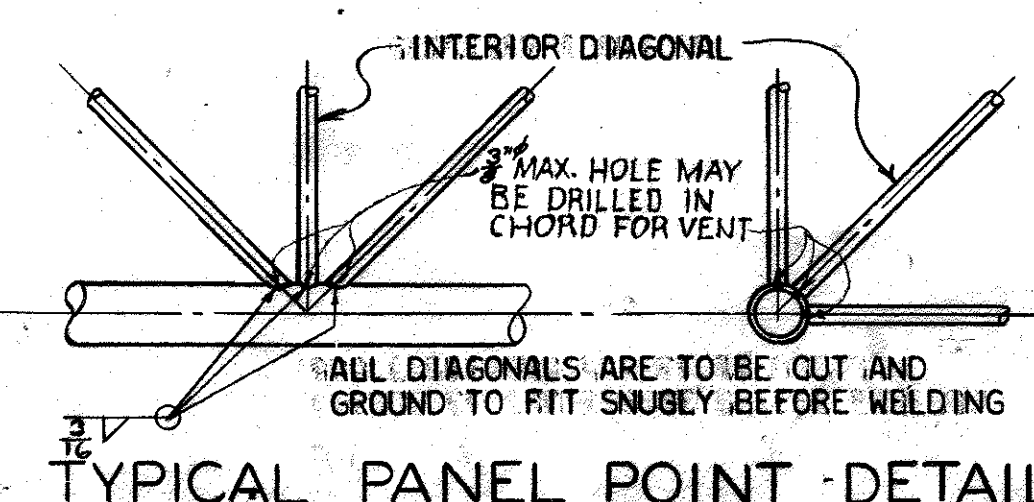
Note: Fabricator shall modify length of a Standard Unit to make the required "Span L" of Sign Support. Sign information does not include "Exit No." signs.



NOTE: MEMBERS SHOWN BY DASHED LINES ARE IN THE FAR SIDE OF UNIT. ⊗ DENOTES END OF INTERIOR DIAGONAL THAT IS AT TOP CHORD.



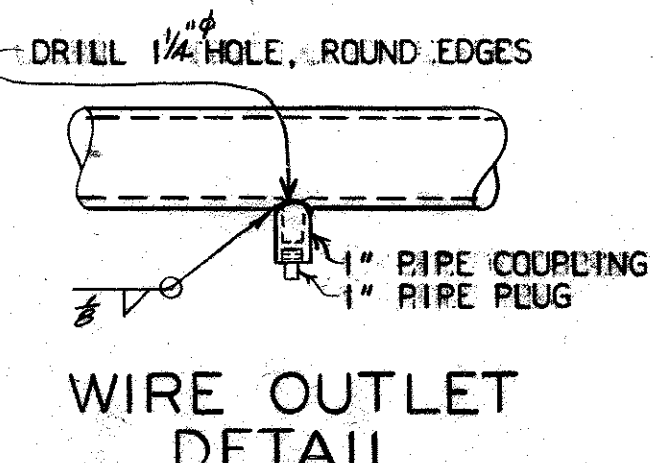
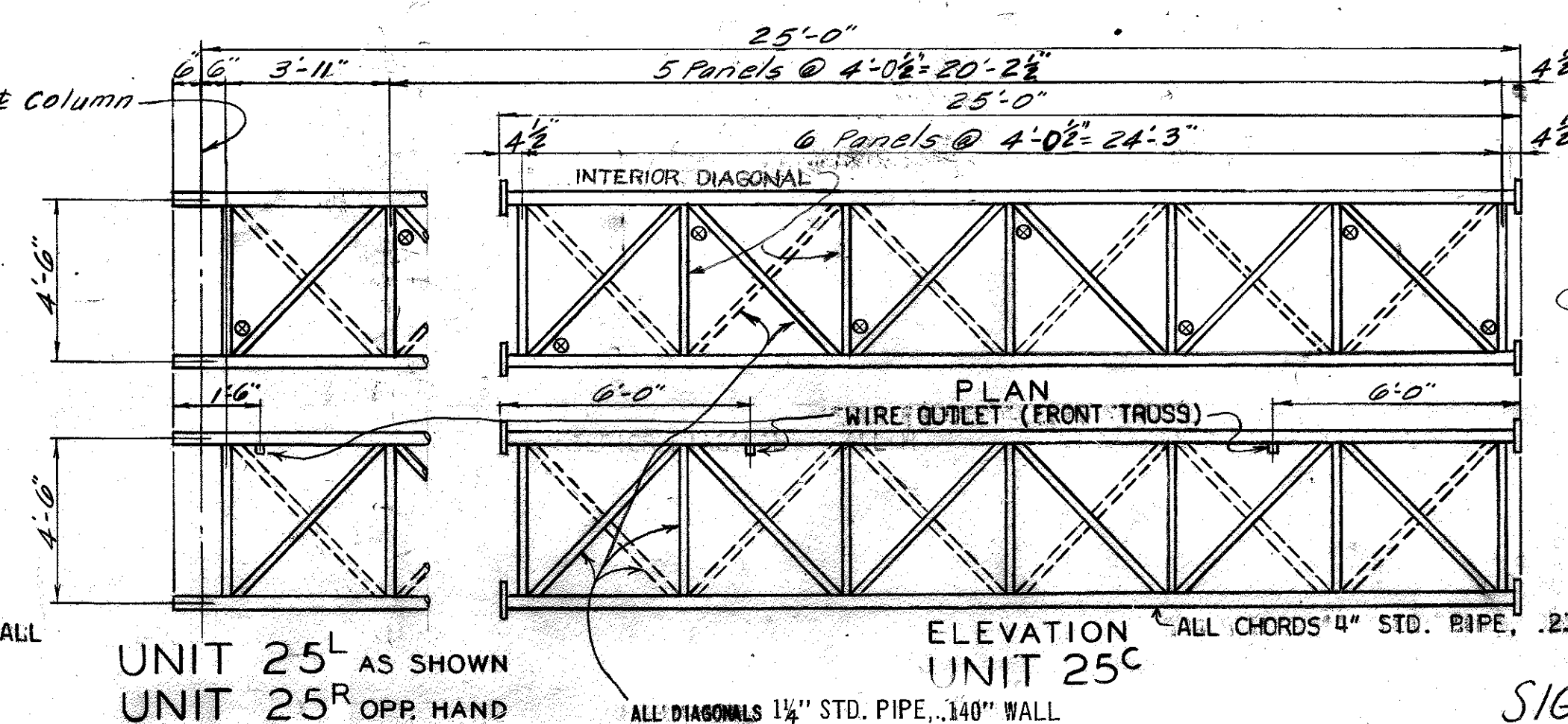
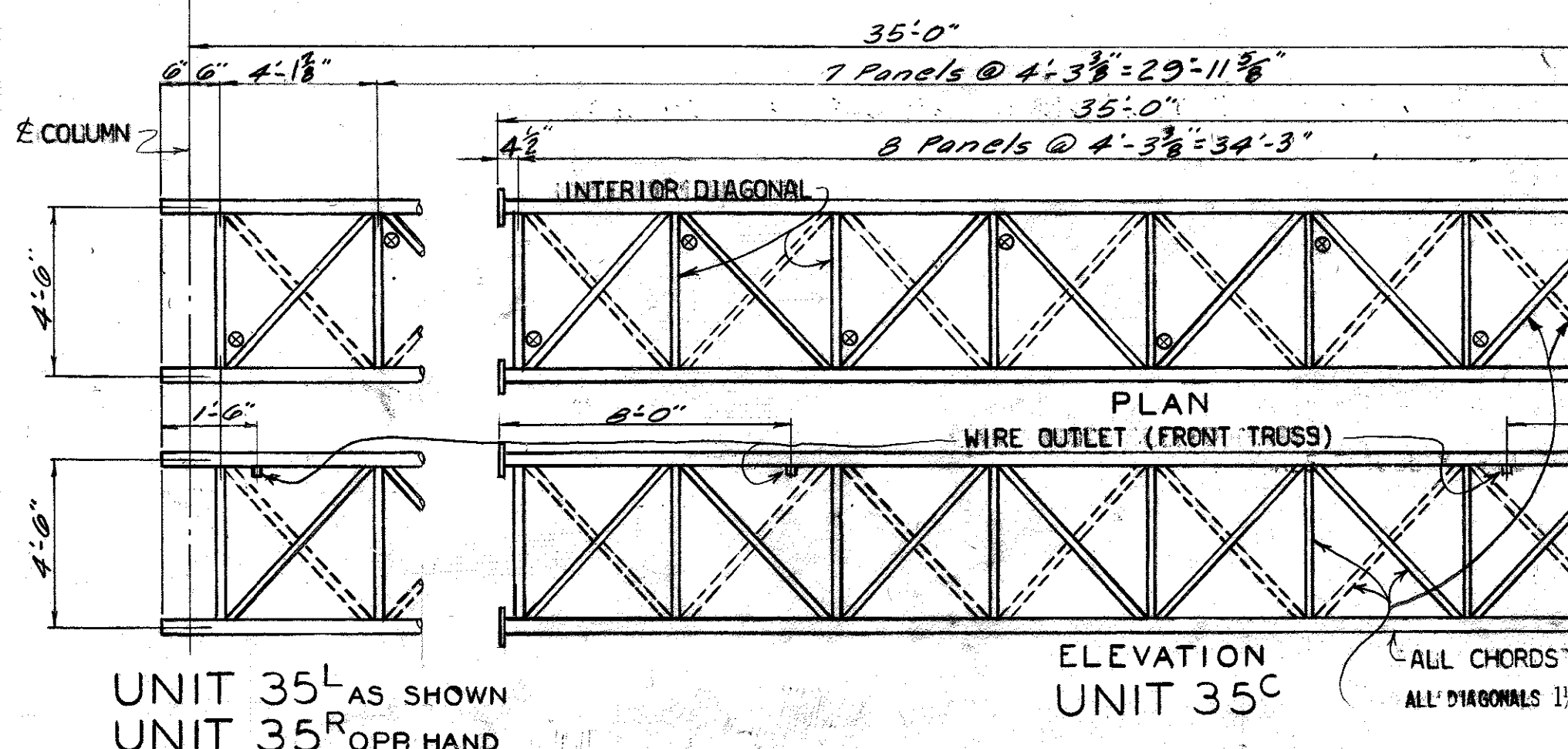
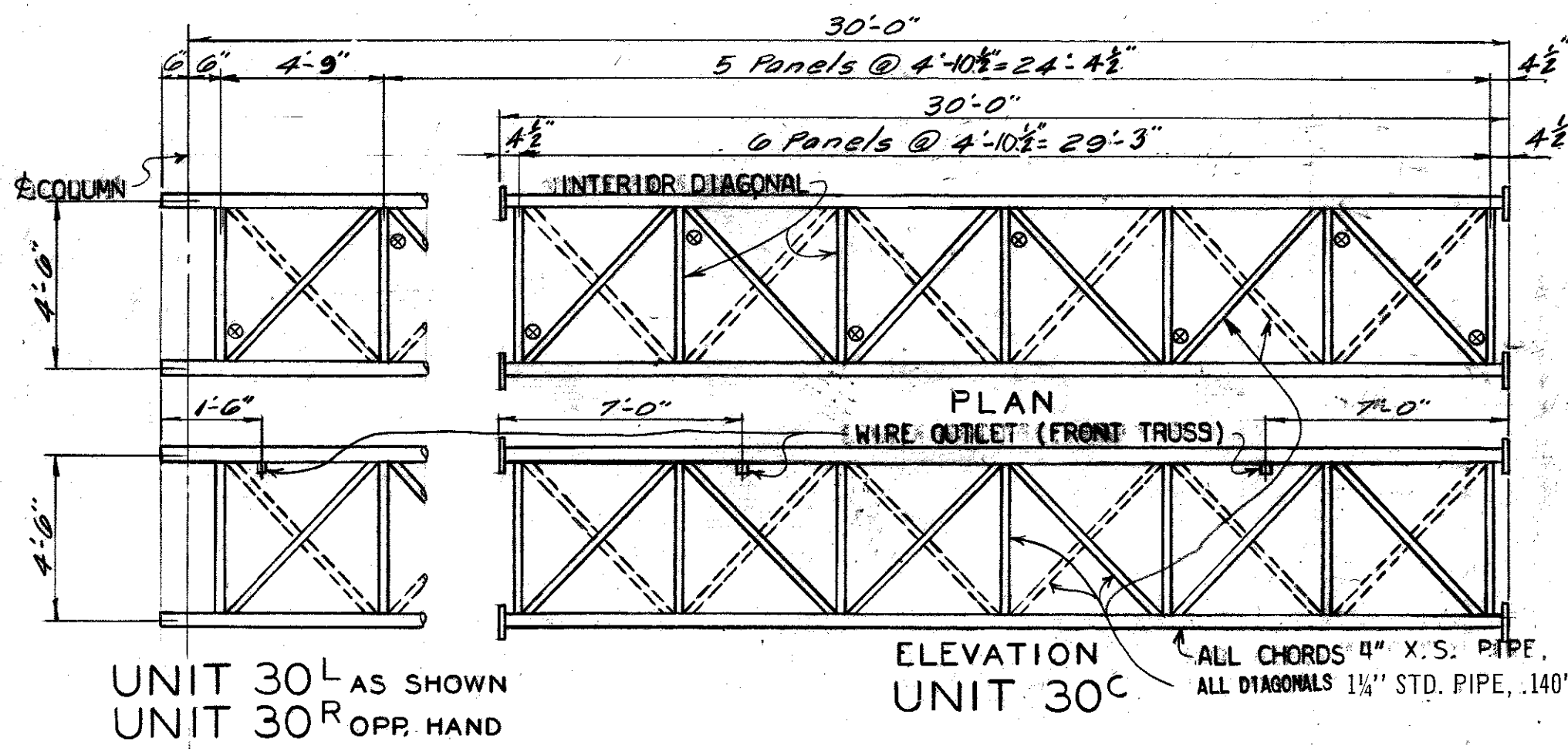
CHORD SPLICE



CHORD END DETAIL

L	X
75'	2 1/2"
80'	2 1/2"
85'	2 1/2"
90'	3"
95'	3 1/2"
100'	3 1/2"
105'	3 1/2"

CAMBER DIAGRAM



SIGN SUPPORT

Sheet 1 of 2
SHEET 39

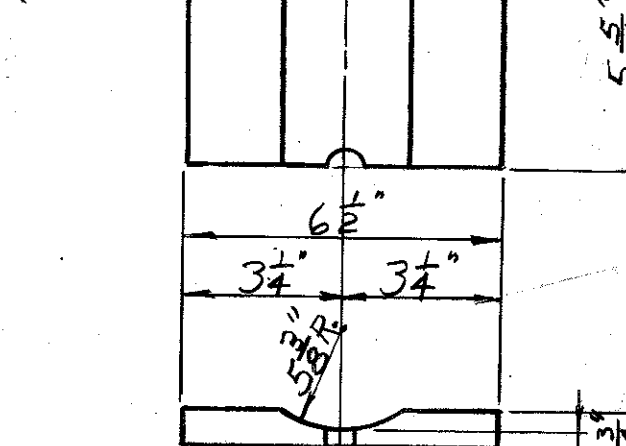
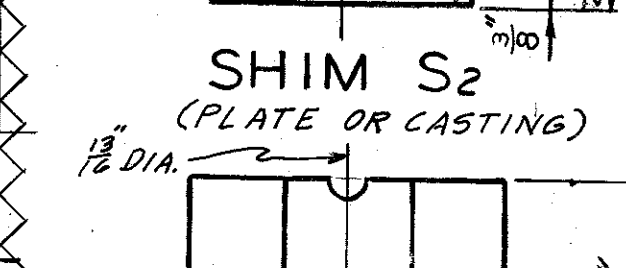
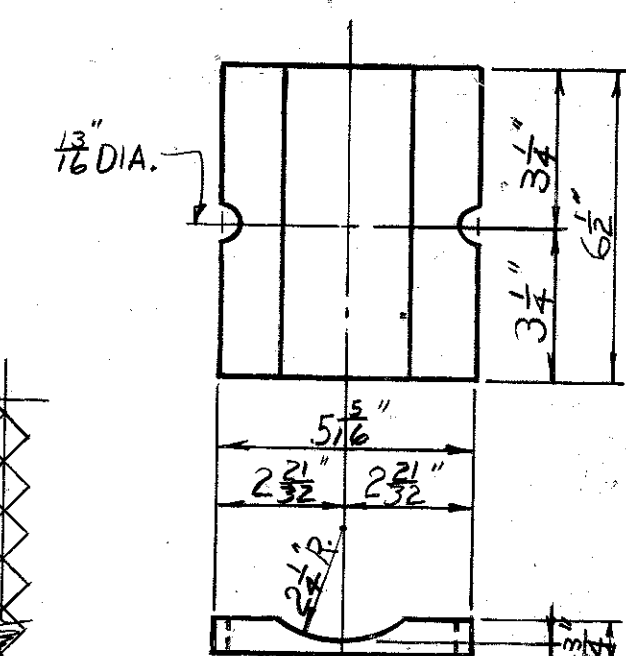
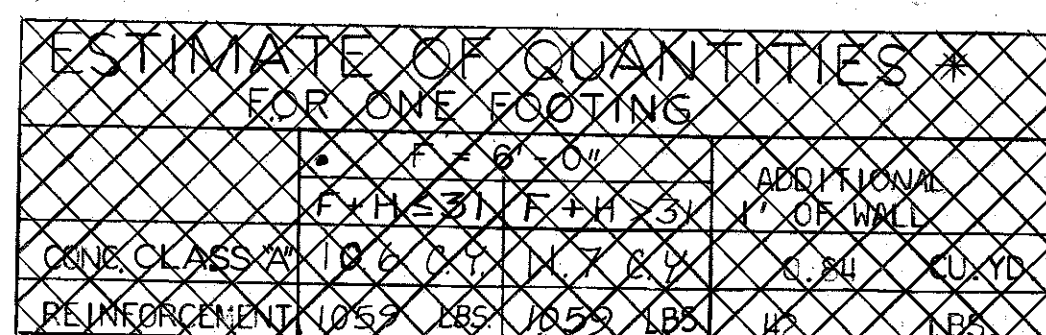
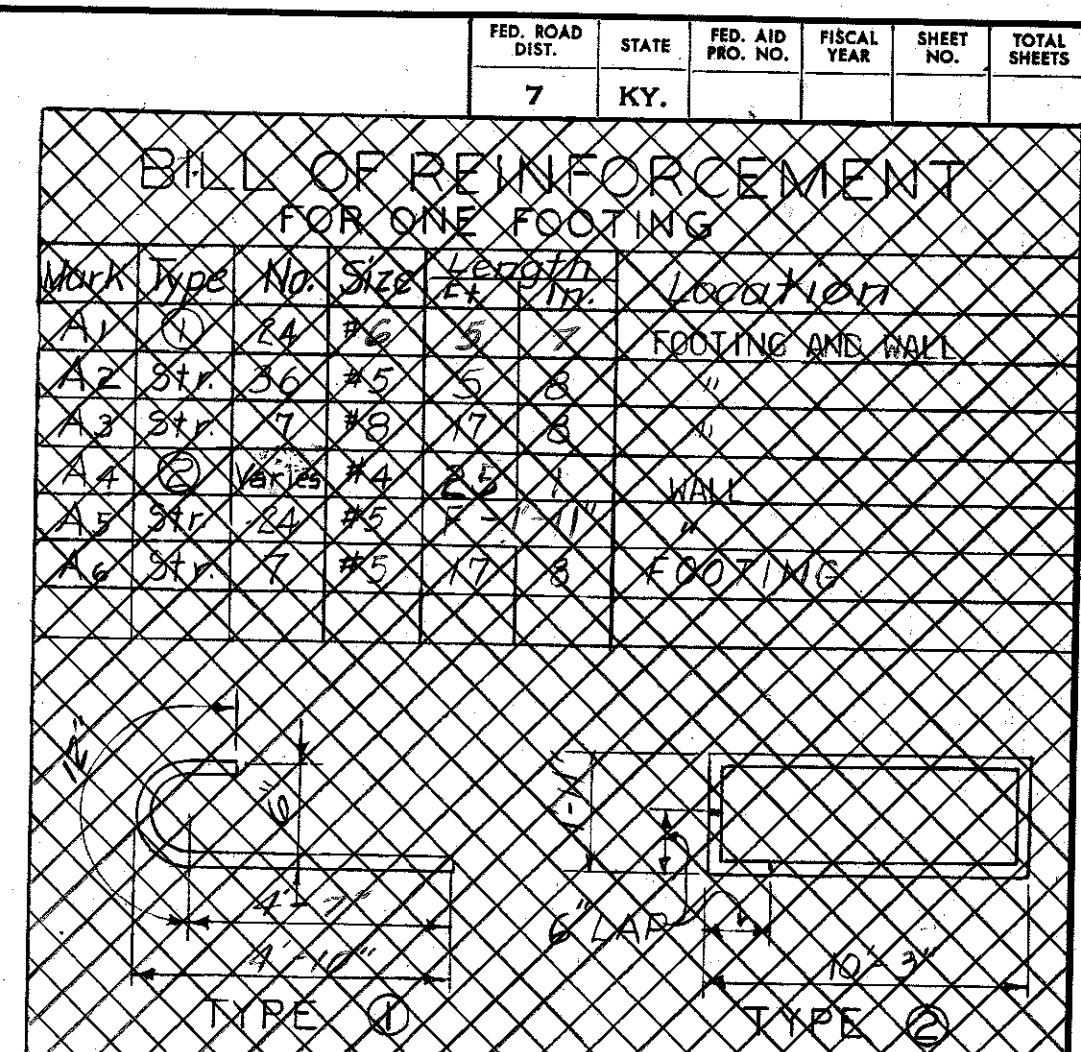
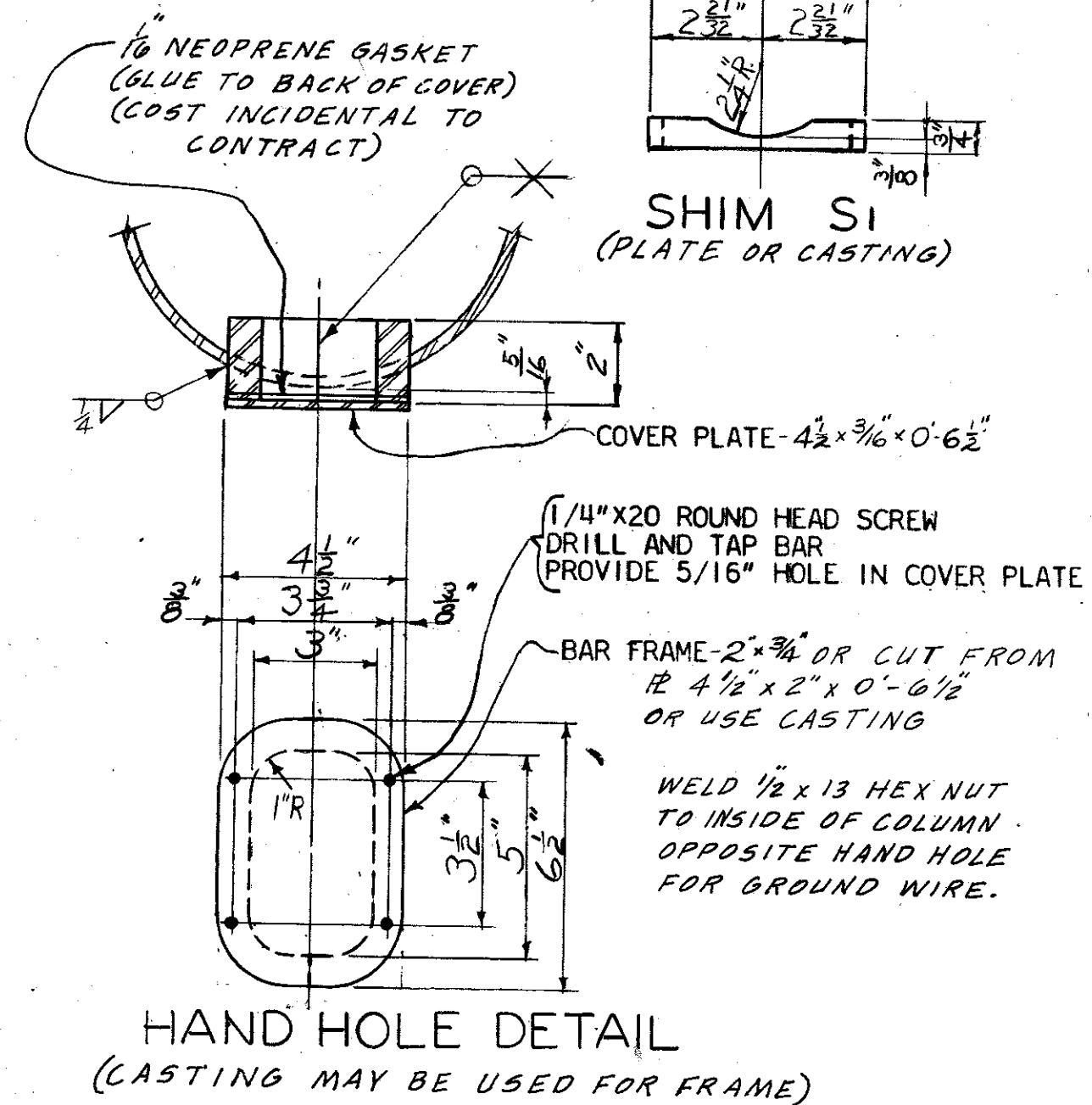
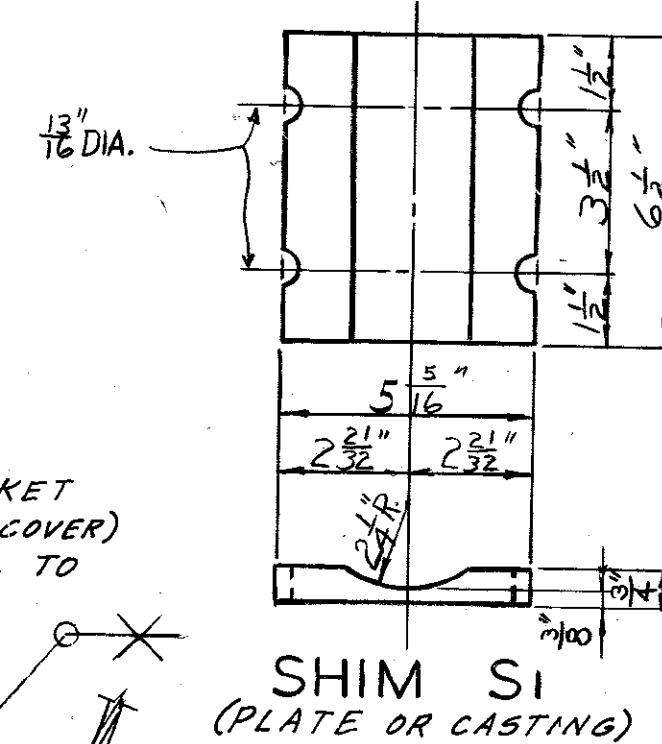
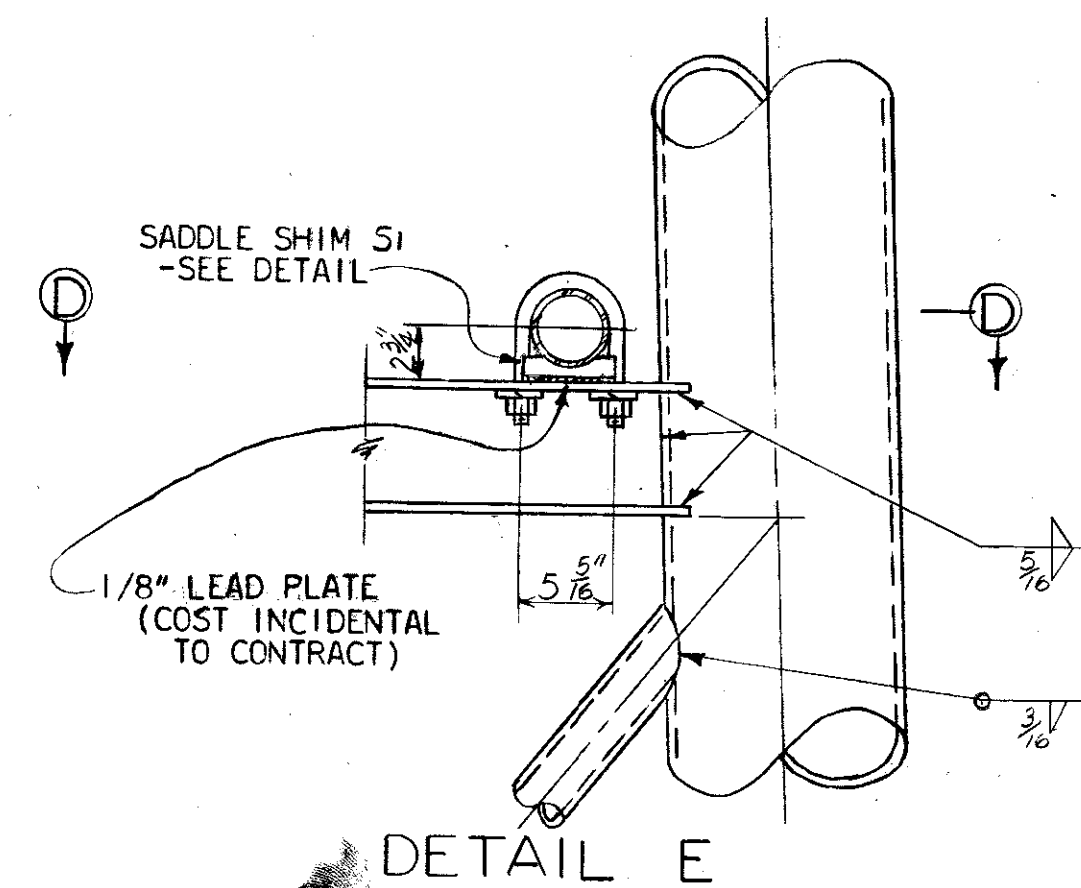
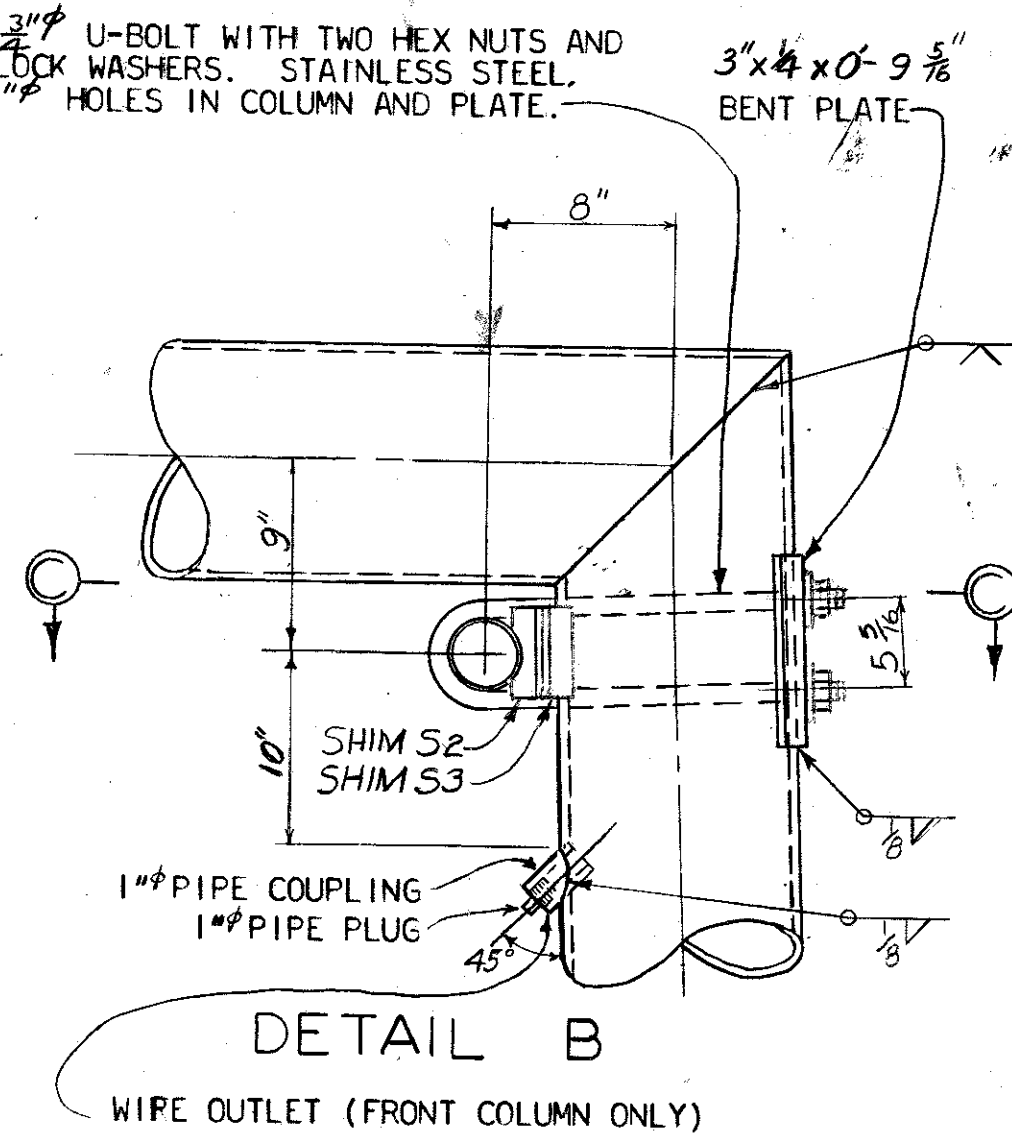
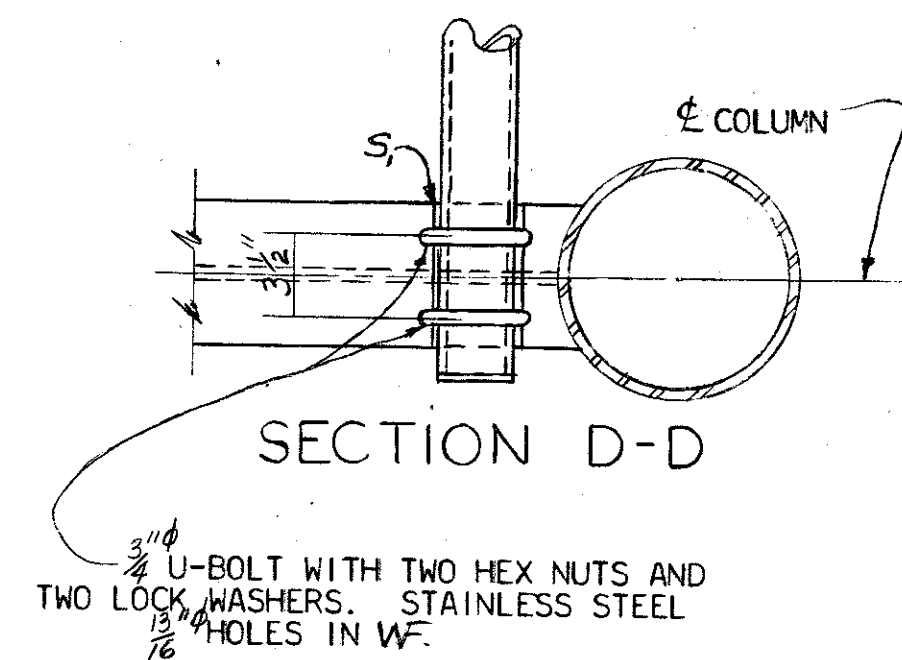
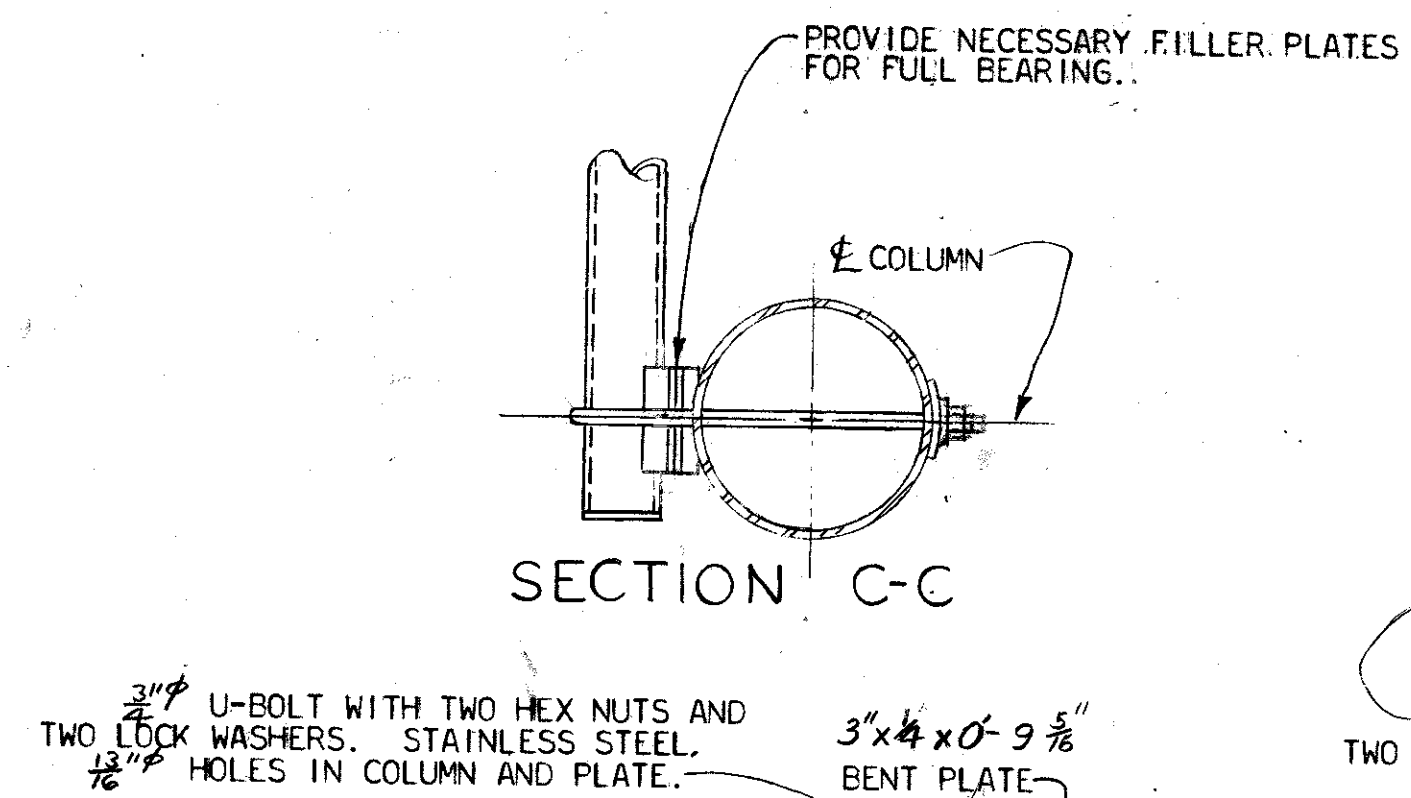
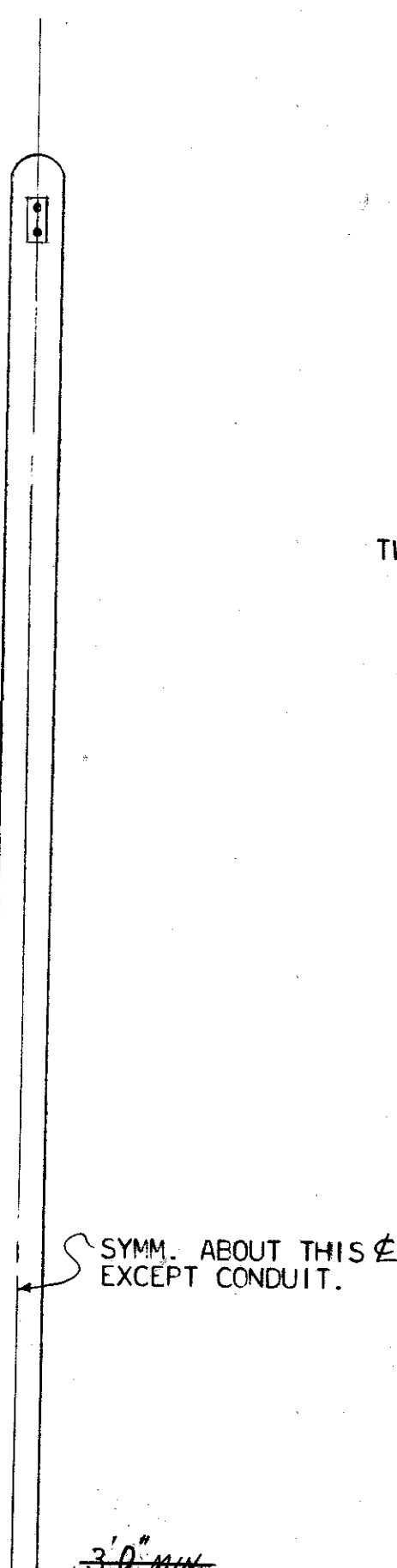
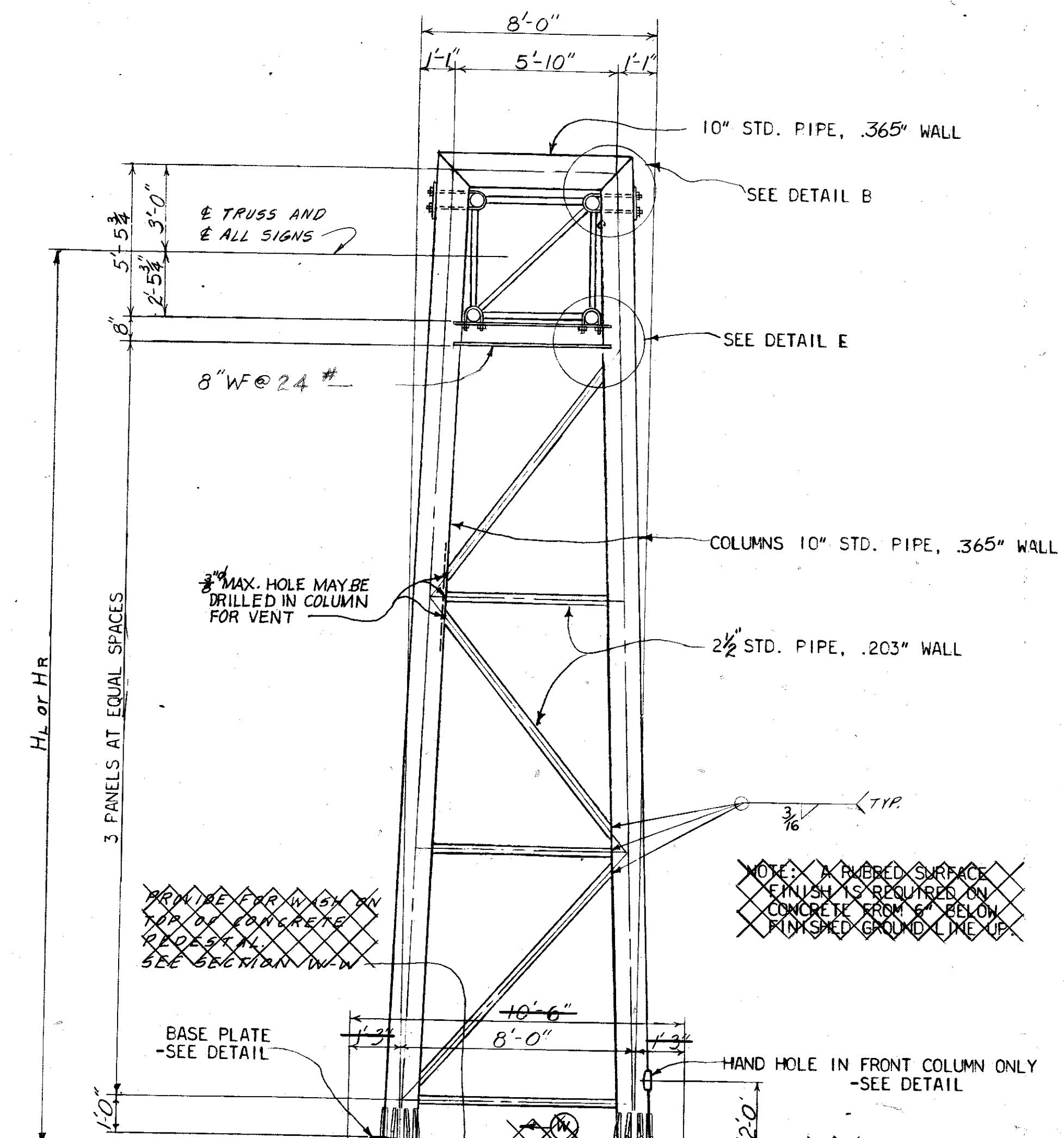
COMMONWEALTH OF KENTUCKY
DEPARTMENT OF HIGHWAYS
FRANKFORT

STANDARD OVERHEAD
SIGN SUPPORTS
75' TO 105' SPANS
GALVANIZED STEEL

STATION: _____ PROJECT NO.: _____

APPROVED: _____ DRAWING NO.: 19238 INDEX

DESIGNED BY: M. J. K. CHECKED BY: J. G. I. DATE: 11/16/72
REVISION: 1. DATE: 11/16/72
REVISION: 2. DATE: 11/16/72
REVISION: 3. DATE: 11/16/72
REVISION: 4. DATE: 11/16/72
REVISION: 5. DATE: 11/16/72
REVISION: 6. DATE: 11/16/72
REVISION: 7. DATE: 11/16/72
REVISION: 8. DATE: 11/16/72
REVISION: 9. DATE: 11/16/72
REVISION: 10. DATE: 11/16/72



Sheet 2 of 2
SHEET 40

COMMONWEALTH OF KENTUCKY
DEPARTMENT OF HIGHWAYS
FRANKFORT
STANDARD OVERHEAD
SIGN SUPPORTS
75' TO 105' SPANS
GALVANIZED STEEL

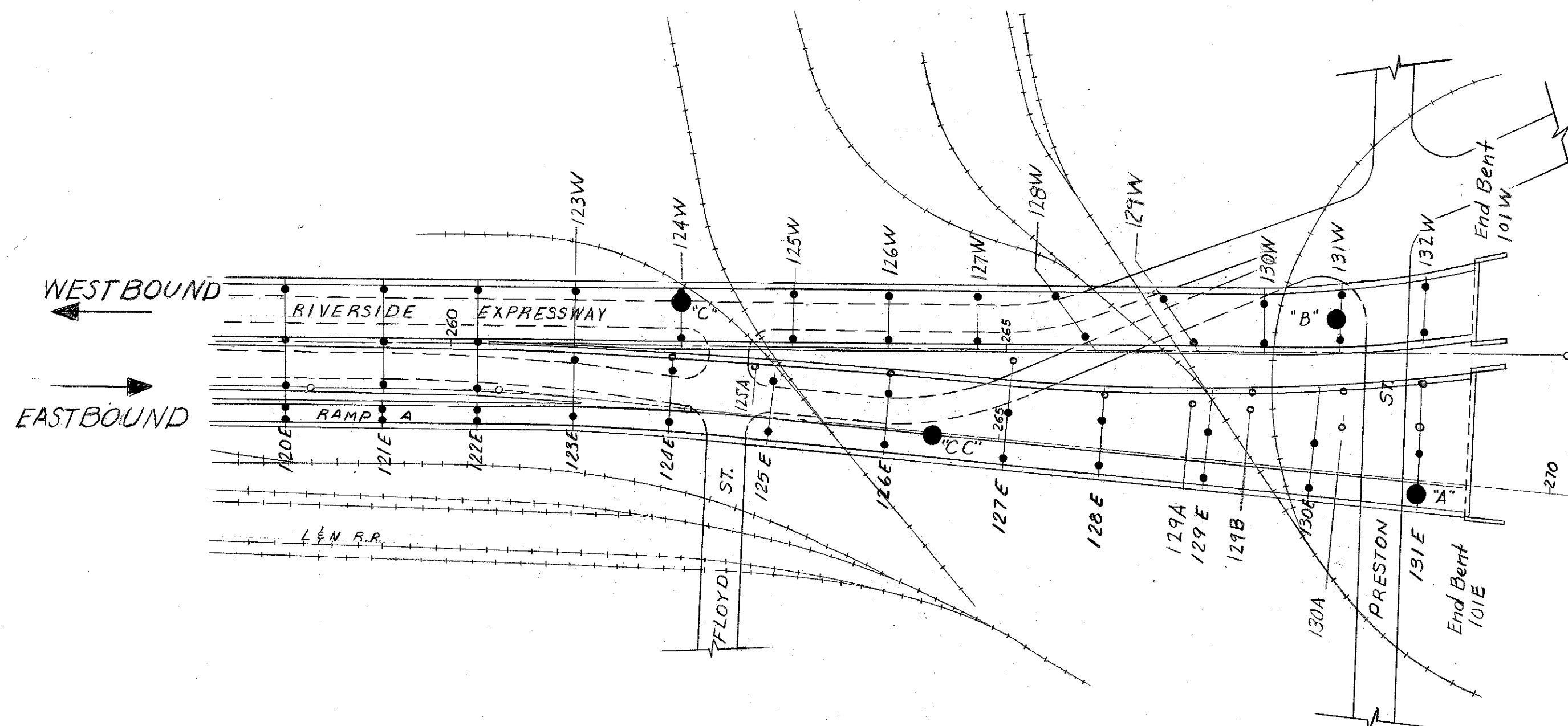
STATION _____ PROJECT NO. _____

APPROVED	<i>[Signature]</i> STATE HIGHWAY ENGINEER	DRAWING 19238 NO.	INDEX
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SIGN SUPPORT

LETTING DATE

PILE DRIVER DATA (For existing piles)
Size & Make of Hammer: Mc Kiernan-Terry D30
Actual Stroke: 6-7 feet
Weight of Ram: 2800 lbs.
Blows Per Minute: 60

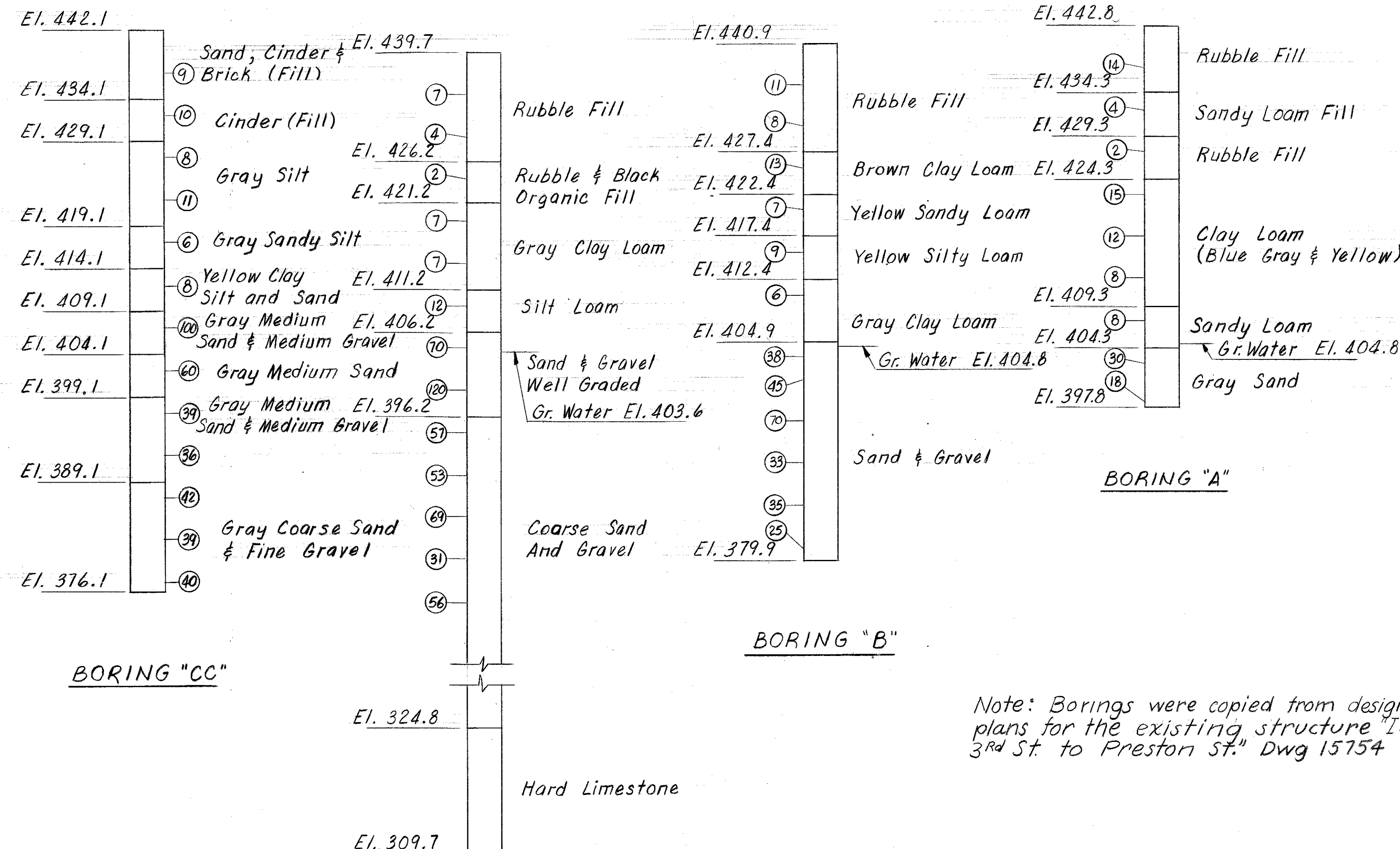


PLAN
Scale: 1"=100'

EXISTING PILE DATA				
FOOTING NO.	① PILE TIP ELEV.(FT)	② BLOWS PER FT.	③ BRG. CAP. ACITY (TON.)	④ DISTANCE DRIVEN(IN.)
124E	405±	234	96 min.	1/4
125E	406±	* 95 ⑦	96 min.	* 1/4
126E	406±	185 ⑧	96 min.	3/4
127E	406±	188 ⑧	96 min.	1/4
128E	404±	180 ⑦	112 min.	1/4
129E	404±	188 ⑧	96 min.	3/8
130E	404±	122 ⑦	96 min.	3/4
131E	403±	188 ⑦	108 min.	3/8
Bent 101E	402±	160 ⑧	96 min.	3/4
124W	405±	113 ⑦	86 min.	1
125W	406±	240 ⑧	112 min.	1/2
126W	405±	* 140 ⑧	96 min.	* 7/8
127W	405±	170 ⑧	102 min.	3/4
128W	405±	180 ⑧	96 min.	5/8
129W	404±	180 ⑦	86 min.	5/8
130W	404±	200 ⑧	96 min.	3/8
131W	404±	190 ⑦	102 min.	1/2
132W	403±	* 240 ⑧	86 min.	* 1/8
Bent 101W	400±	100 ⑦	96 min.	1/8

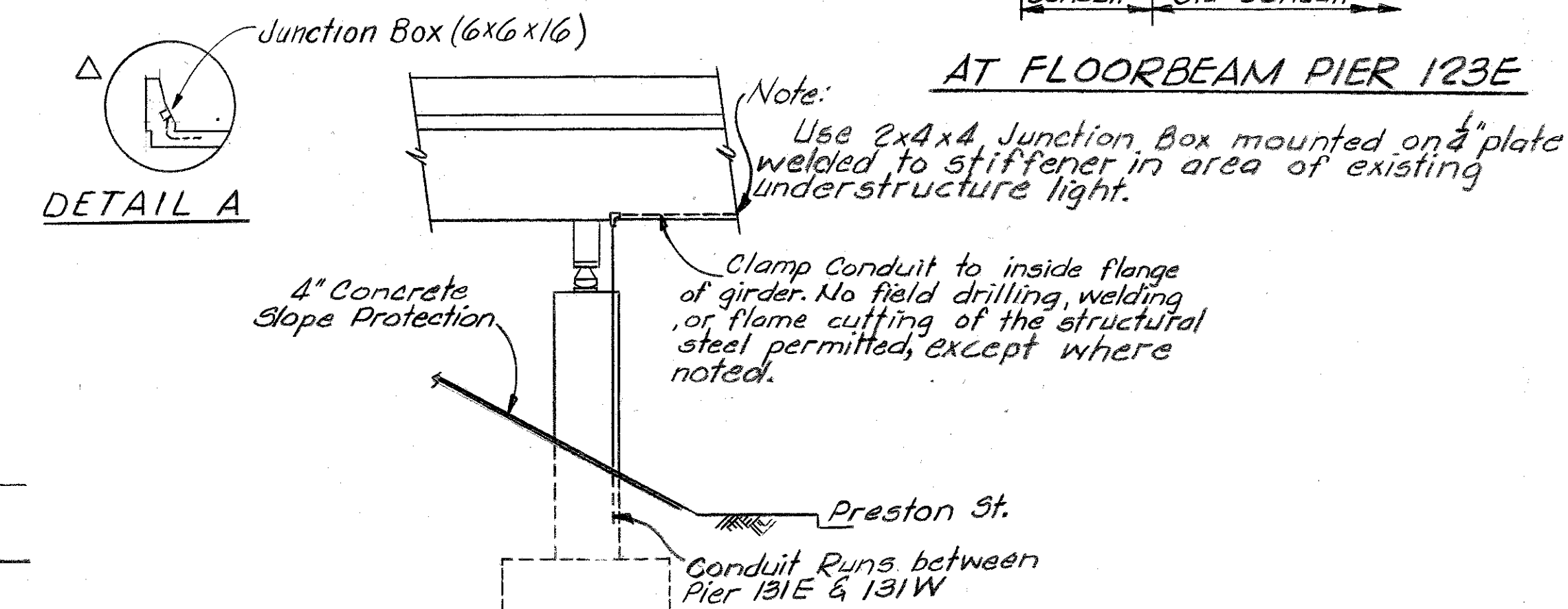
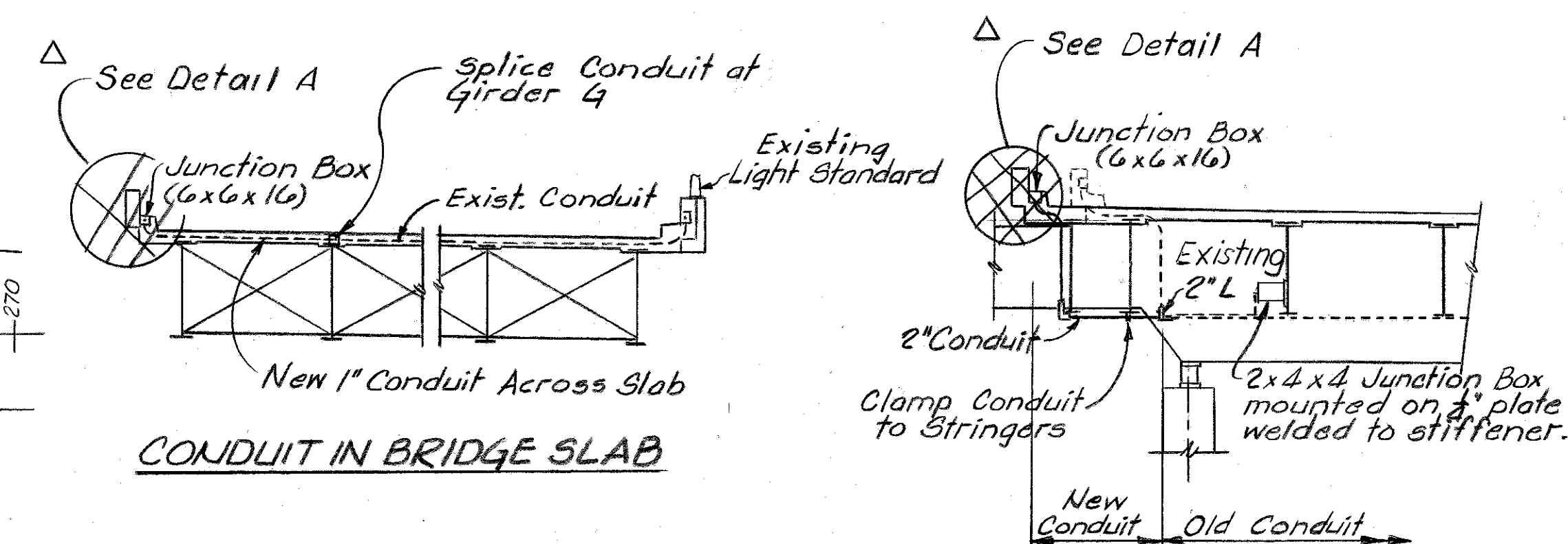
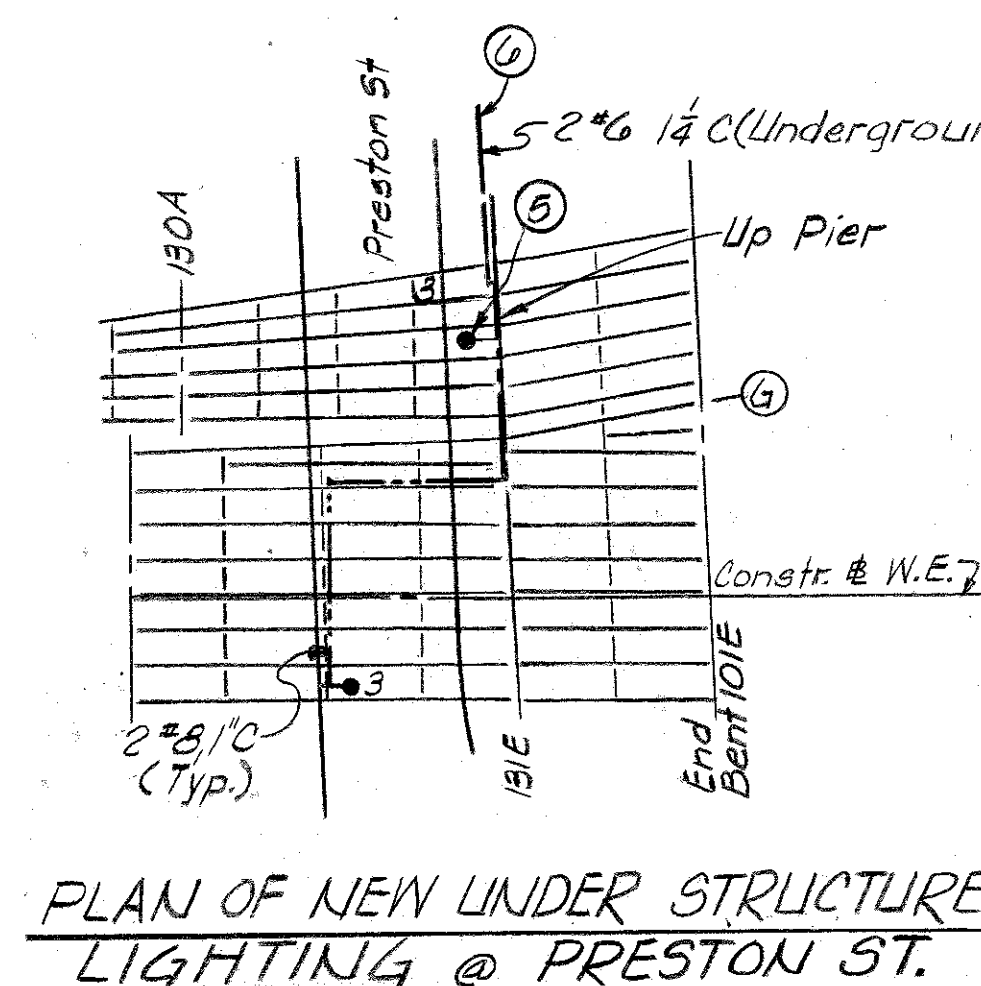
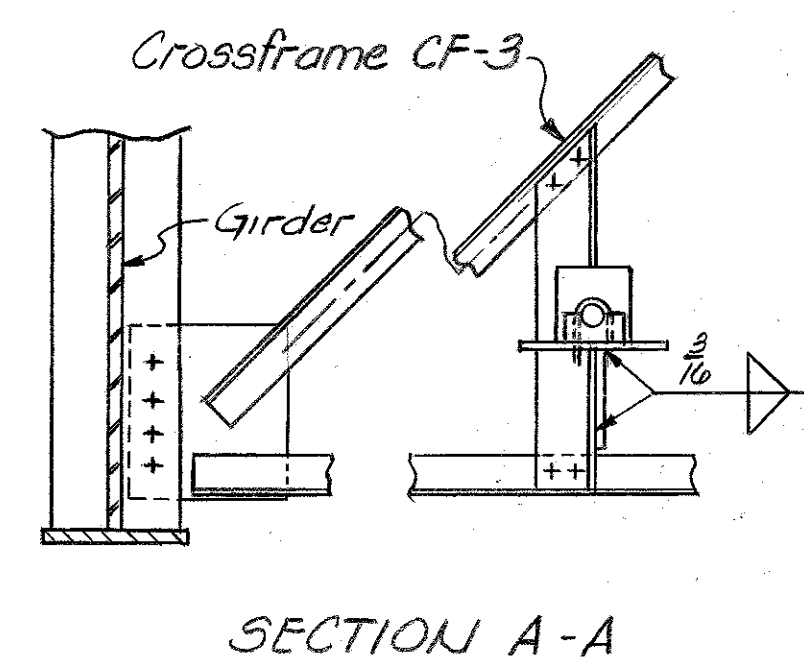
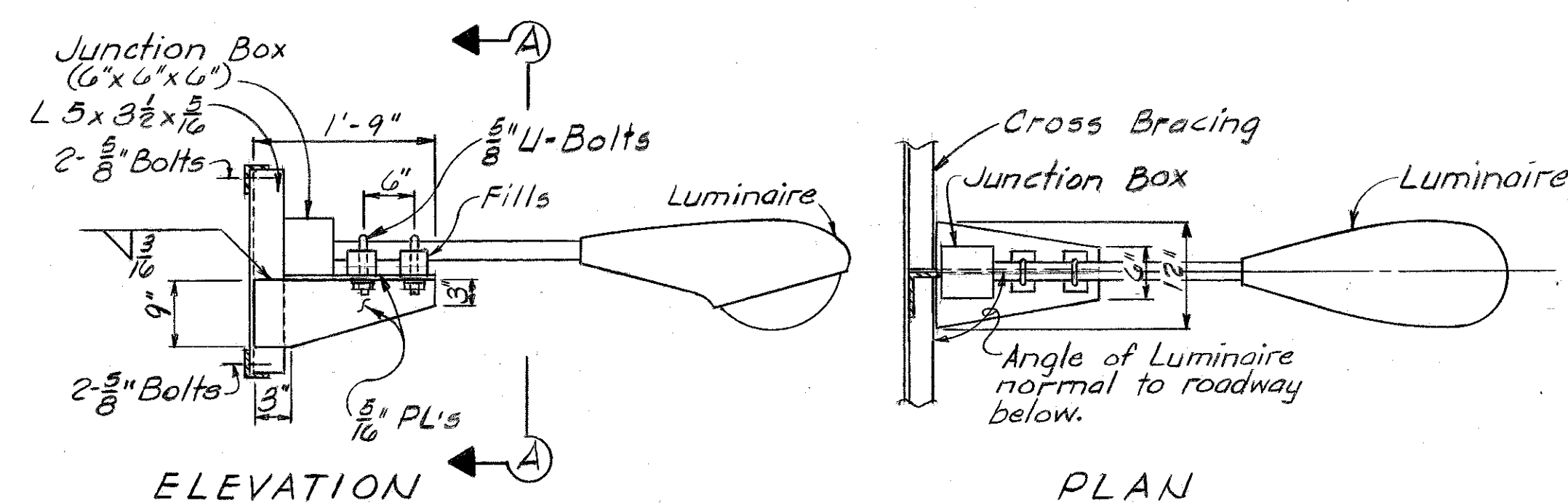
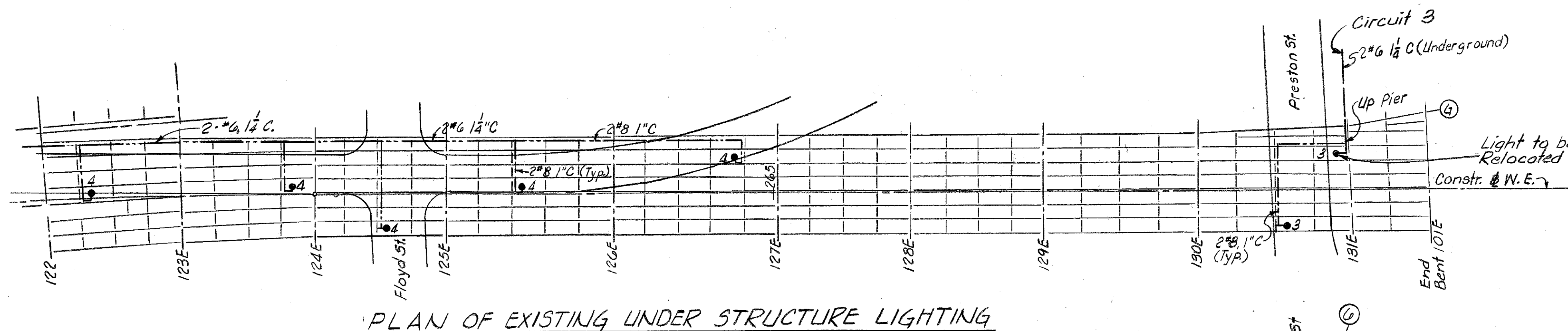
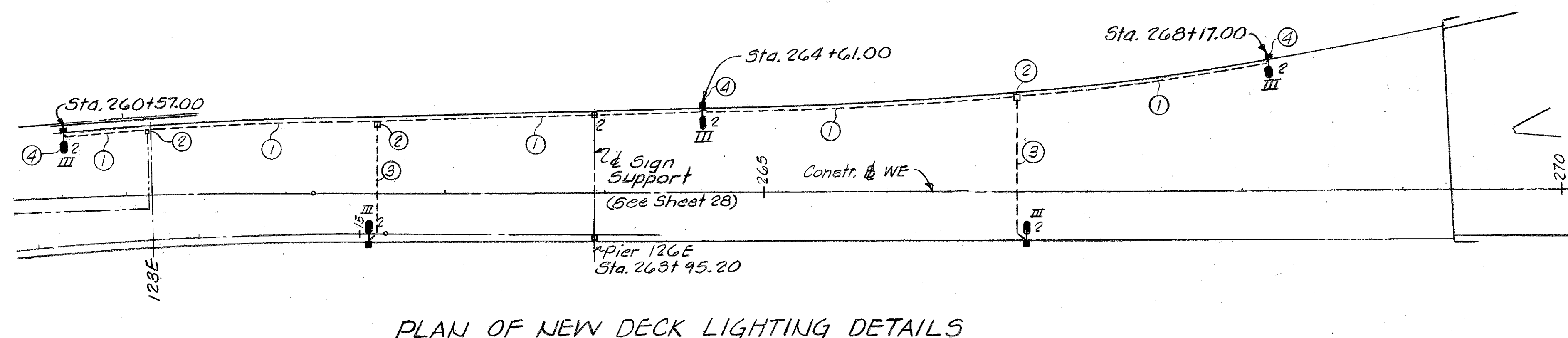
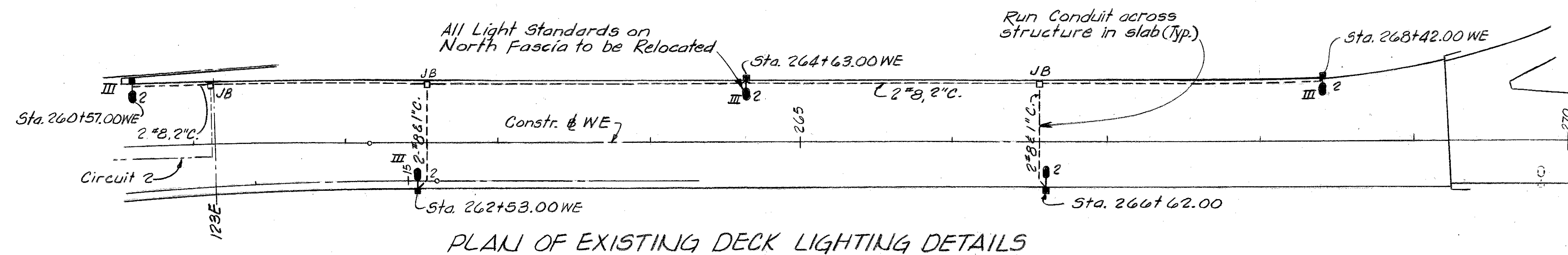
- ① Average elevation of pile tip in footing.
- ② Number of blows on test pile for last ft. driven or the number of blows for the last ② inches.
- ③ This value is the smallest in the specified footing. (Determined by formula)
- ④ Distance driven in the last 10 blows of driving the pile.

Note: The above data is for the south footing on the westbound roadway and the north footing on the eastbound. An * indicates the data is obtained from the adjacent footing of the same pier.



Note: Borings were copied from design plans for the existing structure 164-3rd St. to Preston St. Dwg 15754

LOG OF SOUNDINGS



ELEVATION @ PIER 13/E
Looking from North to South

TYPICAL CONDUIT FOR DECK & UNDERSTRUCTURE LIGHTING

Notes

- ① New 2" Conduit with 2*8 wires.
- ② New 6"Hx6"Dx16"L Junction Box in ~~Curb~~ Barrier Δ
- ③ New Conduit is to be spliced to existing conduit at existing girder G and extended to new Junction Box in Curb. The existing wire in the deck slab is to be replaced with 2 new #8 wires from Junction Box to Light Standard. 53 Δ
- ④ Relocated Light Standard. See Sheet 28 for anchor bolt locations and concrete details for base anchorage.
- ⑤ Relocated under structure light.
- ⑥ 1/4" ϕ underground conduit between End Bent 101W and 101E to be replaced where destroyed by new footings. 2 new #8 wires to be installed between permitted splice points.

LEGEND

- 2  Indicates Steel Light Standard with 4ft bracket mounted on bridge plinth (30'± mounting height) and 400w, 20,000 Lumen
- III Mercury Vapor Luminaire with built-in ballast.
- III indicates I.E.S. Type III distribution
- Number(2) indicates circuit connected to luminaire.
- 3 250w, 11,600 Lumen Fused Mercury Vapor Luminaire for Street & Wharf Lighting under structure.
- Number(3) indicates circuit connected to luminaire.
-  Indicates Illuminated Overhead Sign
- Number(2) indicates circuit for connection to sign transformer.
- Indicates Conduit Run concealed in structure concrete.
- Indicates Conduit Run buried below ground.
- Indicates Conduit Run exposed on structure.

SHEET 42

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

FRANKFORT
COUNTY OF

JEFFERSON

LOUISVILLE-LEXINGTON

ROAD

STATION 267 + 00

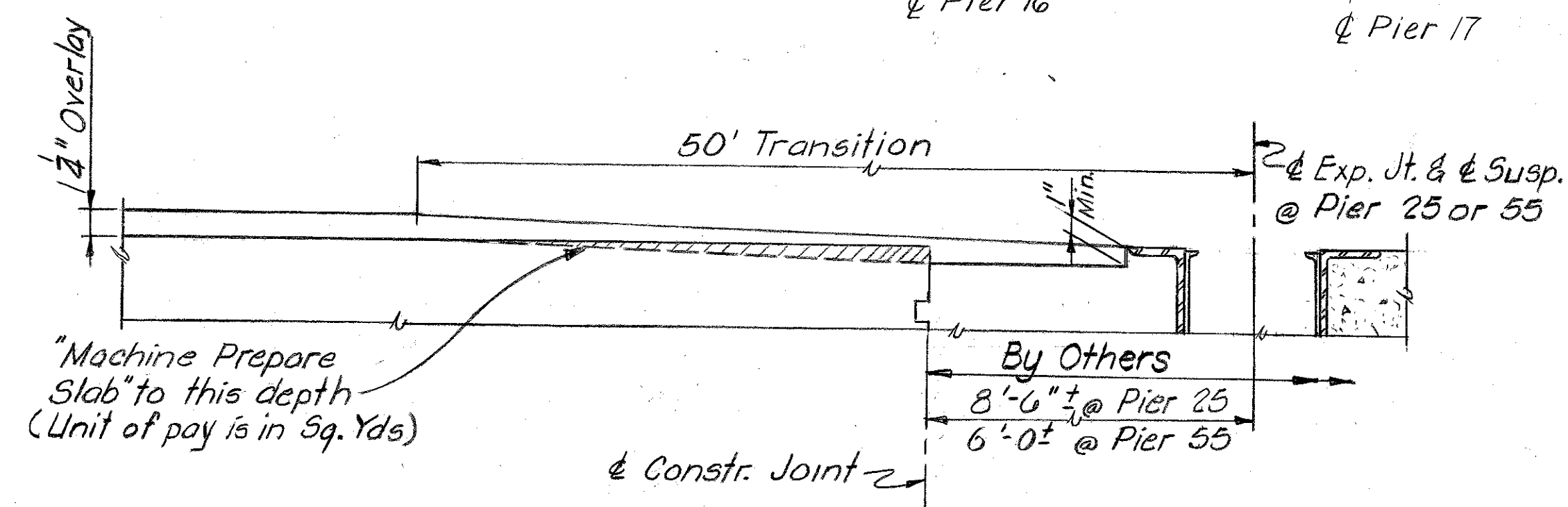
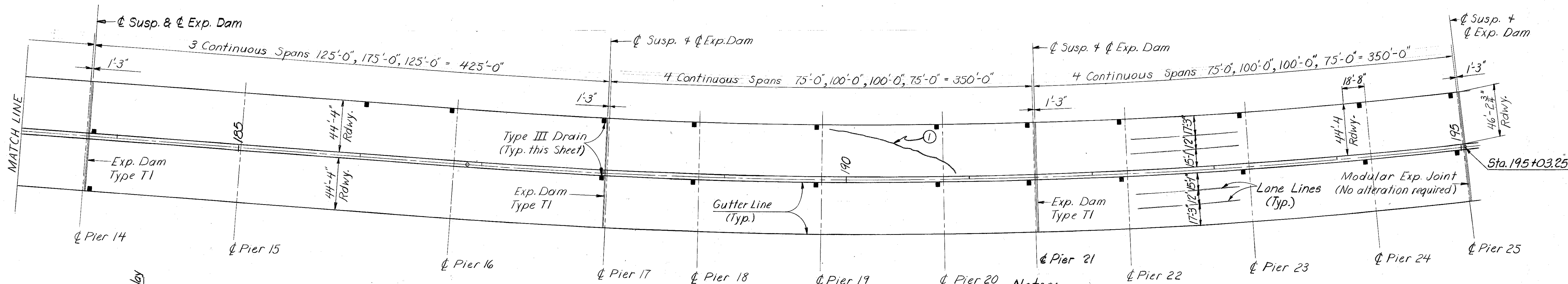
P. E. PROJECT NO. I64-2(92)	
MAINTENANCE PROJECT NO.	DRY

DRAWING	1923
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BRIDGE LIGHTING

DESIGNED BY <i>RHC</i>	CHECKED BY <i>RLC</i>	DATE <i>4-75</i>	<i>Δ Indicates Revision For G.M. Barrier</i>	DATE <i>7/75</i>
DESIGNED BY <i>RHAC</i>	CHECKED BY <i>RLC</i>	DATE <i>4-75</i>		DATE
DESIGNED BY	CHECKED BY	DATE		DATE

DESIGNED BY _____	CHECKED BY _____	DATE _____	REVISED _____	DATE _____
OBTAINED BY <u>R.H.M.C.</u>	CHECKED BY <u>BHC & Block</u>	DATE <u>4-75</u>	REVISED _____	DATE _____
_____	CHECKED BY _____	DATE _____	REVISED _____	DATE _____
_____	CHECKED BY <u>KRV</u>	_____	_____	_____



Notes:

For Drain Alterations see Sht. 49

① Deck Patches-All patches with visible cracks or no bond between the concrete Deck and the patch shall be removed in accordance with the "Special Notes for Deck Overlay". (The basis of payment is "Machine Prepare Slab", measured in Sq.Yds).

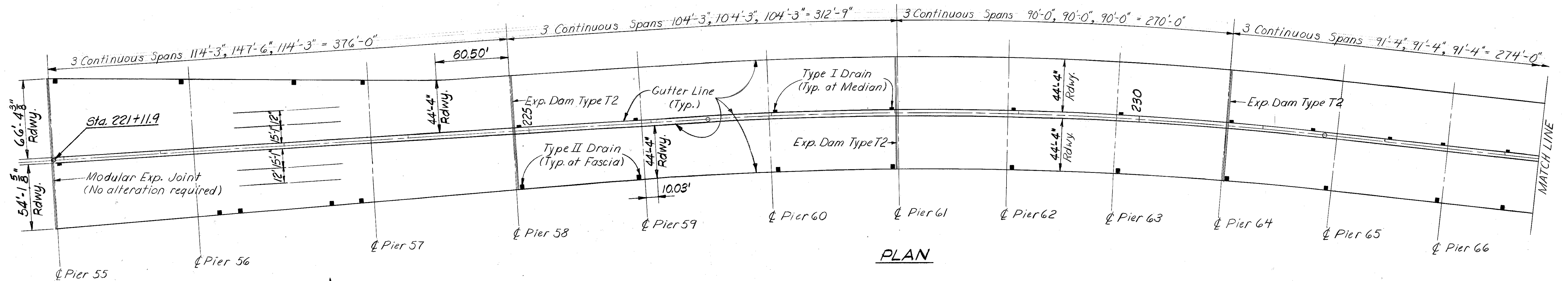
DECK OVERLAY - 17TH - 13TH STREETS

DRAWING NO	
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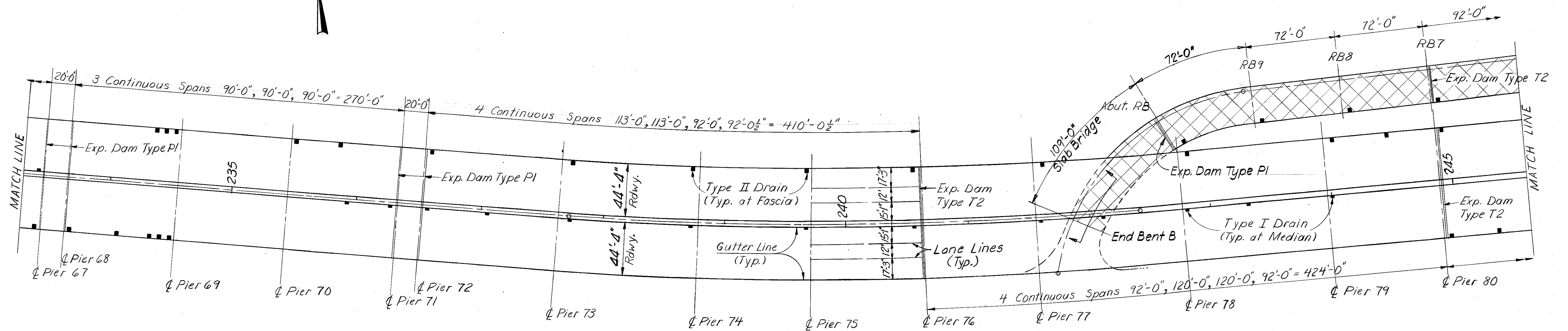
DRAWING NO.
19238

UPDATE DATE
LETTING DATE

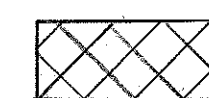
DESIGNED BY: R.H.M.C. CHECKED BY: R.H.M.C. DATE: 4-75
TRACED BY: K.M.U. CHECKED BY: K.M.U. DATE: 4-75



PLAN



PLAN



Not a part of this contract.
No surface treatment.

NOTE:

For Drain Alterations See Sht. 49

For Expansion Dam Alterations See Sht. 48

For Deck Overlay transition @ Pier 55 see Sht. 45

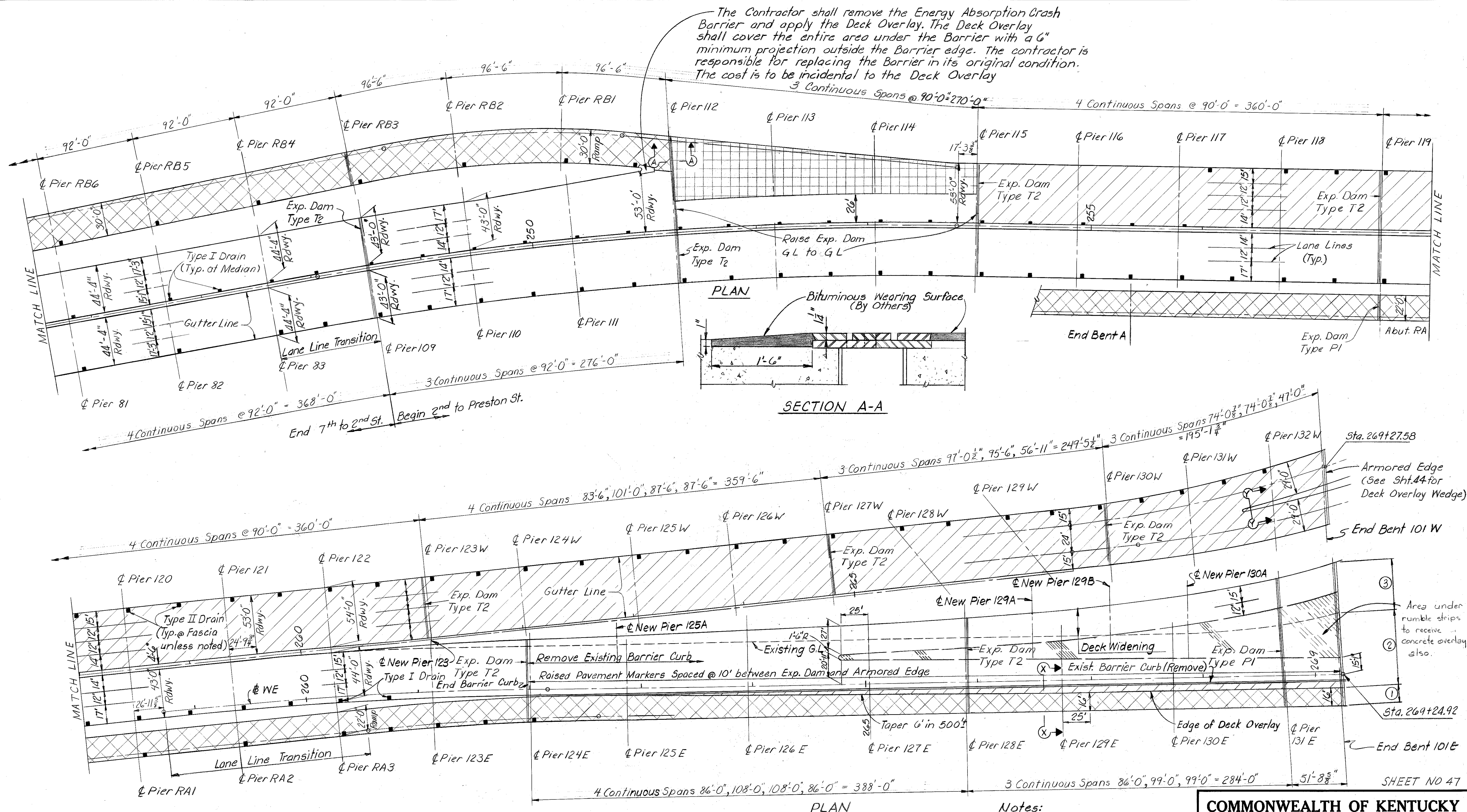
SHEET NO 46

COMMONWEALTH OF KENTUCKY	
BUREAU OF HIGHWAYS	
FRANKFORT	
COUNTY OF	
JEFFERSON	
LOUISVILLE-LEXINGTON	
ROAD	
STATION 267+00	P. E. PROJECT NO. 164-2(92)
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.
DRAWING NO. 19238	

DECK OVERLAY-7TH-2ND ST.

UPDATE DATE
LETTING DATE

DESIGNED BY: *EHM* CHECKED BY: *EHM* DATE: *4-15*
 TRACED BY: *EHM* CHECKED BY: *EHM* DATE: *4-15*



Note:
All areas to receive Deck Overlay are to be sand blasted. The overlay shall be applied within 36 hours of sand blasting.

Not a part of this Contract.
No surface treatment.

Area previously open to traffic to be Machine Prepared to 1/4" depth and to receive 1/2" min. Deck Overlay.

Indicates Rumble Strips- See Special Notes and Sht. 49 for Details.

Area to receive bituminous wearing surface. (by others)

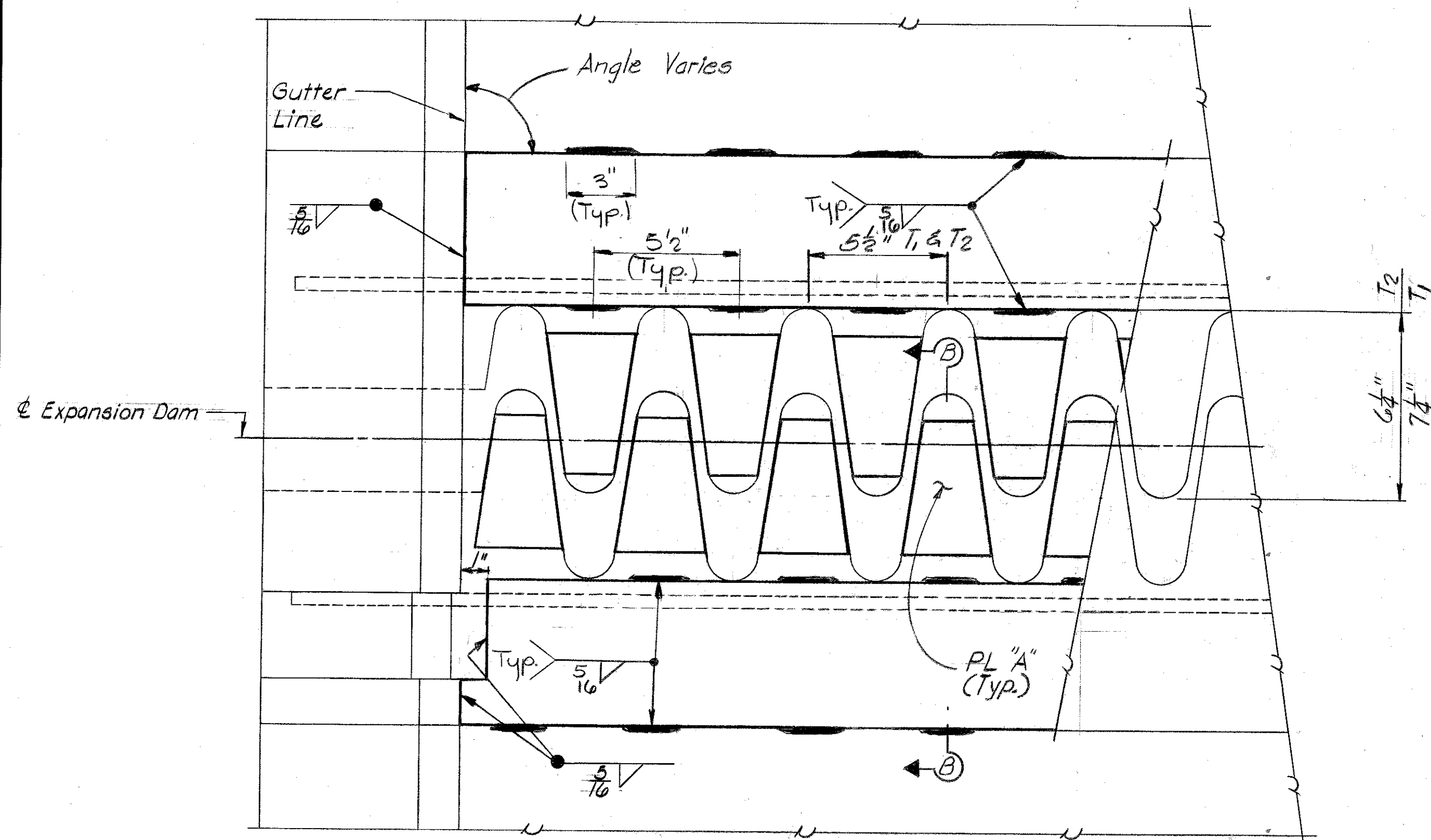
Notes:
For Drain Alterations see Sht. 49
 ① No alterations required on existing Armored Edges.
 ② For Armored Edge Alterations see Sht. 49
 ③ No alterations required on new Armored Edges.
 For Sections X-X & Y-Y see Sht. 48

DECK OVERLAY 2ND-PRESTON

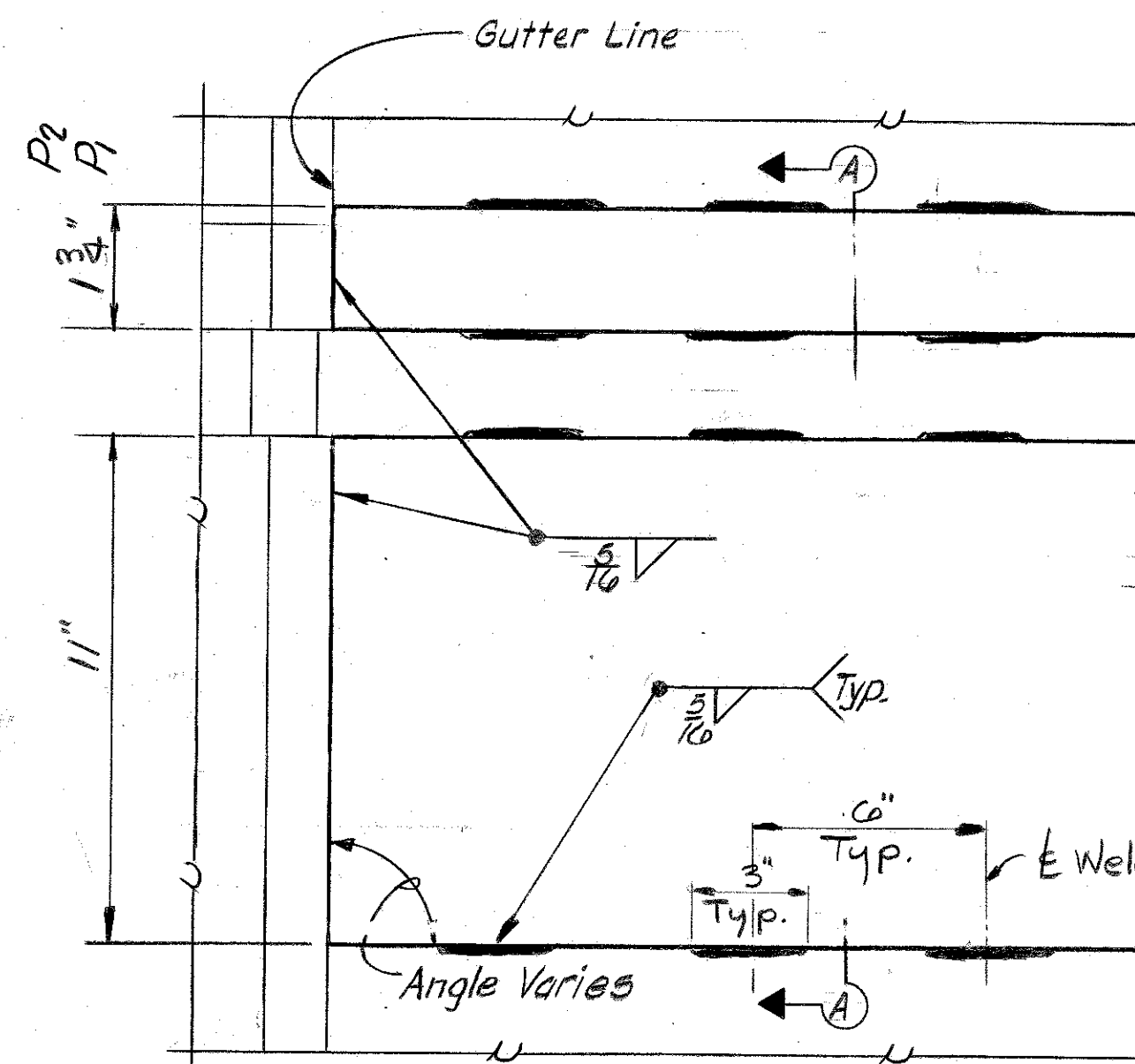
COMMONWEALTH OF KENTUCKY
 BUREAU OF HIGHWAYS
 FRANKFORT
 COUNTY OF
JEFFERSON
 LOUISVILLE-LEXINGTON
 ROAD
 STATION 267+00
 P. E. PROJECT NO. 164-2(92)
 CONSTRUCTION PROJECT NO. MAINTENANCE PROJECT NO. DRAWING NO. 19238

SHEET NO 47

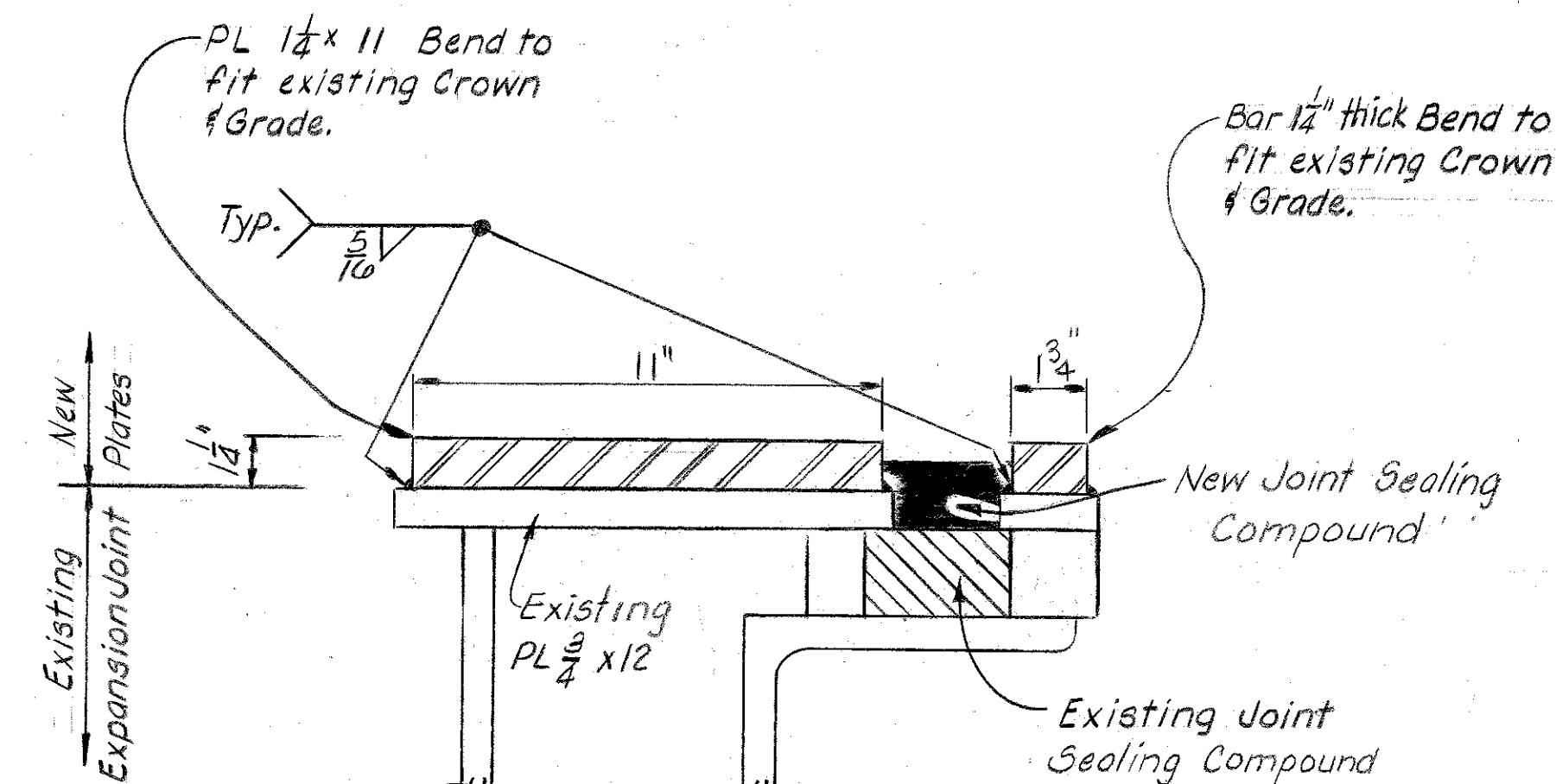
UPDATE DATE
LETTING DATE



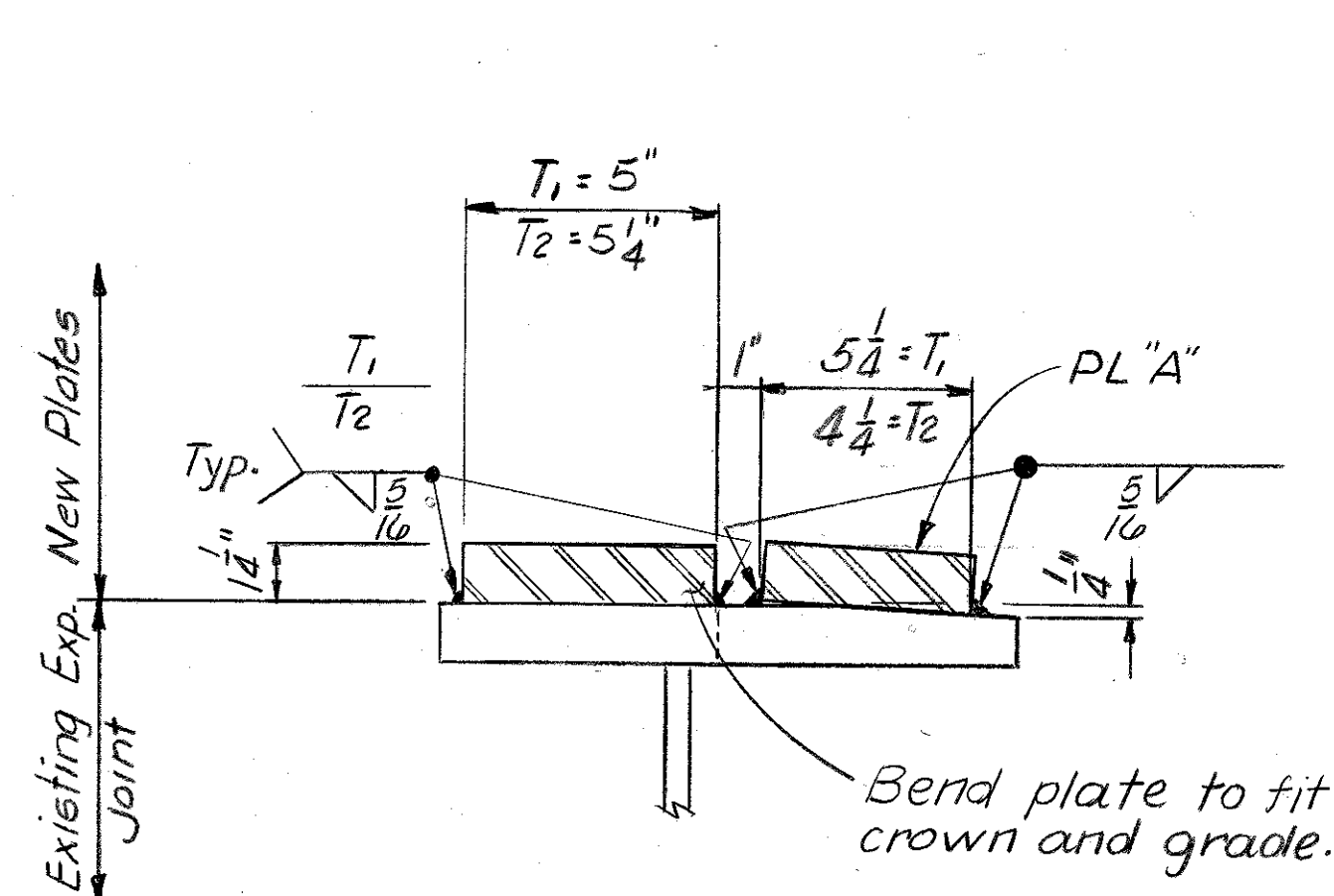
PLAN
EXPANSION DAM TYPES T1 & T2



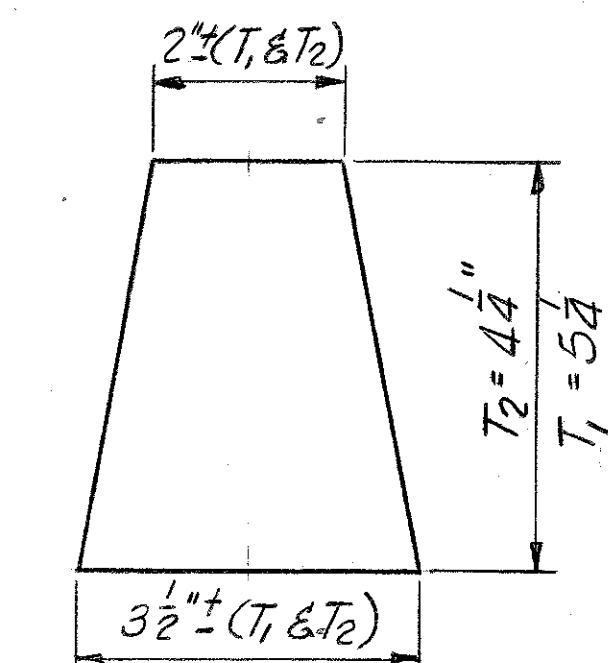
PLAN
EXPANSION DAM TYPE P1 & P2



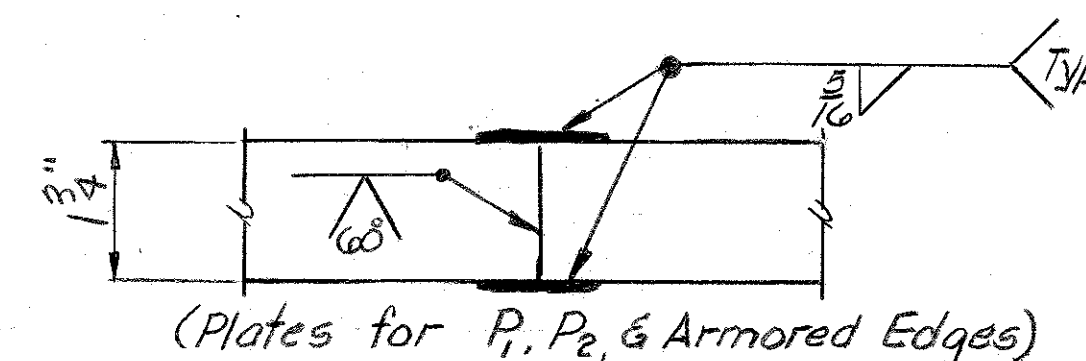
SECTION A-A



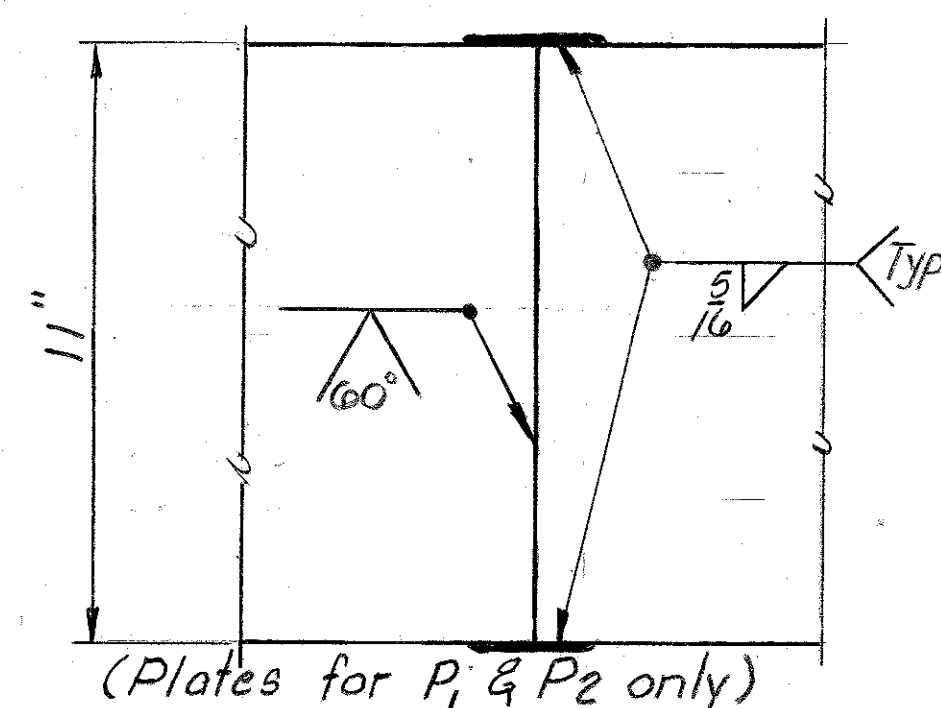
SECTION B-B
(Plates opposite side similar)



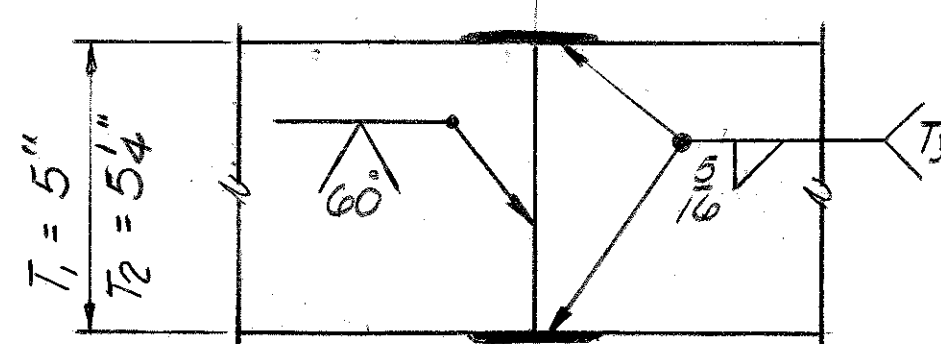
DETAIL OF PLATE "A"



(Plates for P1, P2, & Armored Edges)

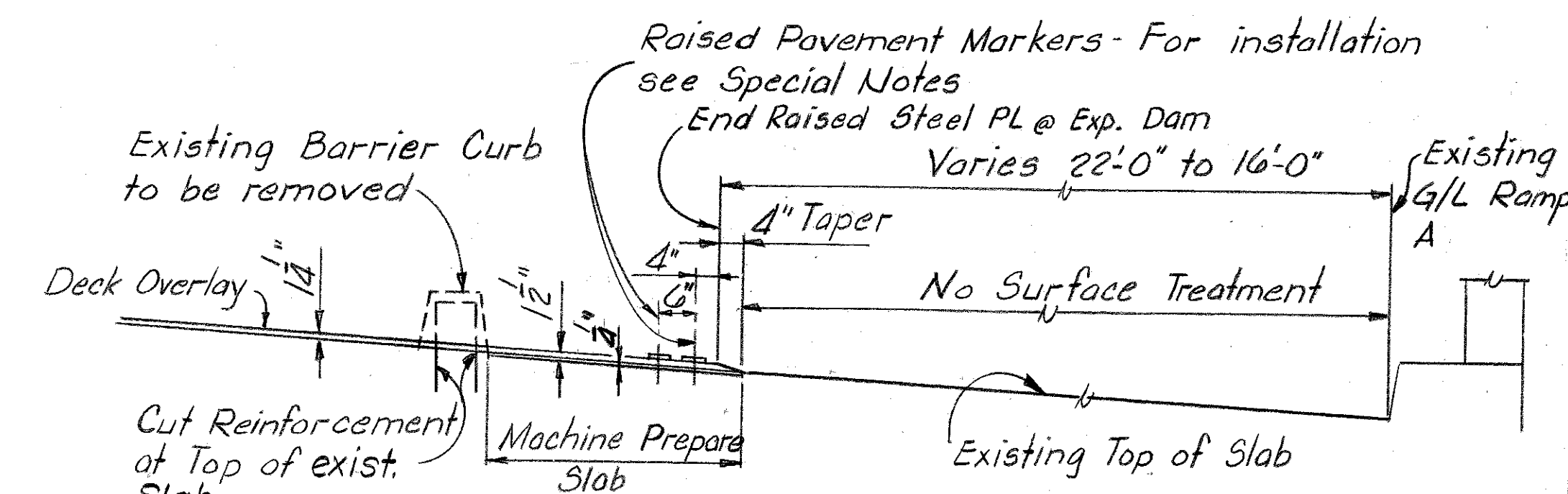


(Plates for P1 & P2 only)

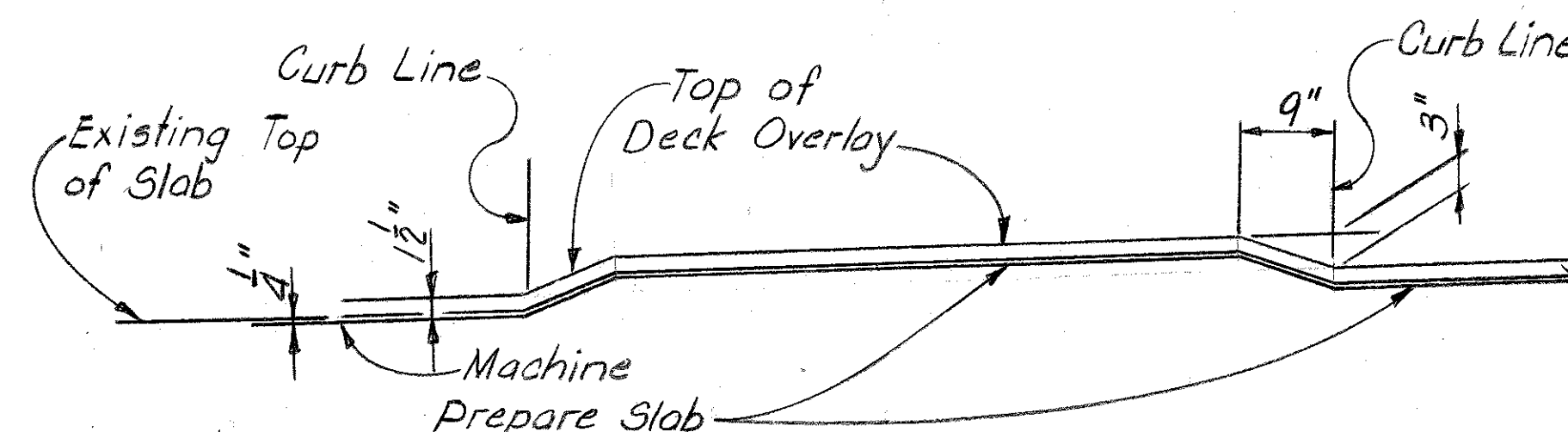


(Plate for T1 & T2 only)

OPTIONAL PLATE SPLICE DETAILS



SECTION X-X



SECTION Y-Y

Note:

All Expansion Dam and Armored Edge Steel shall be A242 unless noted.

The expansion dam dimensions are taken from the bridge design drawings. The Contractor shall obtain the plate length and verify the plate width and tooth dimensions by field measurement.

DESIGNED BY: RHMC
CHECKED BY: BHC, Black
DATE: 4-75
TRACED BY: BHC, Black
DATE: 4-75

DECK OVERLAY - DETAILS

SHEET NO 48

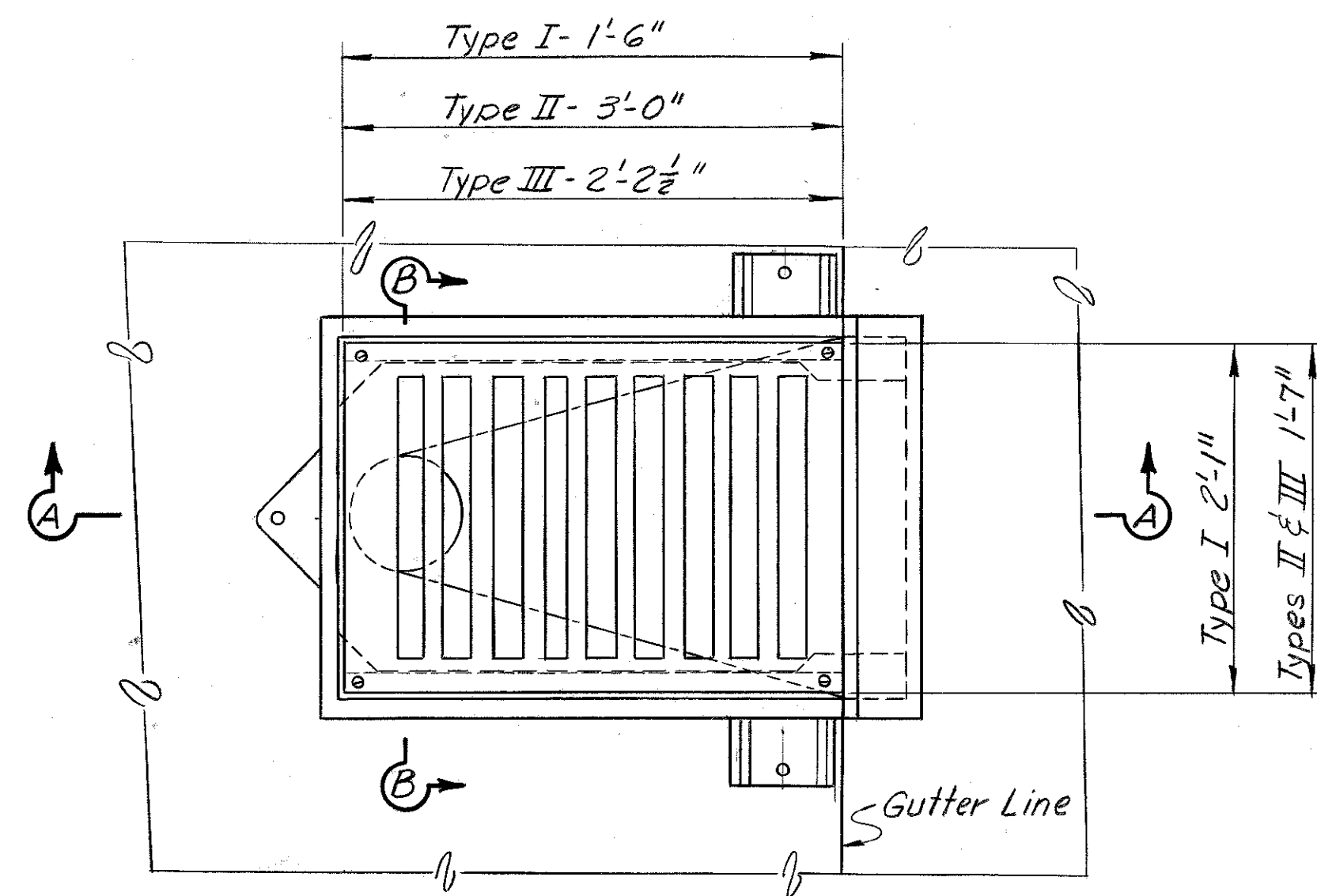
COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE - LEXINGTON
ROAD

STATION 267+00
CONSTRUCTION PROJECT NO.

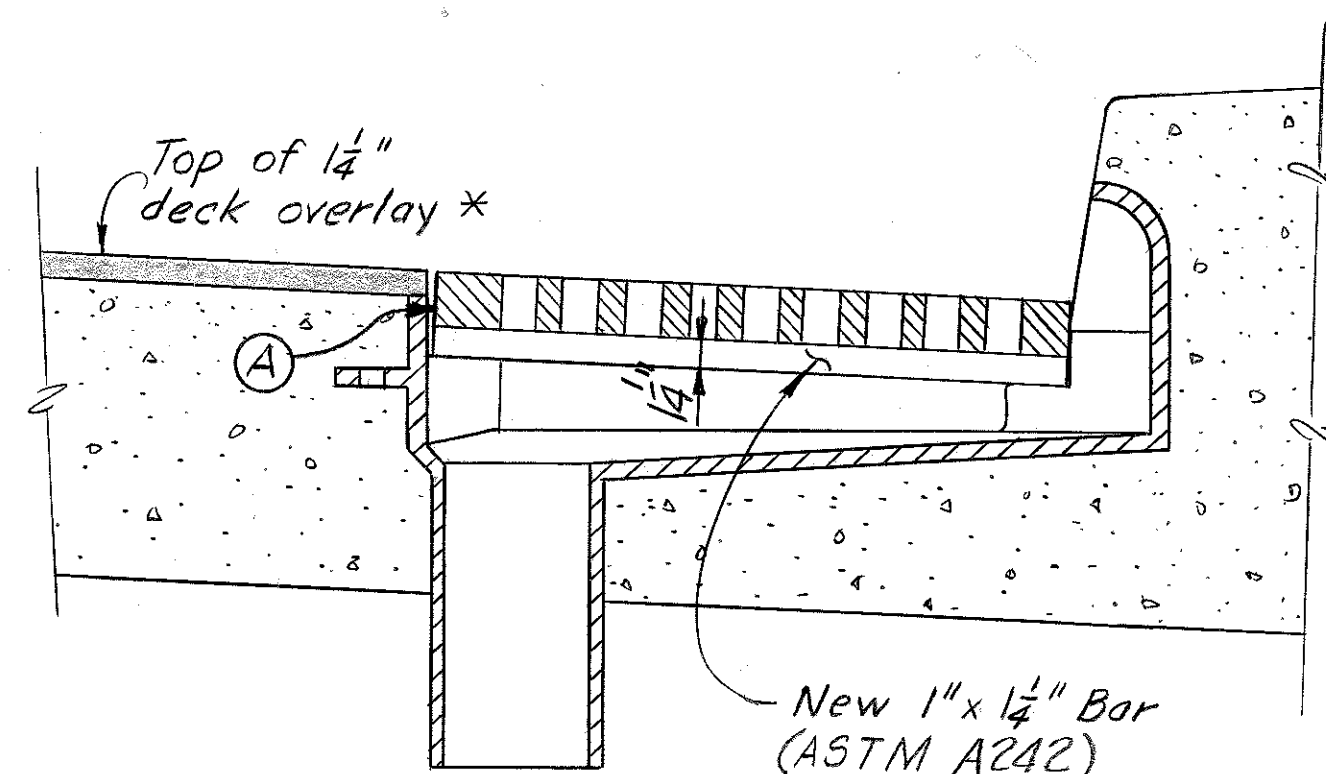
P. E. PROJECT NO. 164-2(92)
MAINTENANCE PROJECT NO.

DRAWING NO.
19238

UPDATE DATE
LETTING DATE

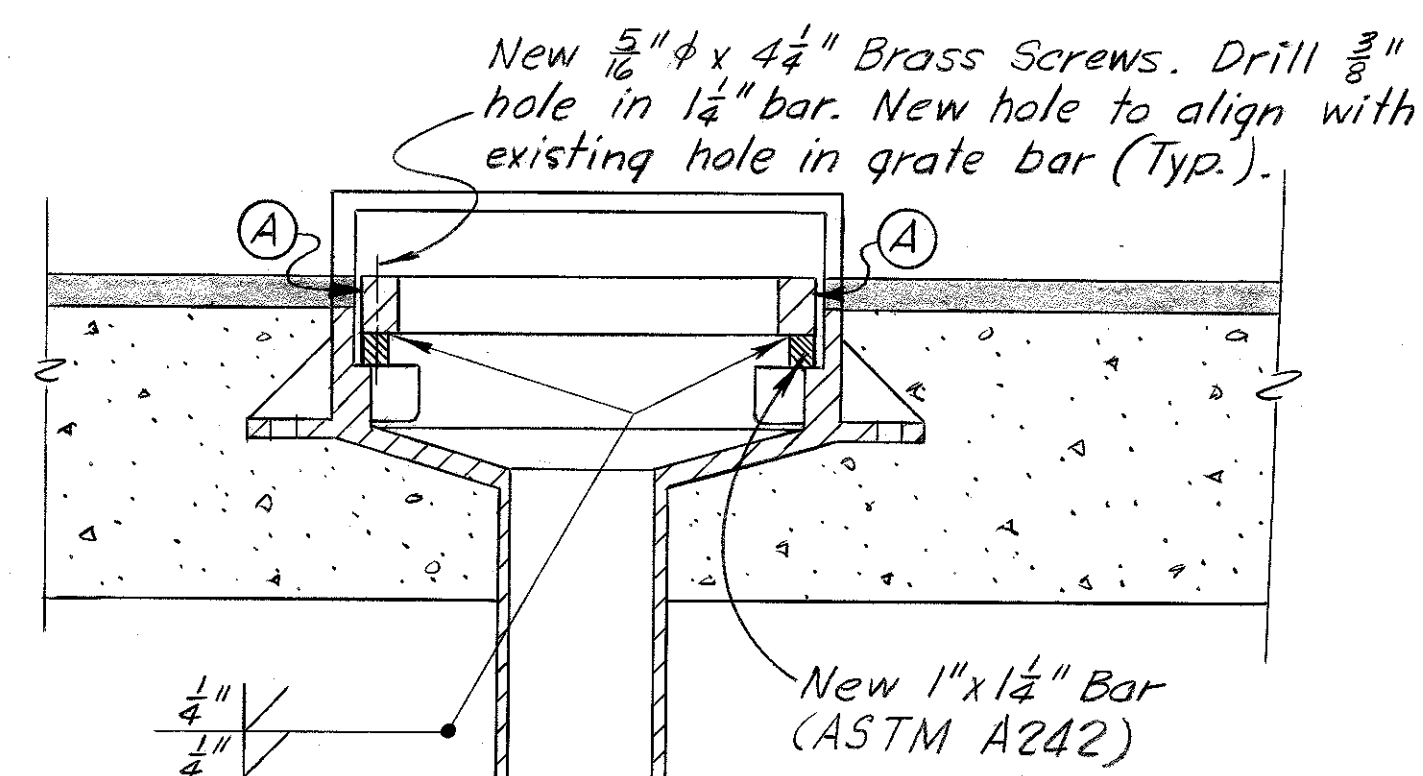


PLAN
DECK DRAIN MODIFICATIONS



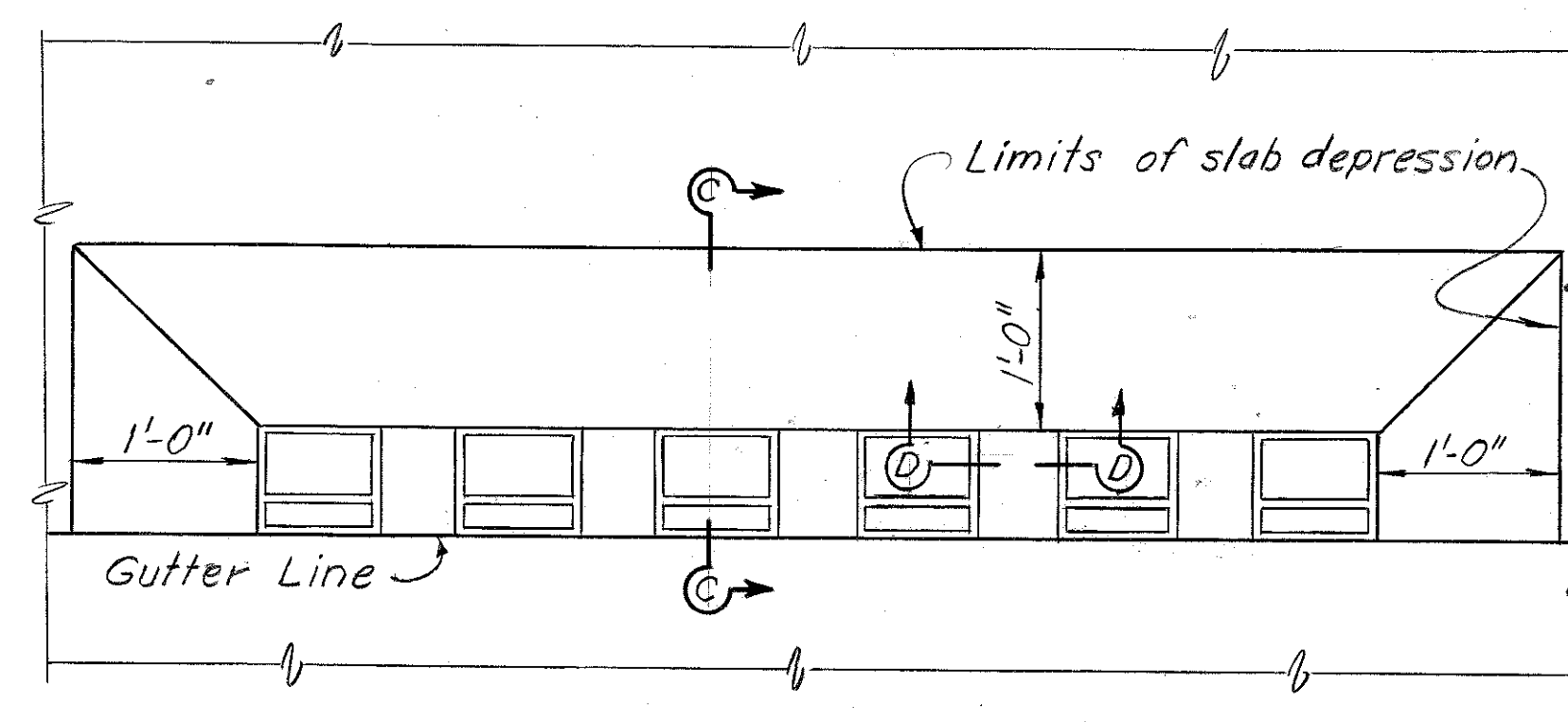
SECTION A-A

*Care shall be taken to maintain thickness of overlay adjacent to drains so as to reproduce depression in top of existing slab.

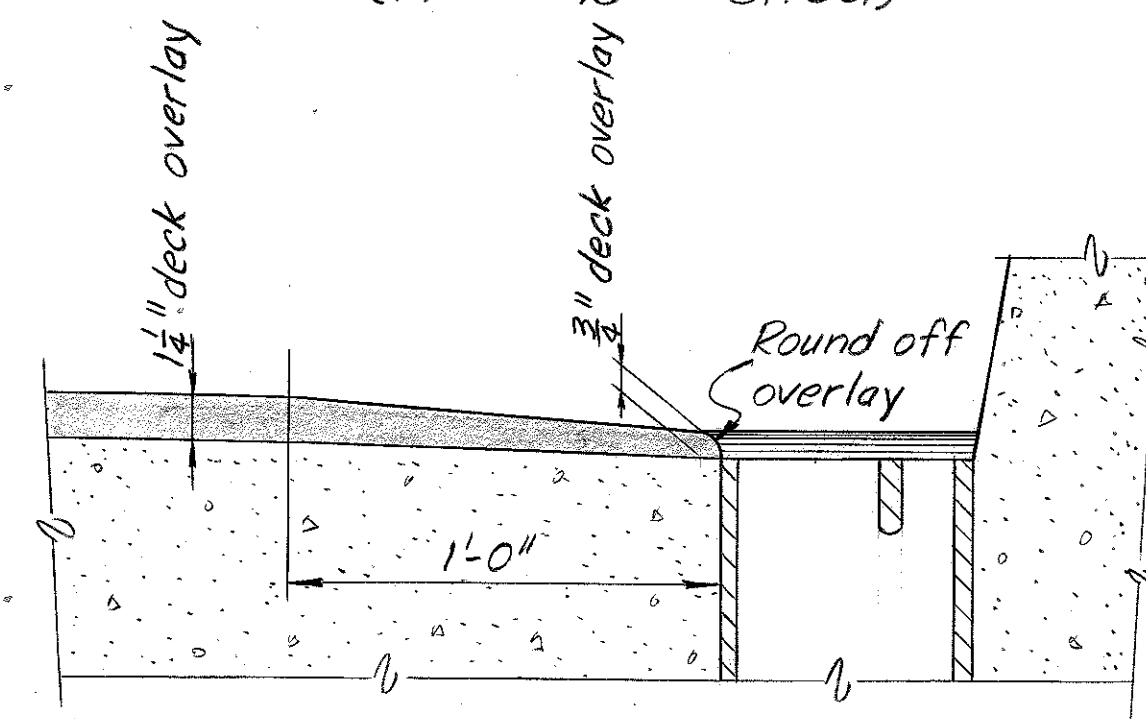


SECTION B-B

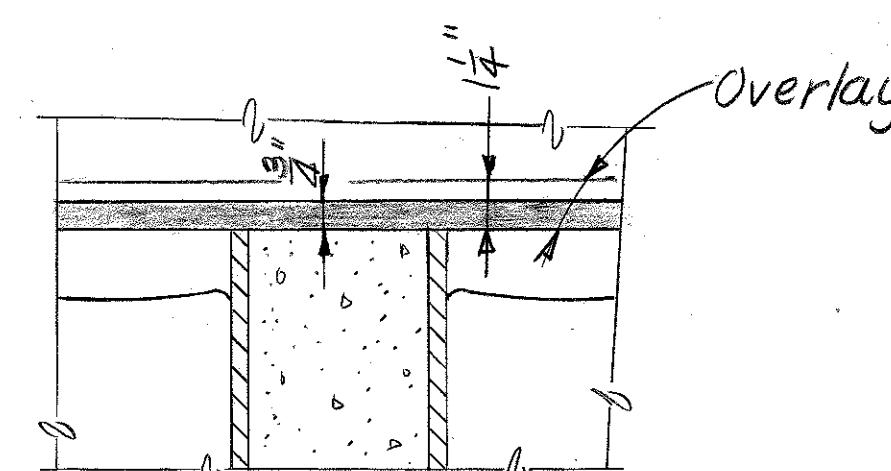
Note (A)
Coat surface of drain grates, which will be in contact with overlay material, with hot wax or tallow. No direct payment will be for this item but it shall be incidental to Deck Overlay.



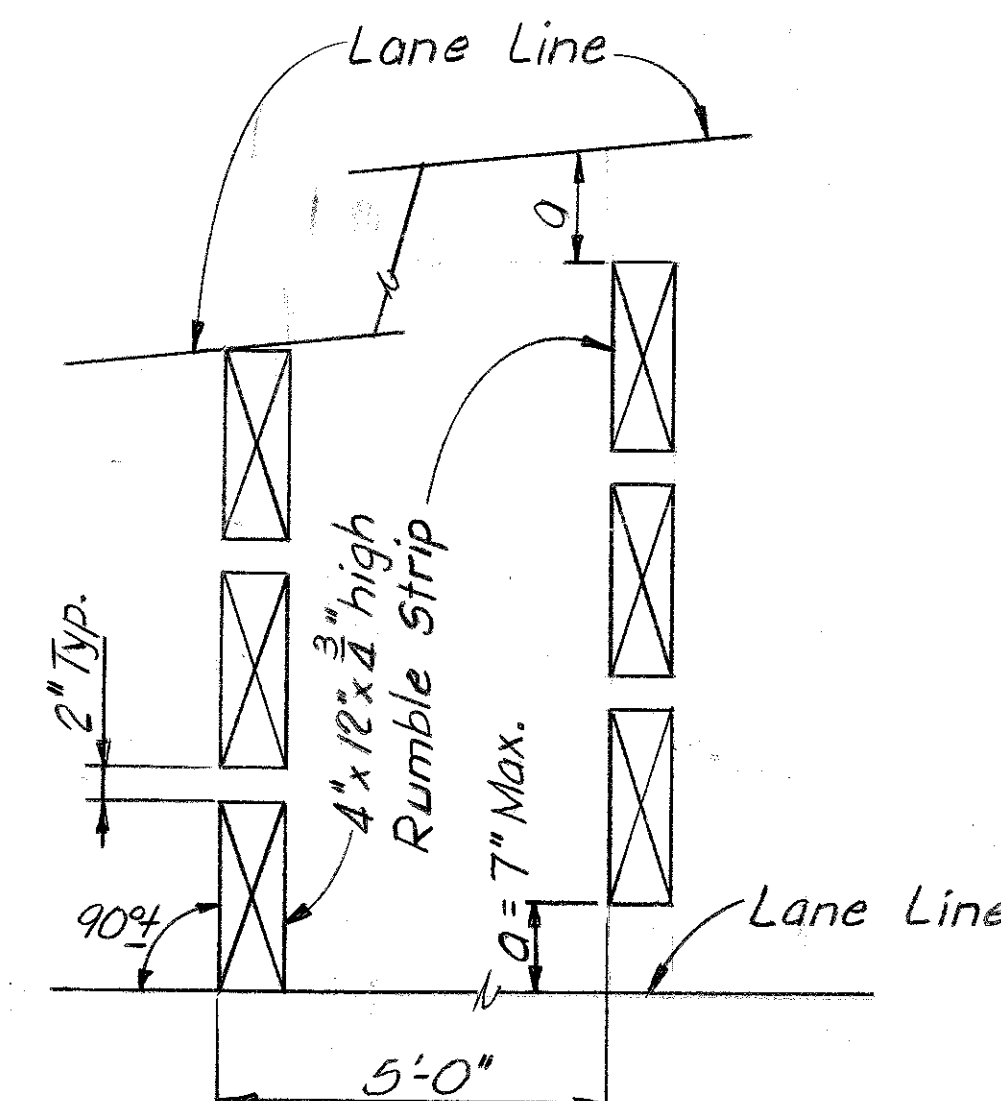
PLAN
HEAVY DUTY DRAIN AT PIER 6
(17TH - 13TH Street)



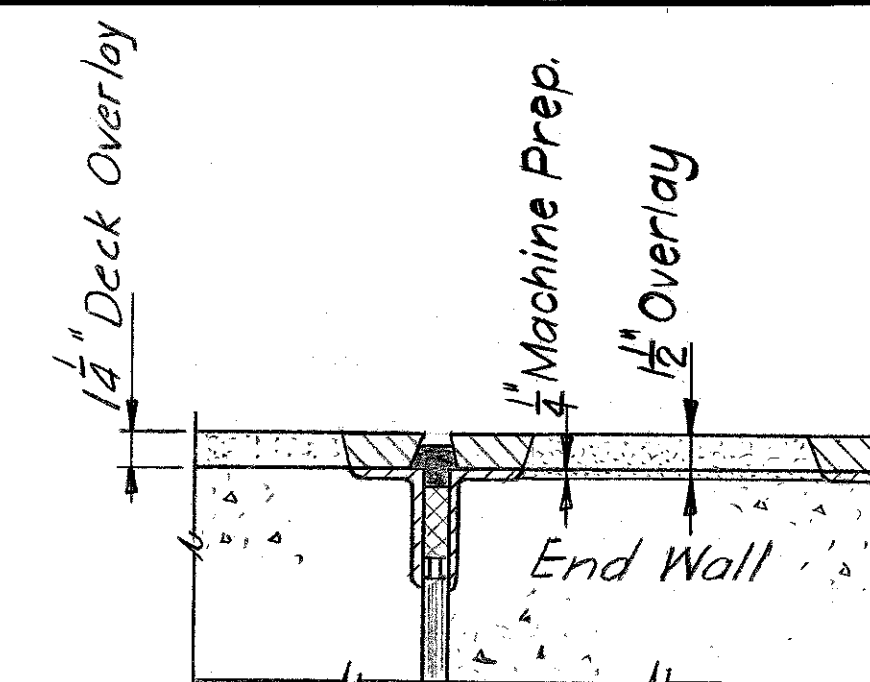
SECTION C-C



SECTION D-D



TYPICAL LAYOUT OF RUMBLE STRIPS
(See Sheet 47 for location)



For details not shown see "Section @ Armored Edge", Sheet. 45

DECK OVERLAY AT
END BENT 101 E
Existing Section
(For details not shown see
Section thru Armored
Edge, Sheet 44)

SHEET NO. 49

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

FRANKFORT

COUNTY OF

JEFFERSON
LOUISVILLE-LEXINGTON

ROAD

STATION 267+00

CONSTRUCTION PROJECT NO.

P. E. PROJECT NO. 164-2(92)

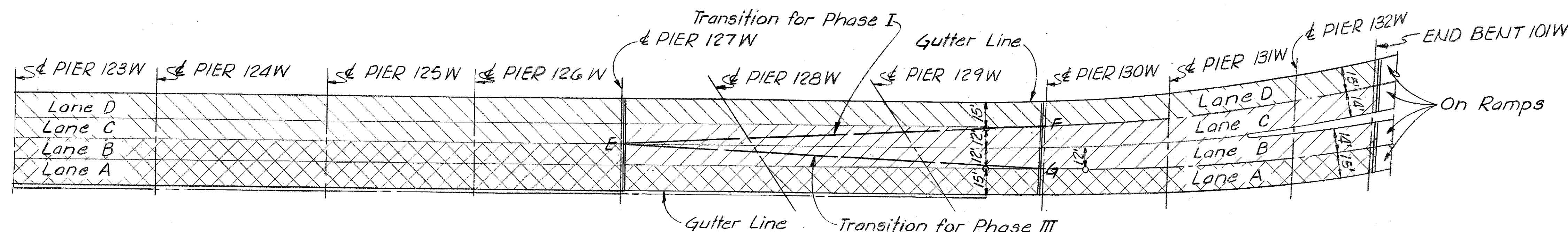
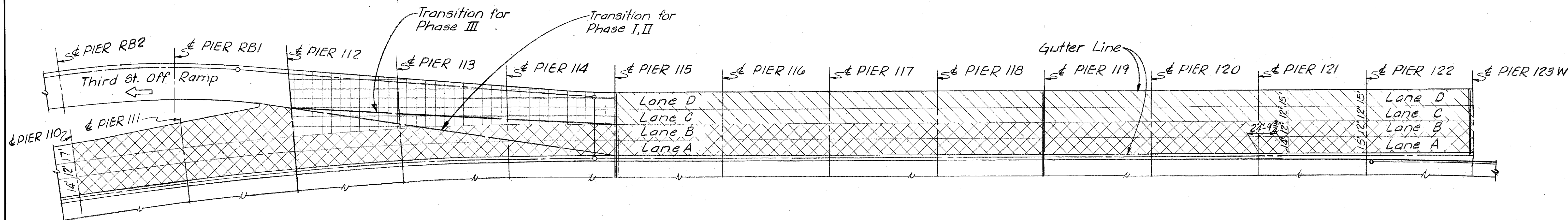
MAINTENANCE PROJECT NO.

DRAWING NO.

19238

DECK OVERLAY-DETAILS

UPDATE DATE
LETTING DATE



LEGEND

- Phase I
- Phase II
- Phase III
- Bituminous Overlay (By others)

PLAN OF DECK OVERLAY (PIER 110 to END BENT 101W)

CONSTRUCTION SEQUENCE

PHASE I OVERLAY

- 1a. Close all of Lane D from Pier 115 to beyond the structure on the ramp.
- 1b. Close Lane B on the ramp east of the structure.
- 1c. Transition Lane C traffic to Lane B by placing barricades along line EF from Pier 127W to Pier 130W.
- 1d. Close Lane C from Pier 115 to Pier 127W.
- 1e. Barricade from the gore of the Third St. Off Ramp to the left pavement edge at Pier 115. (Traffic at this point is using Lanes A and B from Pier 115 to Pier 127W).
- 1f. Surface all of Lane D and surface Lane C from Pier 115 to Pier 127W. (This is the portion shown thus).

PHASE II OVERLAY

- 2a. Open Lane D throughout and close Lane C from Pier 115 to east of the structure on the ramp.
- 2b. Close Lane B from Pier 115 to the east of the structure.
- 2c. Barricade between Lanes C and D and between Lanes A and B from Pier 115 to east of the structure on the ramps. Traffic is maintained at this point in Lanes A and D.
- 2d. Surface Lanes B and C from Pier 127W east to end of structure. (This is the portion shown thus).

PHASE III OVERLAY

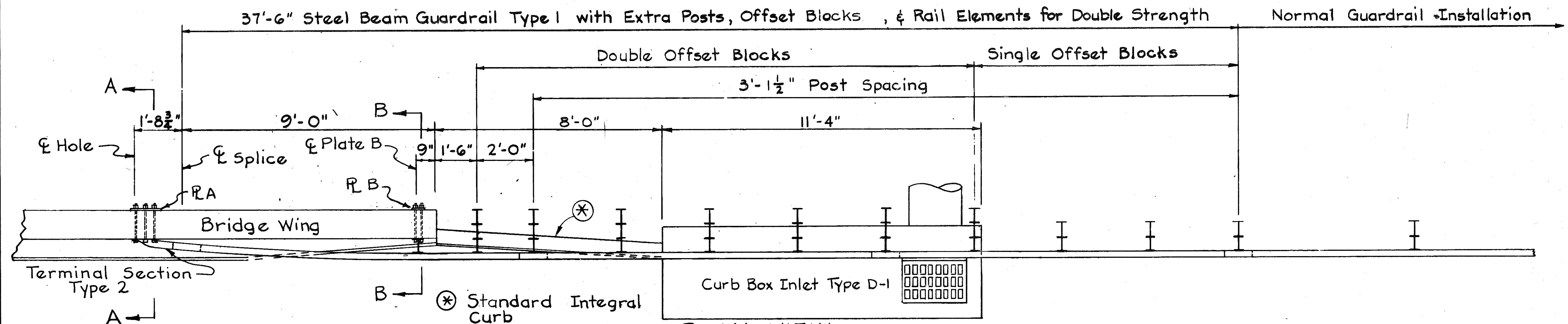
- 3a. Close Lane C on ramp east of the structure.
- 3b. Close Lane A from beginning of project to east of the structure on ramp.
- 3c. Transition Lane B traffic to Lane C by placing barricades along line EG from Pier 127W to Pier 130W.
- 3d. Close Lane B from beginning of Project to Pier 127W.
- 3e. Barricade from gore of Third St. Off Ramp to center of Pier 115 between Lane B and C. (Traffic using Lanes C and D from Third St. Ramp to Pier 127W).
- 3f. Surface all of Lane A and surface Lane B from beginning of project to Pier 127W. (This is the portion shown thus).

Signing and placement of barrels, barricades, etc., for lane closures on the ramp beyond the structure should conform as nearly as possible to standard drawing TSC-210. Due to the length available, modifications of spacing will be necessary and should, if possible, conform to the spacing shown for 35 and 40 MPH. Portable flashing arrows shall be used.

OVERLAY CONSTRUCTION SEQUENCE

SHEET 50

COMMONWEALTH OF KENTUCKY		
BUREAU OF HIGHWAYS		
FRANKFORT		
COUNTY OF		
JEFFERSON		
LOUISVILLE-LEXINGTON		
ROAD		
STATION 267+00	P. E. PROJECT NO. 164-2(92)	DRAWING NO.
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO.	19238



NOTES

1. GENERAL

- Guardrail Connector to Bridge End Type A-2 shall be utilized on the following projects where the design of the bridges has been completed prior to the effective date of this drawing: (1) Grade and Drain, (2) Grade, Drain, and Surfacing.
- See current Standard Drawings in the RBB, RBI, RBR, and RPM series for other related details.
- See sheet _____ of _____ of these plans for Curb Box Inlet Type D-1.

2. MATERIAL REQUIREMENTS

- All hardware shall be galvanized. (ASTM A153)
- (1) $\frac{1}{4}$ " Steel Plate A. (ASTM A36)
- (1) $\frac{1}{4}$ " Steel Plate B. (ASTM A36)
- (6) $\frac{7}{8}$ " Hex Head Bolts (Length As Shown). (ASTM A325)
- (6) $\frac{7}{8}$ " Heavy Hex Nuts ($\frac{7}{8}$ " Thick). (ASTM A325)
- (8) $\frac{7}{8}$ " Flat Washers ($\frac{3}{16}$ " Thick). (ASTM A325)
- (4) $\frac{7}{8}$ " Beveled Washers ($\frac{5}{16}$ " Mean Thickness). (ASTM A325, AASHTO 2.10.3)

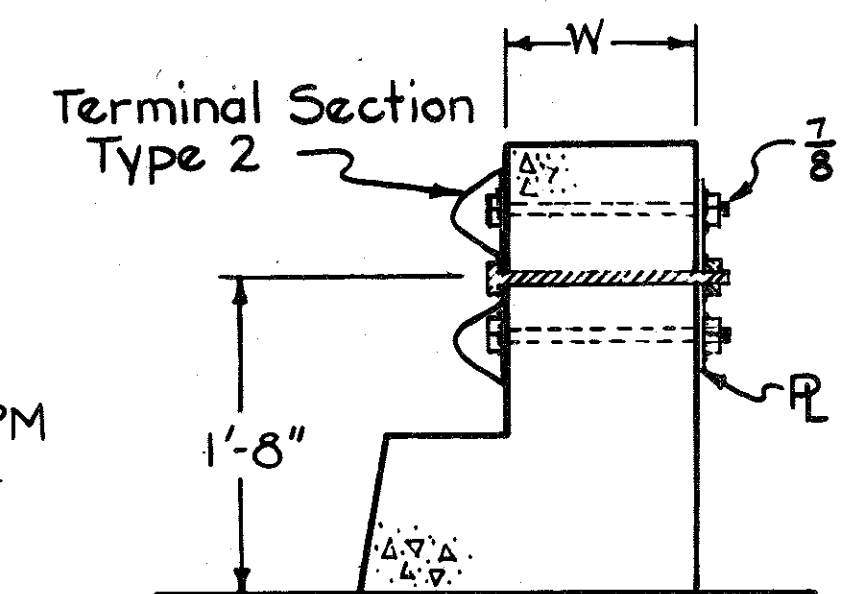
3. CONSTRUCTION METHODS

- Back-up Plates are required at all posts except at splices.
- See Bridge Plans for Bridge Wing details.

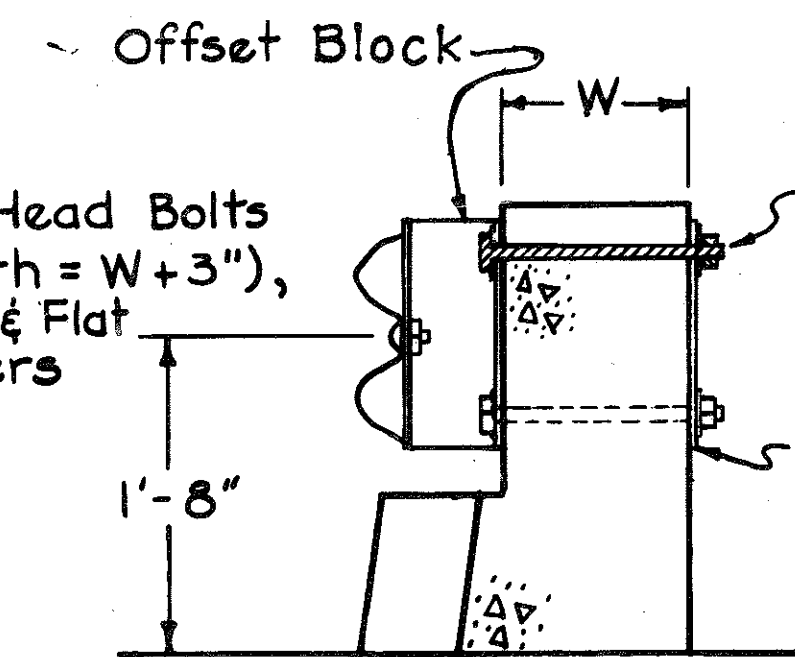
4. METHOD OF MEASUREMENT AND BASIS OF PAYMENT

- Guardrail Connector to Bridge End Type A-2 shall be paid for at the contract unit price each, and includes: Terminal Section Type 2; all items which are in addition to the normal installation of Steel Beam Guardrail Type I (Extra Posts, Offset Blocks, Rail Elements, Hardware, Etc.); and all other incidentals necessary to complete the installation as detailed.
- Steel Beam Guardrail Type I is a separate bid item and is always required. The Integral Curb and Curb Box Inlet Type D-1 are separate bid items that shall be used when required for Bridge End Drainage.
- The Plastic Pipe and cost of forming shall be included in the unit price bid for bridge superstructure concrete.

PLAN VIEW



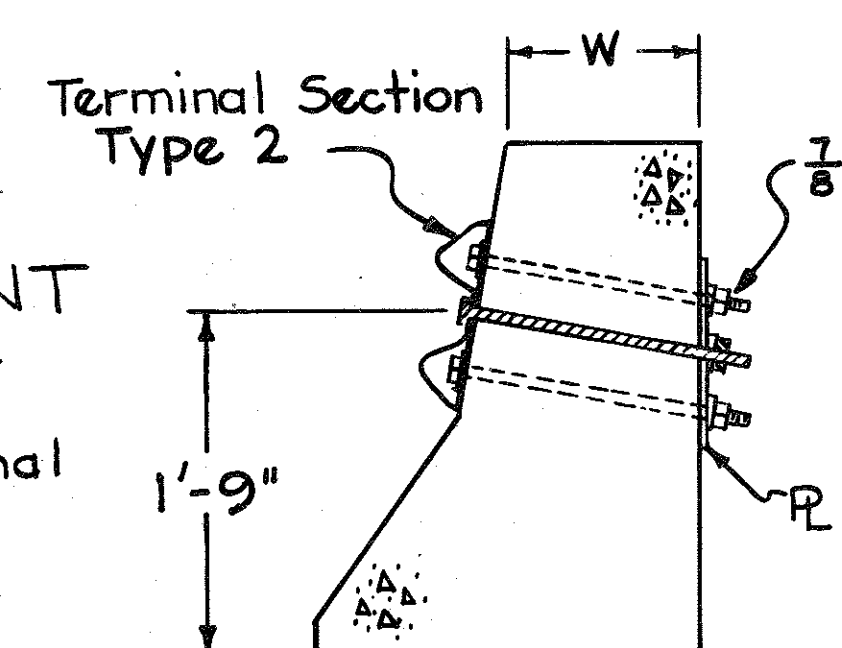
SECTION A-A



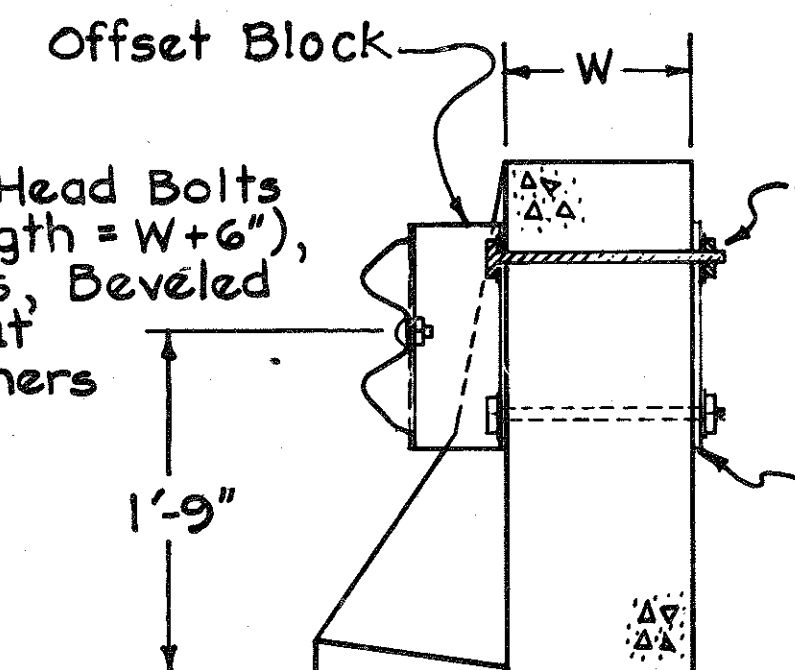
SECTION B-B

BRIDGES WITH 9" OR LESS BRUSH BLOCK WIDTH

Note: Form 1" Holes through Bridge Wings perpendicular to traffic side of wing with 1" I.D. Plastic Pipe, which shall remain in place after fabrication.

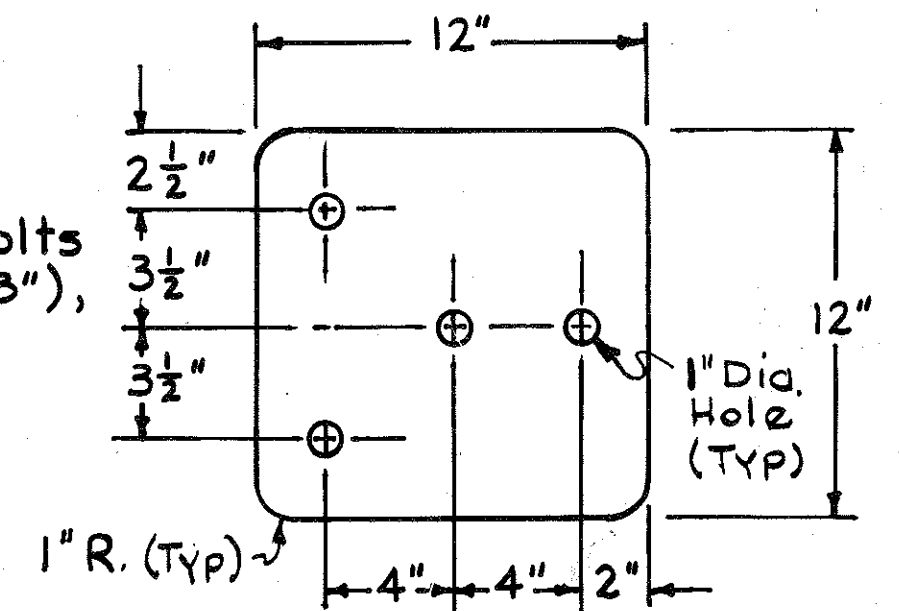


SECTION A-A

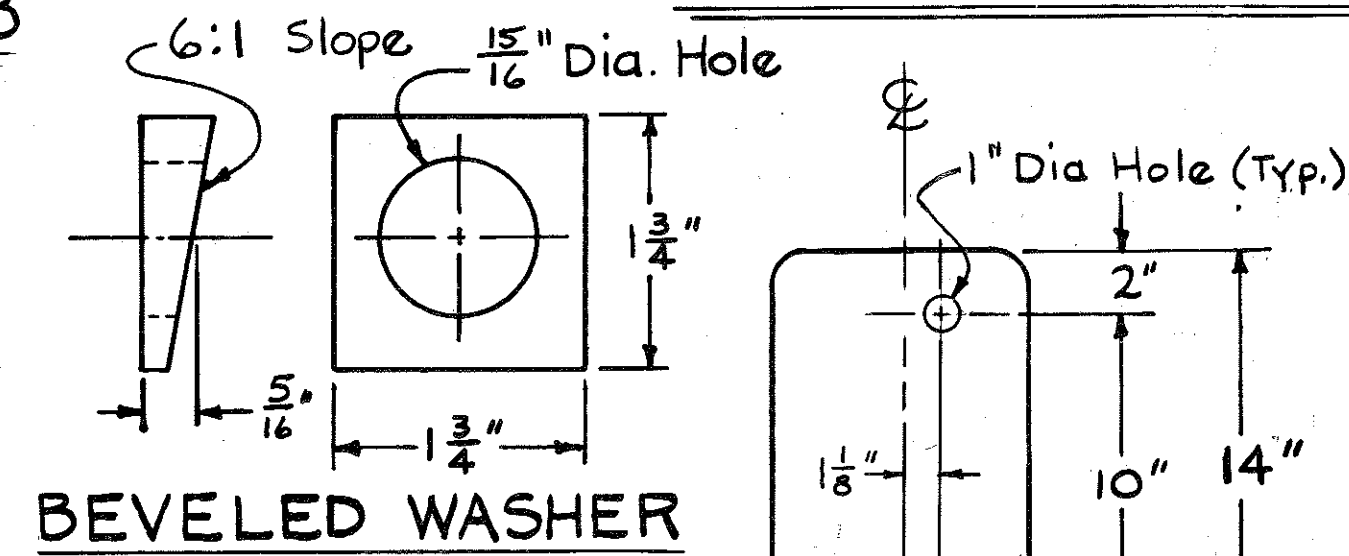


SECTION B-B

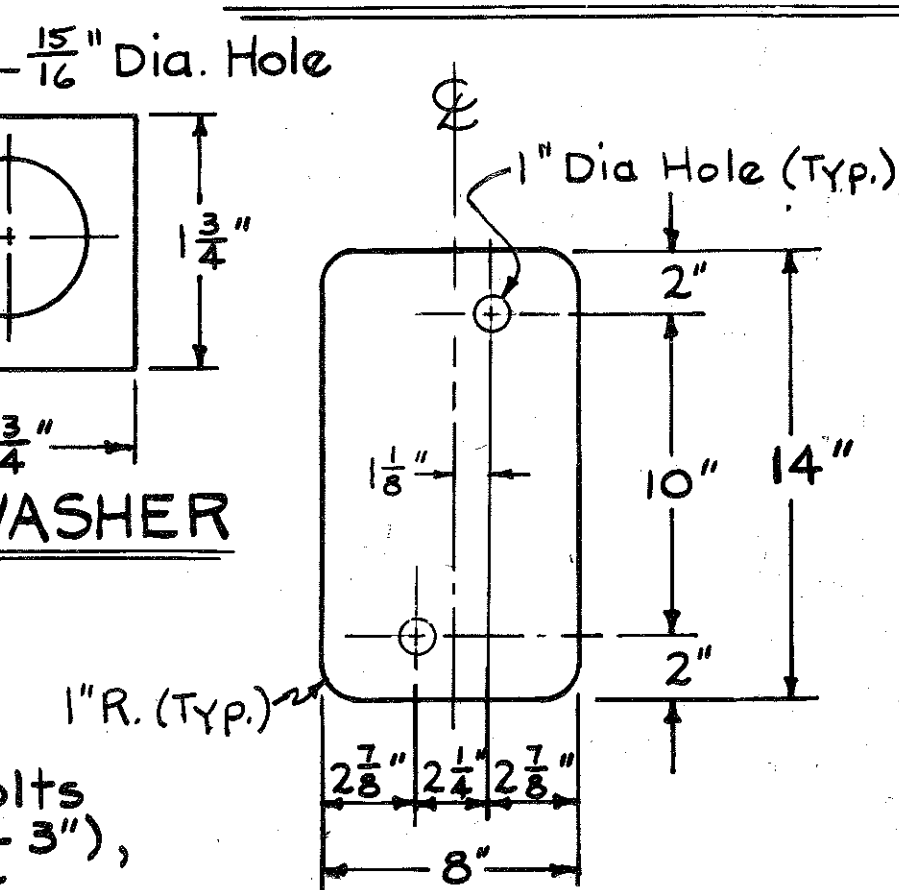
BRIDGES WITH G.M. TYPE BARRIER CURB



1/4" STEEL PLATE A



BEVELED WASHER



1/4" STEEL PLATE B

Drawing No. 19238 Sheet 51

KENTUCKY
BUREAU OF HIGHWAYS
GUARDRAIL CONNECTOR
TO BRIDGE END
TYPE A-2

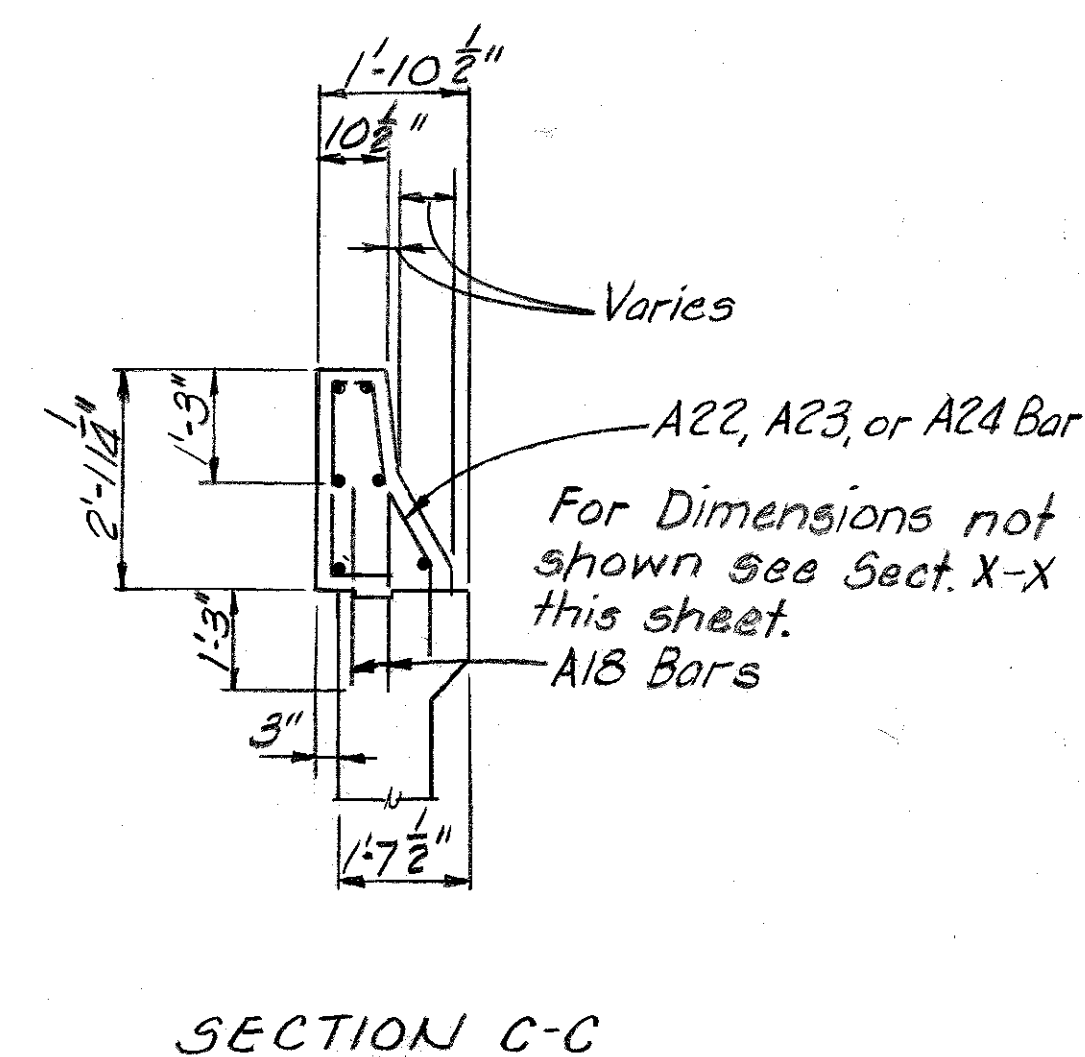
STANDARD DRAWING No.
SUBMITTED *E.B. Drake* *Asst.* 7/16/74
DIRECTOR DIVISION OF DESIGN
APPROVED _____
STATE HIGHWAY ENGINEER

FOR INFORMATION ONLY

DESIGNED BY	CHECKED BY	DATE	REVISED	DATE
TRAFFICED BY	CHECKED BY	DATE	REVISED	DATE

Δ Indicates Revision for G.M. Barrier

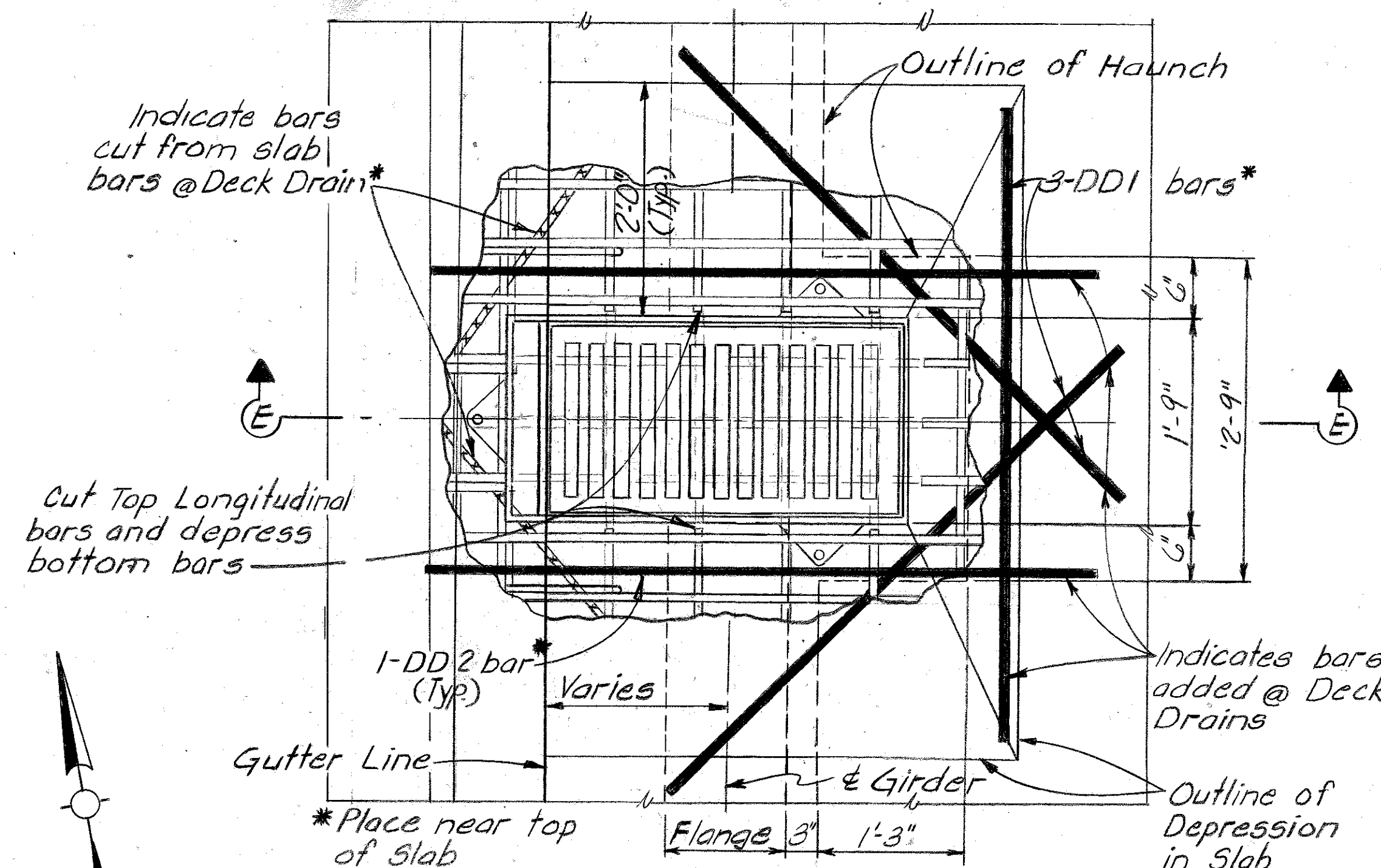
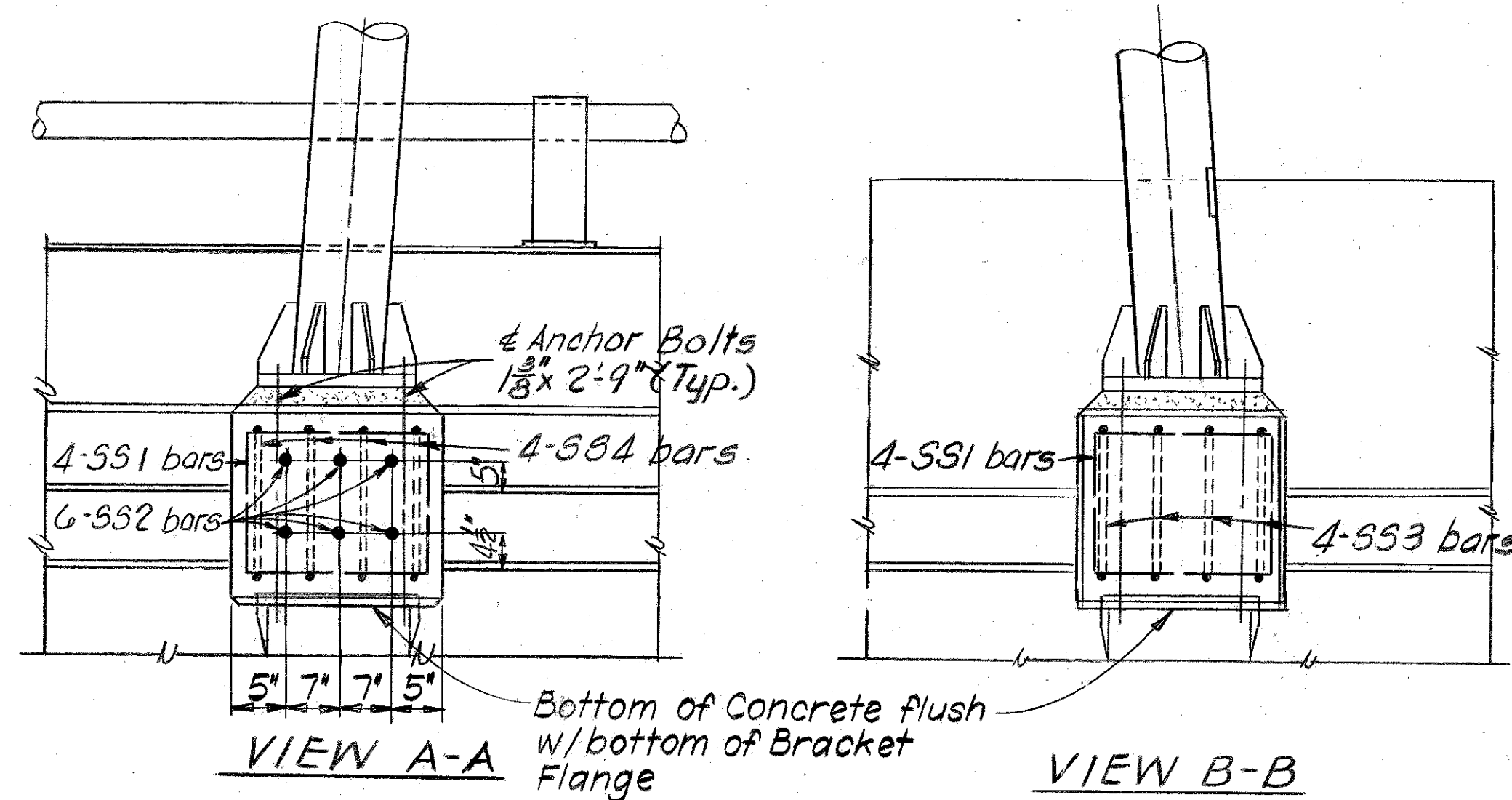
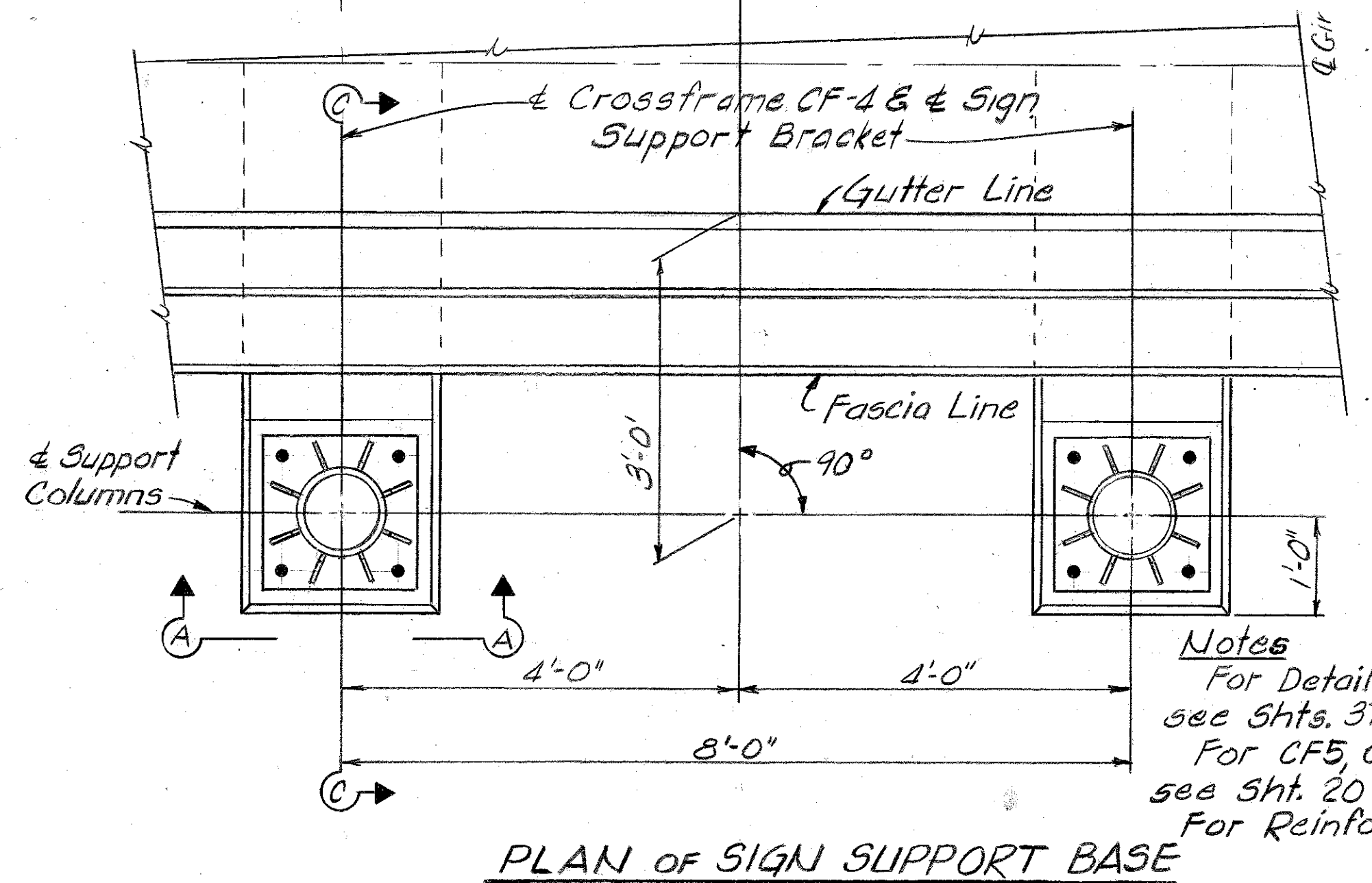
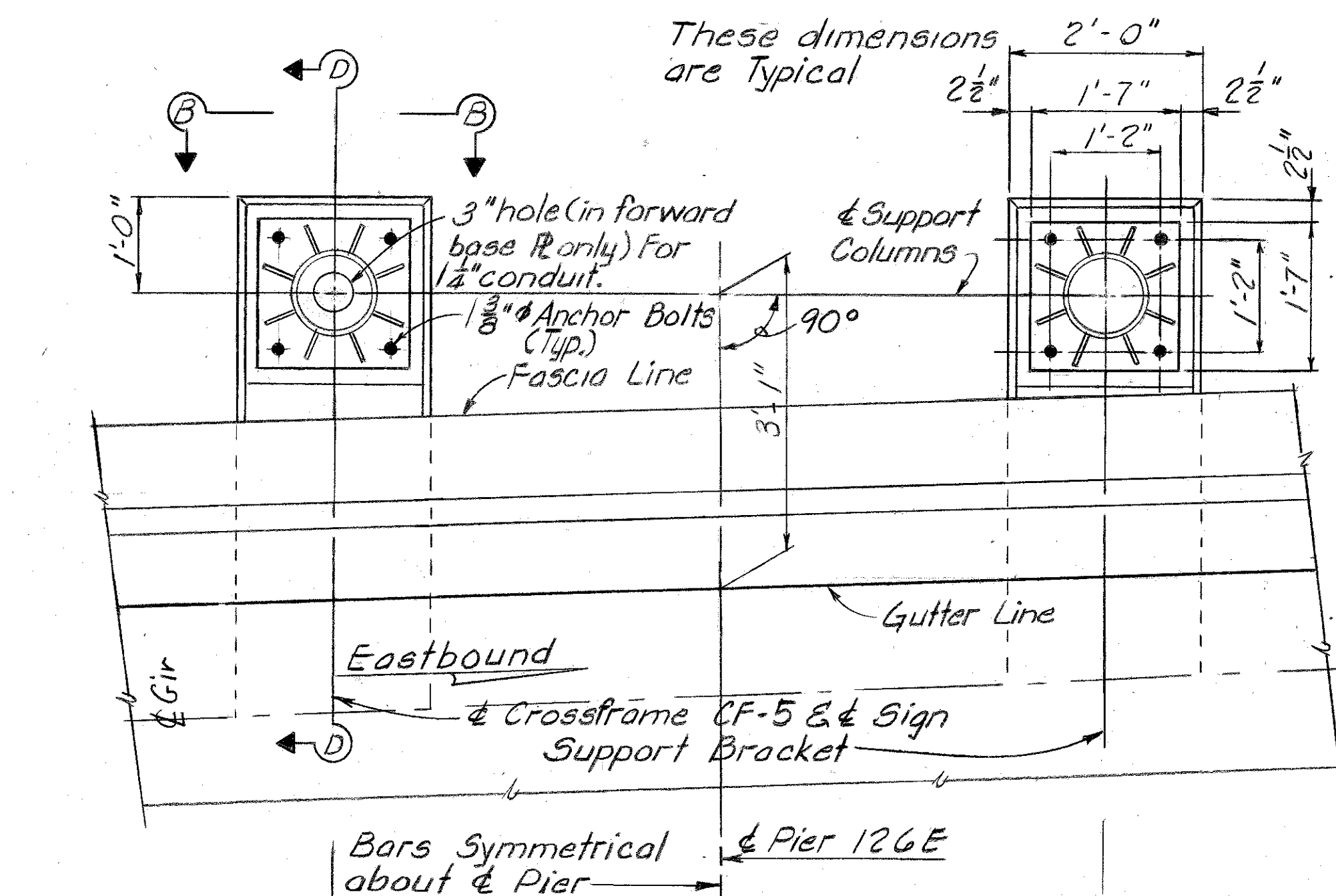
2/75



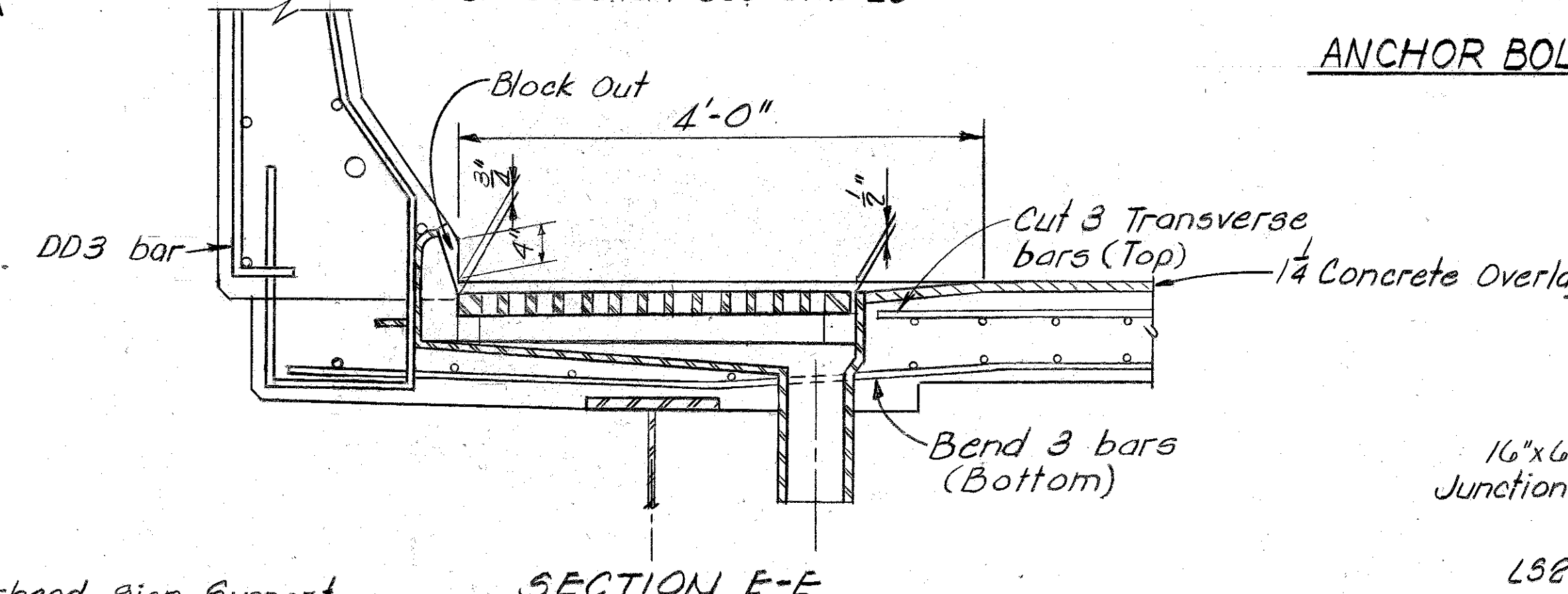
COMMONWEALTH OF KENTUCKY
BUREAU OF HIGHWAYS
FRANKFORT
COUNTY OF
JEFFERSON
LOUISVILLE-LEXINGTON
ROAD

LETTING DATE

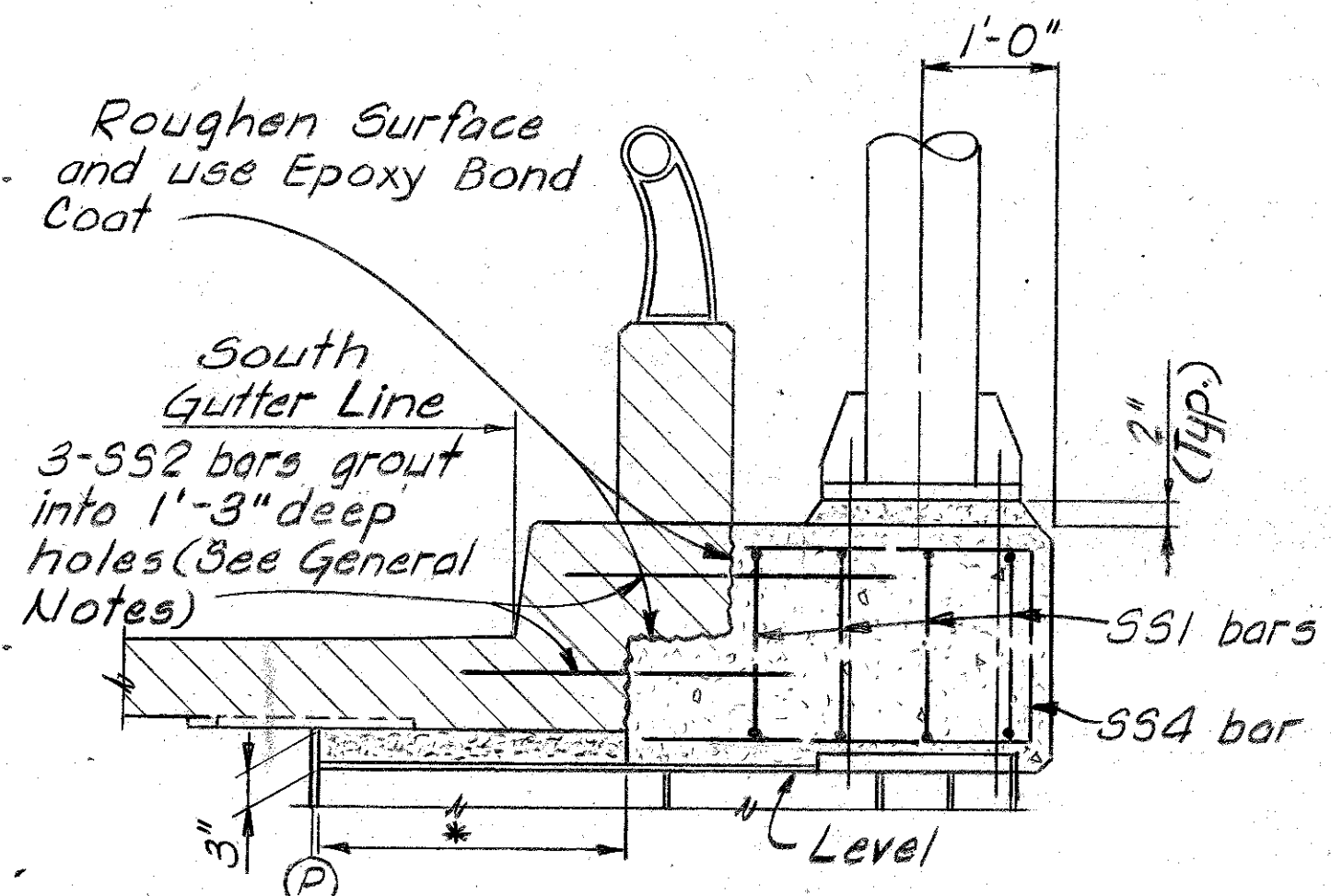
DESIGNED BY: **PM** DATE: **7-75**
 CHECKED BY: **PM** DATE: **7-75**
 TRACED BY: **PM** DATE: **7-75**
 INDICATES REVISION FOR G.M. BARRIER



PLAN OF DECK DRAIN
For Location see Sht. 26

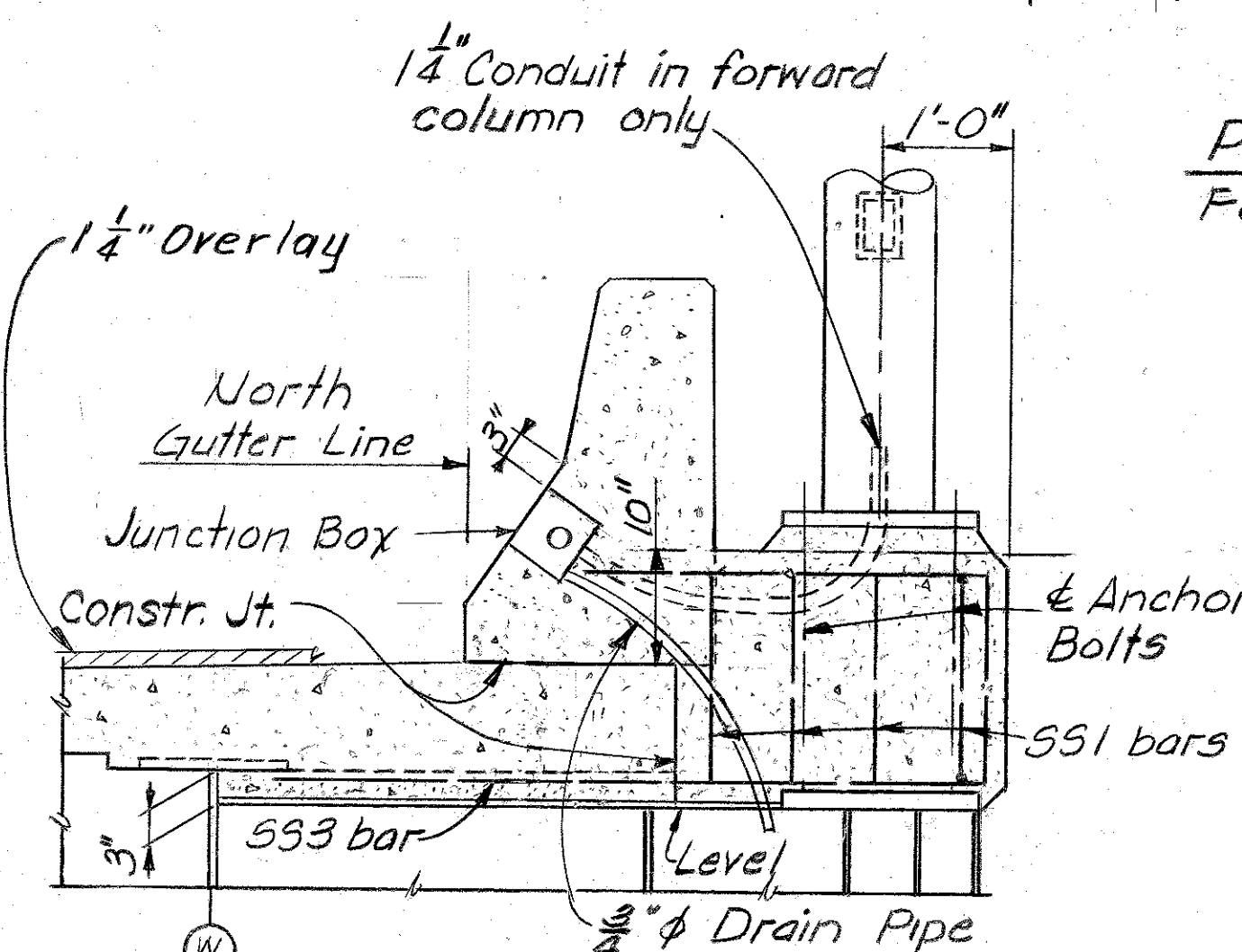


SECTION E-E



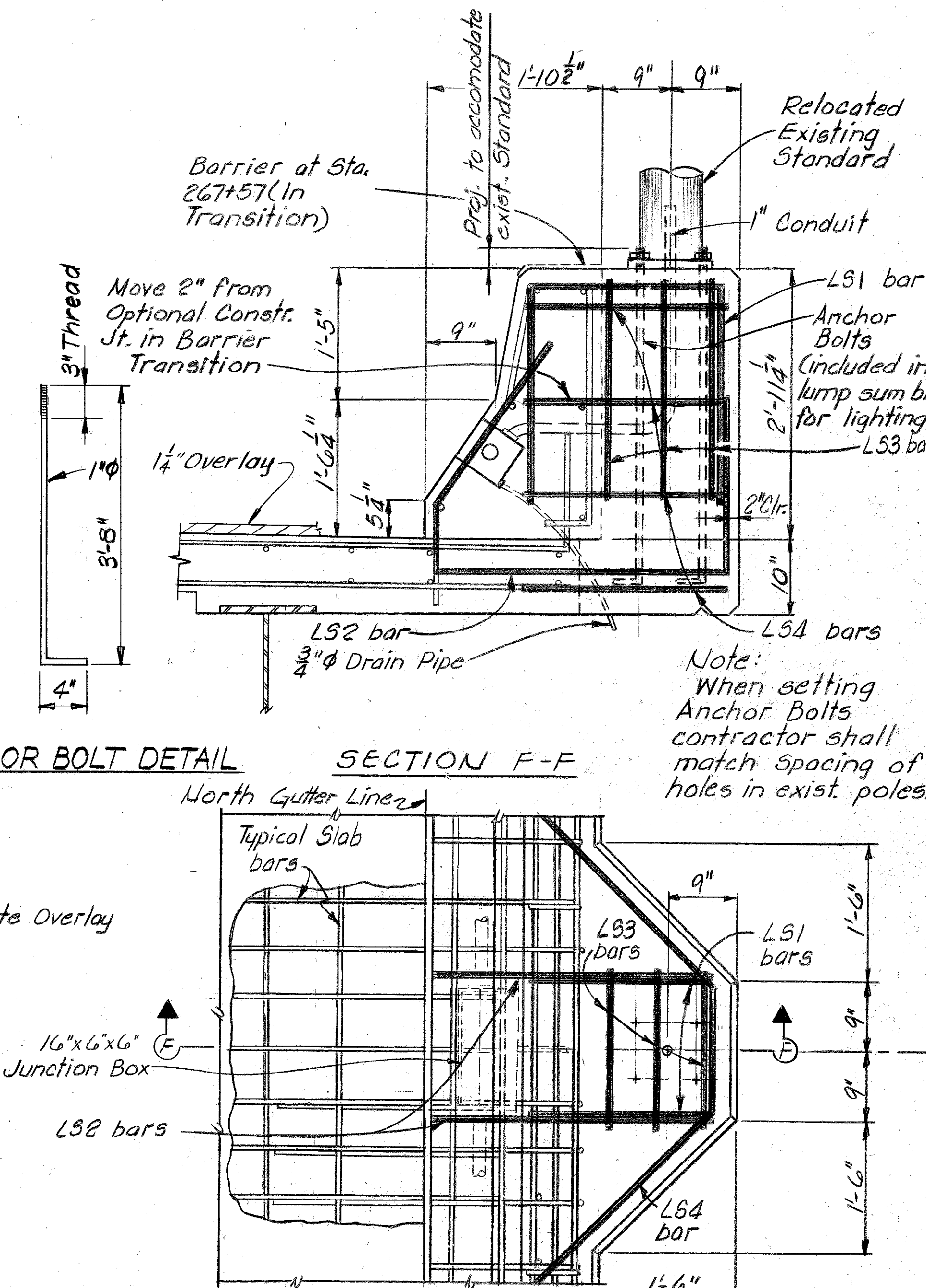
SECTION C-C

* Fill area between top flange of bracket and bottom of slab w/ expansive grout.



SECTION D-D

DECK-MISC. DETAILS



PLAN OF LIGHT STANDARD BASE
For Reinforcing Bar Details see Sht. 54
For Location see Shts. 25 & 26.

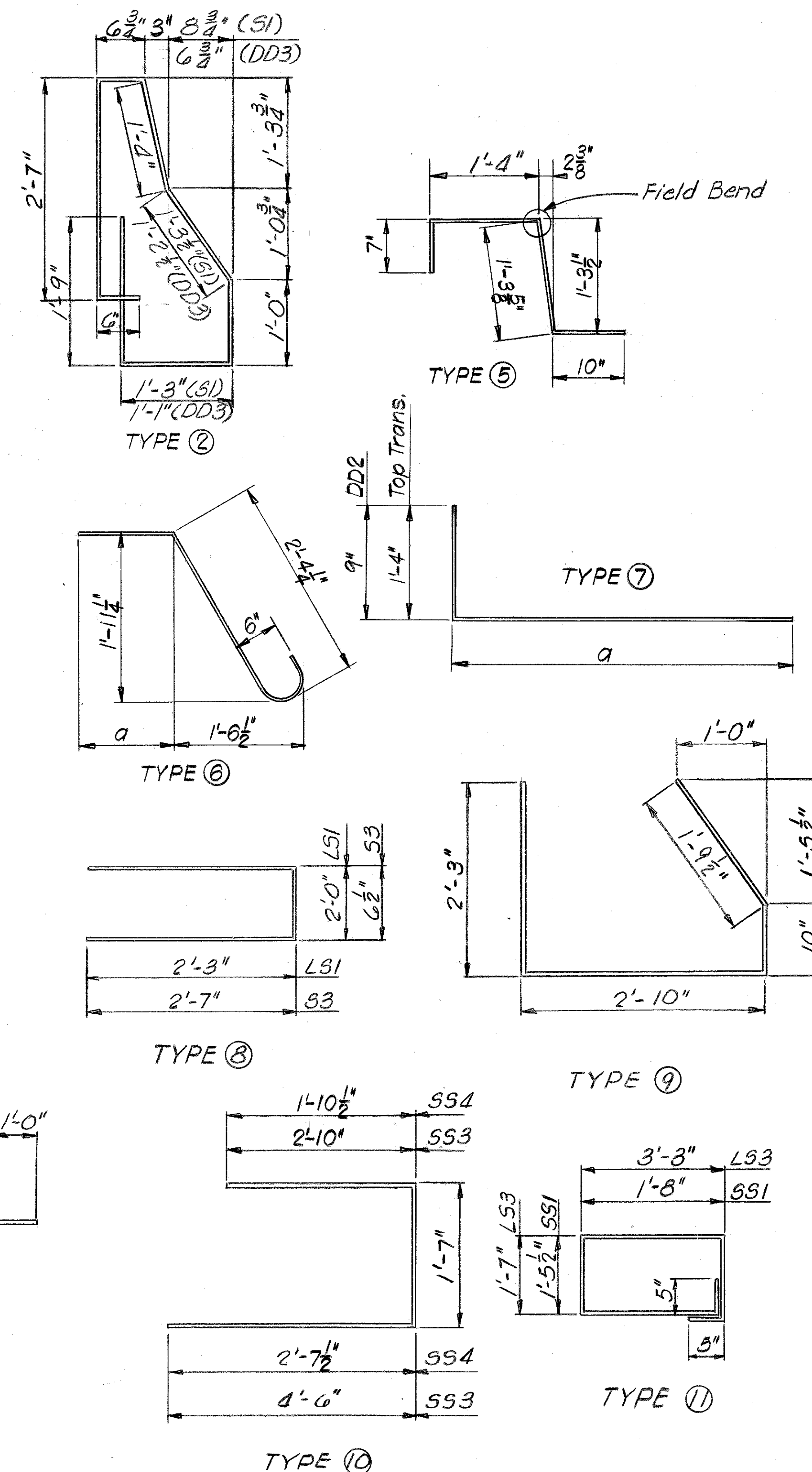
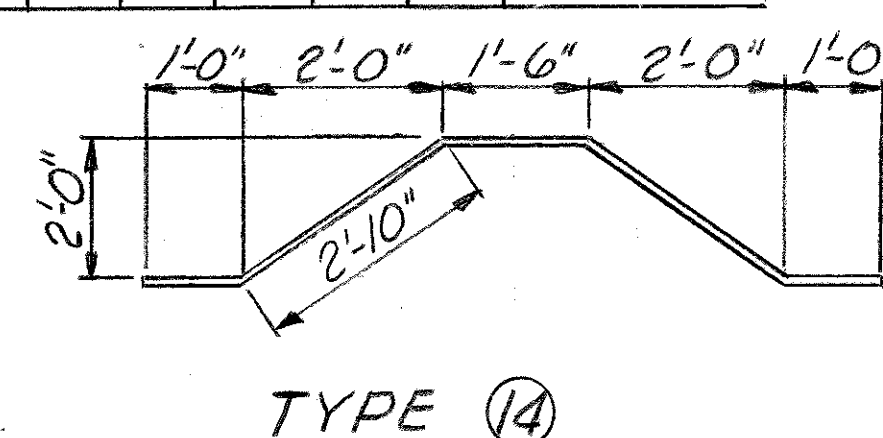
B4 (2) - 56 (3) SLAB																			
MARK	TYPE	SIZE	NO. OF BARS IN SPAN EAST OF PIER												LENGTH		a		LOCATION
			122	123	124	125A	126	127	128	129A	129B	130A	131	ft.	in.	ft.	in.		
S1	(2)	5	56	80	72	109	101	80	56	65	63	72	47	10	0	-	-	Barrier Tie	
S2	(5)	5	24	-	-	-	-	-	-	-	-	-	-	4	0	-	-	" "	
S3	(8)	5	48	-	-	-	-	-	-	-	-	-	-	5	8	-	-	" "	
S4	(6)	6	80	30	-	109	101	80	56	65	65	74	47	4	8	1	7	Haunch	
S5	Str.	4	13	-	-	-	-	-	-	-	-	91	-	42	8	-	-	T/SL	
S6	"	5	13	-	-	-	-	-	-	-	-	127	-	43	1	-	-	B/SL	
S7	(7)	6	158	-	-	-	-	-	-	-	-	-	-	6	6	5	3	T/ST	
S8	Str.	6	68	-	-	-	-	-	-	-	-	-	-	5	3	-	-	B/ST	
S9	"	4	12	-	12	18	-	-	-	-	-	-	-	41	0	-	-	LB	
S10	(7)	6	-	-	144	-	-	-	-	-	-	-	-	11	5	10	2	T/ST	
S11	Str.	6	-	-	144	-	-	-	-	-	-	-	-	10	2	-	-	B/ST	
S12	"	4	-	17	-	-	-	35	-	-	-	-	-	45	0	-	-	T/SL	
S13	"	5	-	20	-	-	-	50	-	-	-	-	-	45	5	-	-	B/SL	
S14	"	4	-	18	-	-	-	18	12	-	-	-	-	31	0	-	-	LB	
S15	(7)	6	-	-	-	216	-	-	-	-	-	-	-	13	10	12	7	T/ST	
S16	Str.	6	-	-	-	216	-	-	-	-	-	-	-	12	7	-	-	B/ST	
S17	"	4	-	-	21	-	-	-	-	-	-	-	-	40	7	-	-	T/SL	
S18	"	5	-	-	28	-	-	-	-	-	-	-	-	41	0	-	-	B/SL	
S19	(7)	6	-	-	-	-	201	-	-	-	-	-	-	16	0	14	9	T/ST	
S20	Str.	6	-	-	-	-	201	-	-	-	-	-	-	14	9	-	-	B/ST	
S21	(7)	6	-	-	-	-	-	159	-	-	-	-	-	19	3	18	0	T/ST	
S22	Str.	6	-	-	-	-	-	159	-	-	-	-	-	18	0	-	-	B/ST	
S23	"	4	-	-	-	-	18	-	-	12	12	-	-	37	0	-	-	LB	
S24	(7)	6	-	-	-	-	-	56	-	-	-	-	-	21	4	20	1	T/ST	
S25	(7)	6	-	-	-	-	-	55	-	-	-	-	-	23	5	22	2	"	
S26	Str.	6	-	-	-	-	-	56	-	-	-	-	-	20	1	-	-	B/ST	
S27	"	6	-	-	-	-	-	55	-	-	-	-	-	22	2	-	-	"	
S28	"	4	-	-	-	39	-	-	42	-	-	-	-	31	8	-	-	T/SL	
S29	"	5	-	-	-	55	-	-	64	-	-	-	-	32	1	-	-	B/SL	
S30	"	4	-	-	-	-	-	-	-	-	-	12	-	42	0	-	-	LB	
S31	"	4	-	-	-	-	-	-	-	-	-	-	12	26	0	-	-	LB	
S32	"	4	-	-	-	11	-	-	-	51	20	-	-	28	0	-	-	T/SL	
S33	"	5	-	-	-	16	-	-	-	73	24	-	-	28	5	-	-	B/SL	
S34	"	4	-	-	-	-	46	-	-	-	50	-	-	37	8	-	-	T/SL	
S35	"	5	-	-	-	-	65	-	-	-	80	-	-	38	1	-	-	B/SL	
S36	"	4	-	-	-	-	-	-	-	30	12	-	-	19	2	-	-	T/SL	
S37	"	5	-	-	-	-	-	-	-	47	17	-	-	19	7	-	-	B/SL	
S38	"	4	-	-	-	-	-	-	-	-	-	-	96	26	8	-	-	T/SL	
S39	"	5	-	-	-	-	-	-	-	-	-	-	130	27	1	-	-	B/SL	
S40	"	5	-	5	-	8	12	14	-	21	31	40	-	15	0	-	-	T/S@Piers	
S41	(7)	6	-	159	-	-	-	-	-	-	-	-	-	9	3	8	0	T/ST	
S42	(7)	6	-	-	-	-	-	-	-	43	-	-	-	25	8	24	5	"	
S43	Str.	6	-	-	-	-	-	-	-	43	-	-	-	24	5	-	-	B/ST	
S44	(7)	6	-	-	-	-	-	-	-	43	-	-	-	28	0	26	9	T/ST	
S45	Str.	6	-	-	-	-	-	-	-	43	-	-	-	26	9	-	-	B/ST	
S46	(7)	6	-	-	-	-	-	-	-	43	-	-	-	30	4	29	1	T/ST	
S47	Str.	6	-	-	-	-	-	-	-	43	-	-	-	29	1	-	-	B/ST	
S48	(7)	6	-	-	-	-	-	-	-	-	44	-	-	33	2	31	11	T/ST	
S49	Str.	6	-	-	-	-	-	-	-	-	44	-	-	31	11	-	-	B/ST	
S50	(7)	6	-	-	-	-	-	-	-	-	43	-	-	36	0	34	9	T/ST	
S51	Str.	6	-	-	-	-	-	-	-	-	43	-	-	34	9	-	-	B/ST	

[illegible]

T/ST=Top of Slab Transverse
B/ST= Bott. " " "
T/SL= Top " " Longitudinal
B/SL= Bott. " " "
LB= Longitudinal Barrier Wall
SS/NF= Sign Support, North Fascia
SS/SF= " " , South "

Long Haunch Bars are included with B/SL Bars.

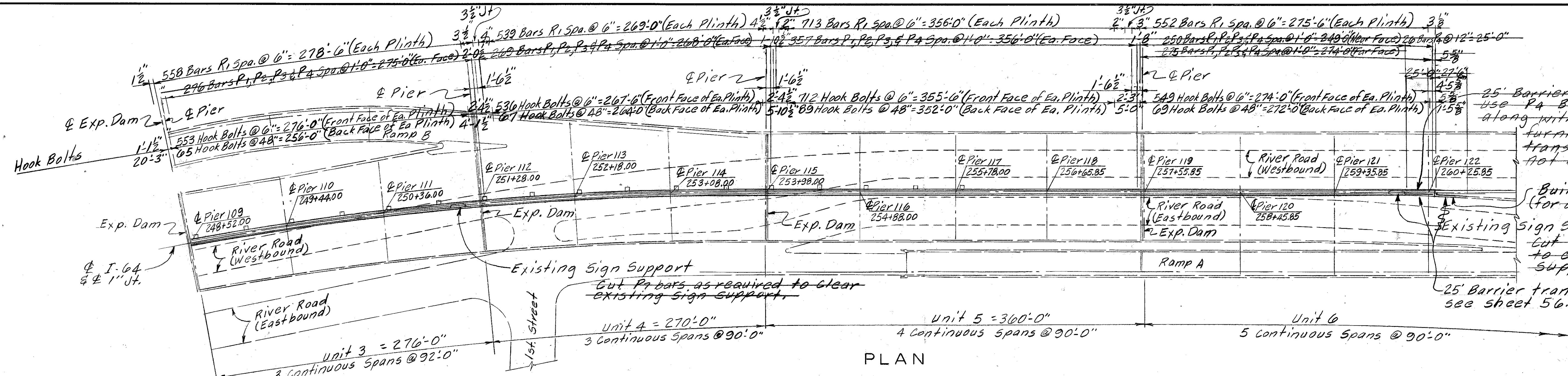
* Indicates no change



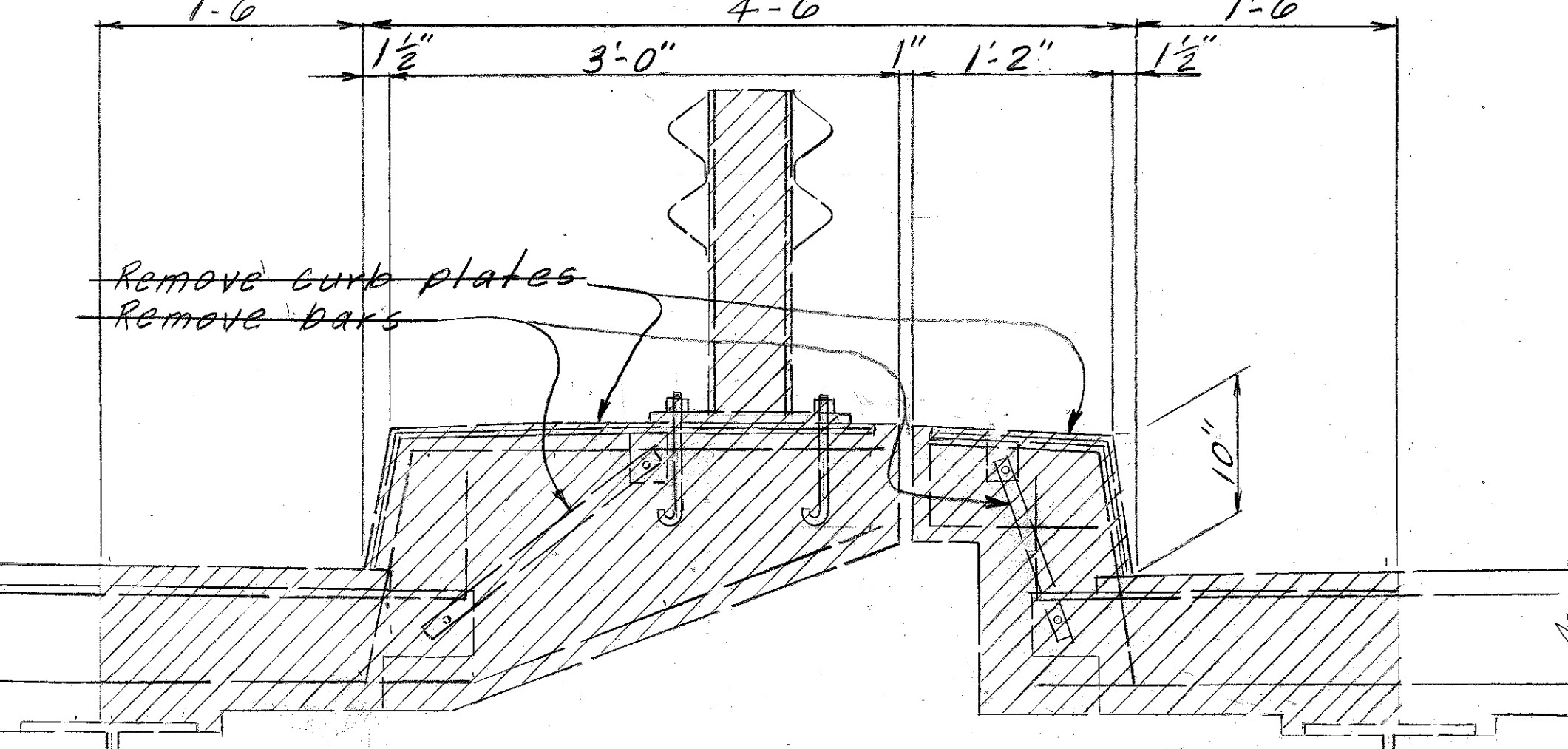
DECK BAR LIST Δ

UPDATE DATE
LETTING DATE

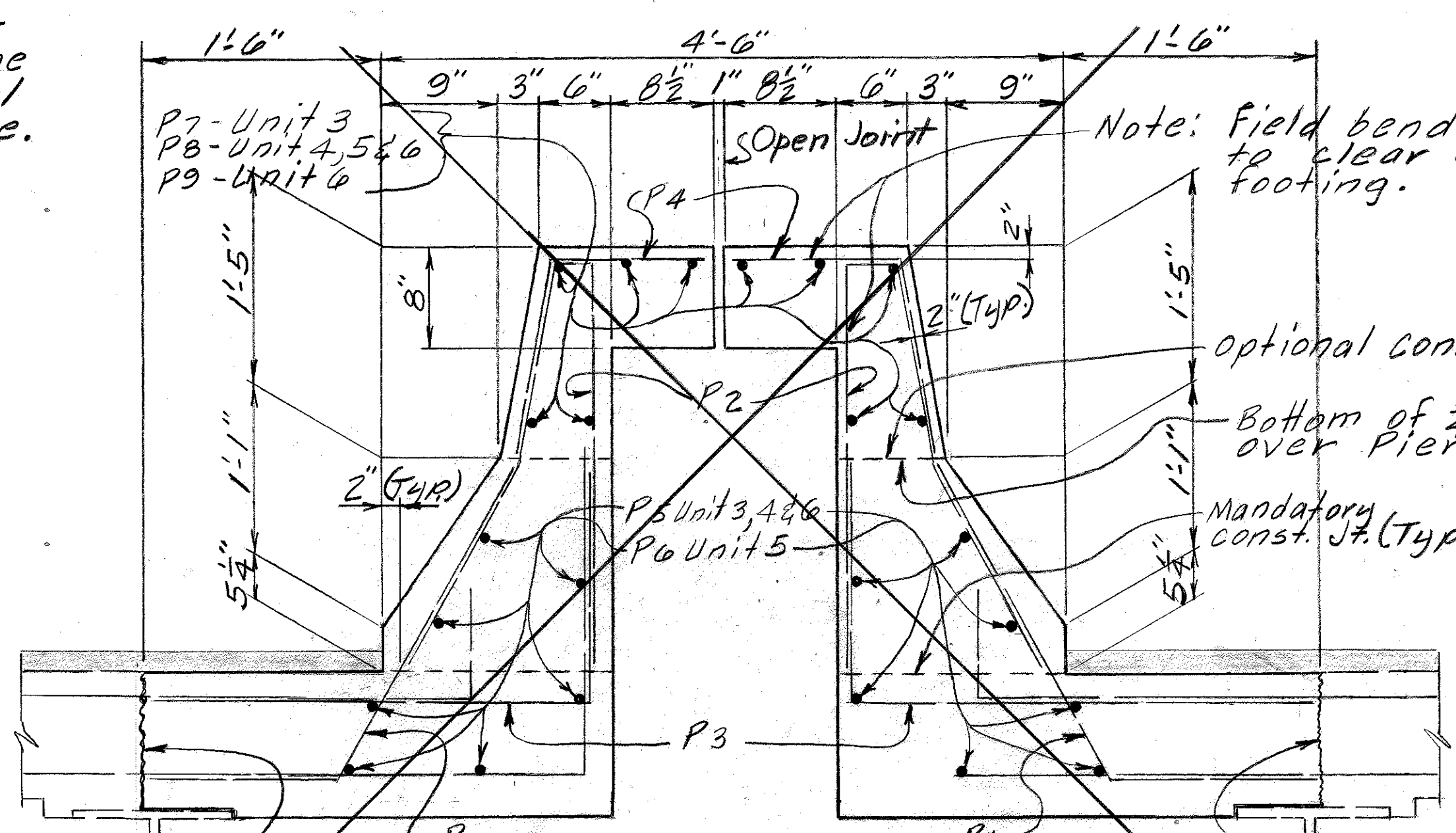
DESIGNED BY: Stratton CHECKED BY: A. DUNCAN DATE: 3/76
 TRACED BY: _____ DATE: _____
 REVISIONS: _____
 1. Replaces Barrier with Plinth



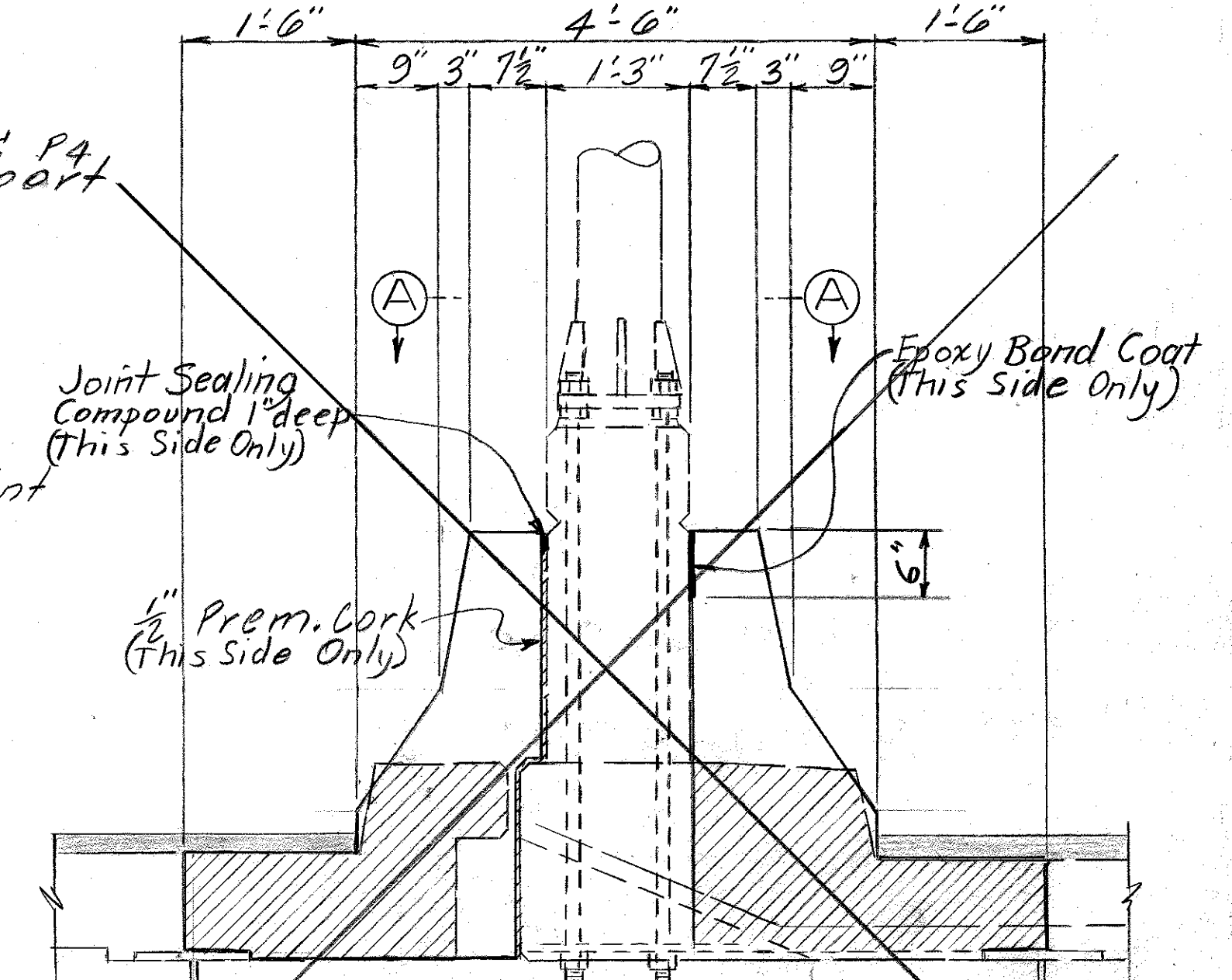
Remove Guardrail and Guardrail Posts and store at a site near Job site as directed by the Engineer. These items shall remain the property of the Bureau. The cost of this work shall be incidental to the Lump Sum Bid for Removal of Existing Reinforced Concrete.



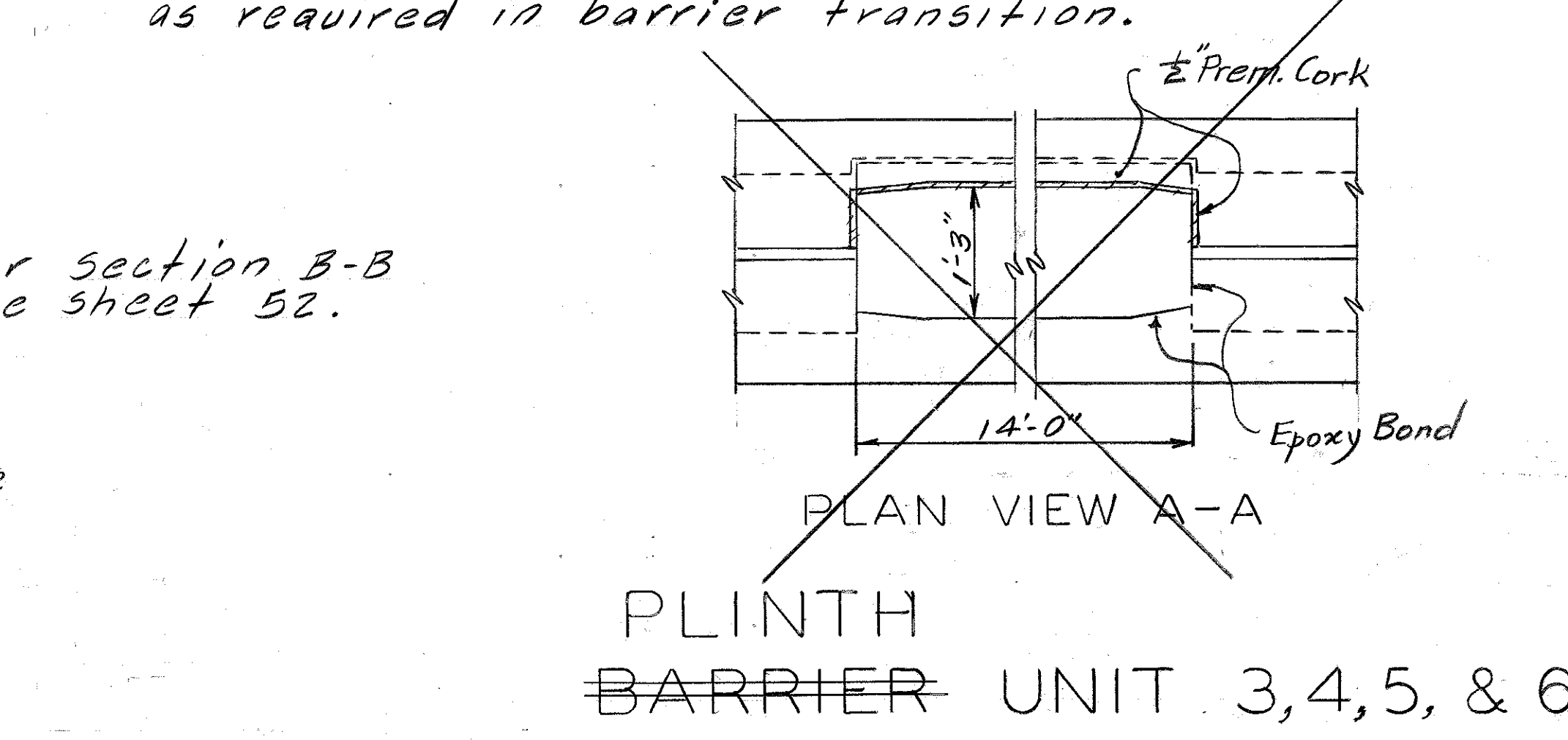
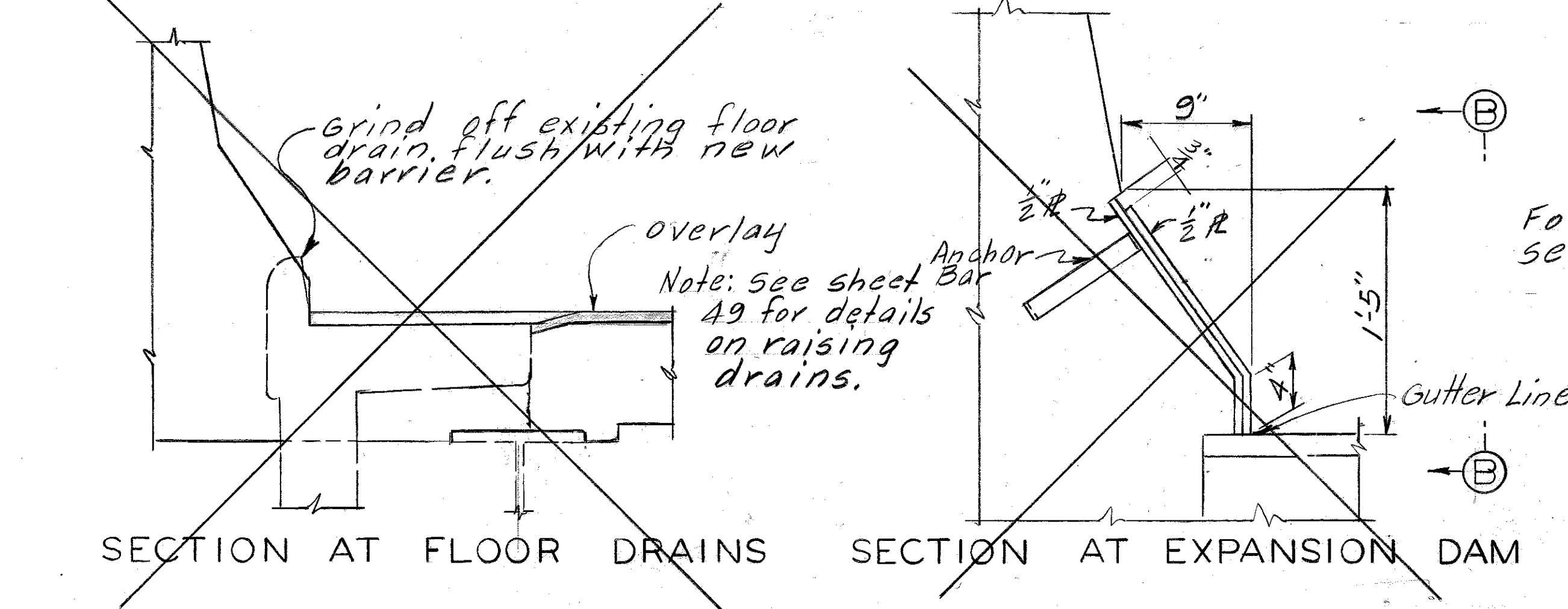
Remove cross hatched portion of existing concrete. Do not cut existing horizontal reinforcement. Clean and straighten to embed in new concrete.
 TYPICAL SECTION (showing existing concrete, guardrail and portion of expansion dam to be removed)



Epoxy Bond Coat to be applied to existing slab before new concrete is poured.
 TYPICAL SECTION
 Note: Bars P1, P2, P3 & P4 may be field bent as required in barrier transition.

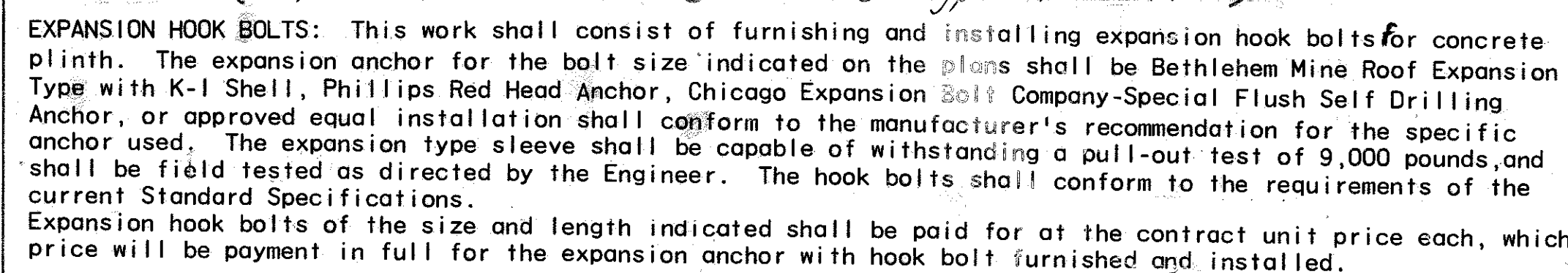
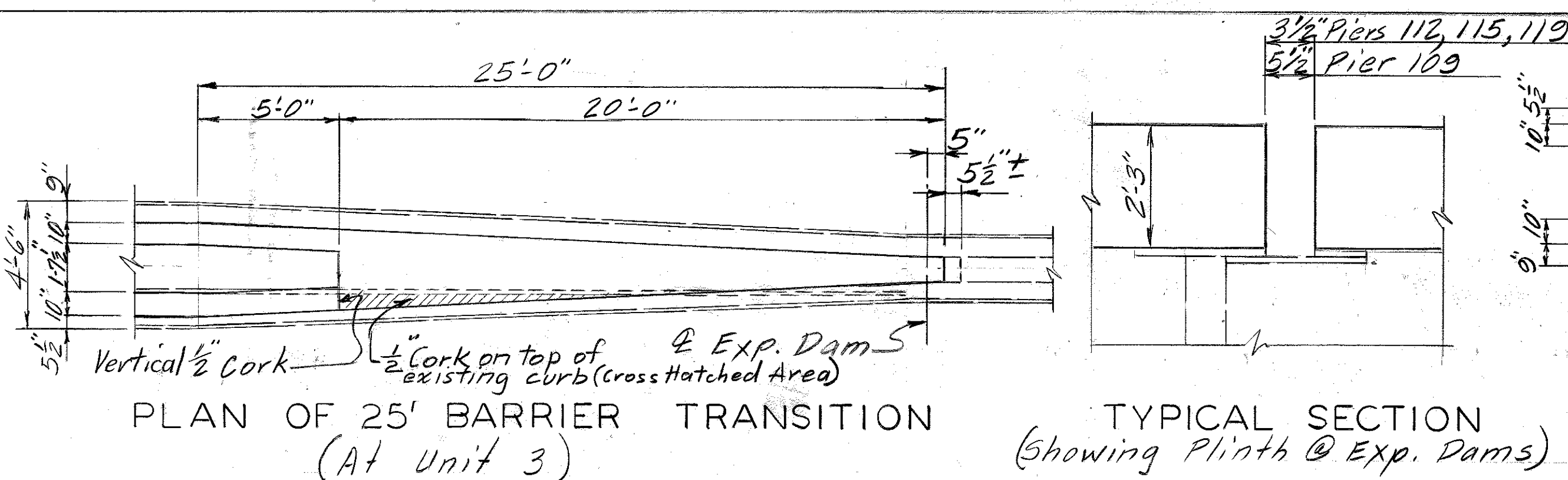


SECTION AT SIGN SUPPORT
 Details not shown same as Typical Section



CONSTRUCTION REVISION
 Sheet 55

COMMONWEALTH OF KENTUCKY
 BUREAU OF HIGHWAYS
 FRANKFORT
 COUNTY OF
 JEFFERSON
 LOUISVILLE - LEXINGTON
 ROAD
 STATION P. E. PROJECT NO.
 CONSTRUCTION PROJECT NO. MAINTENANCE PROJECT NO. DRAWING NO. 19238

[illegible]

COMMONWEALTH OF KENTUCKY

BUREAU OF HIGHWAYS

**FRANKFORT
COUNTY OF**

JEFFERSON

LOUISVILLE - LEXINGTON

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STATION	Road P. E. PROJECT NO.
CONSTRUCTION PROJECT NO.	MAINTENANCE PROJECT NO. 19238